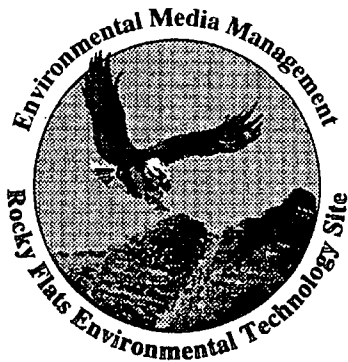




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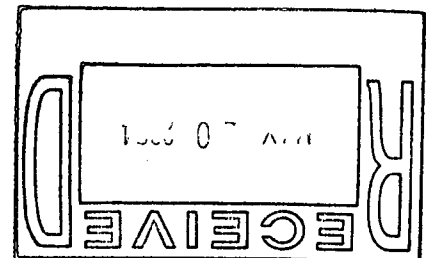


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**FINAL**

**Fate and Transport  
Modeling of Volatile Organic  
Compounds at Rocky Flats  
Environmental Technology Site**

**April 2004**



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## Executive Summary

This report summarizes computer modeling results of the fate and transport of volatile organic compounds (VOCs) present in groundwater at the Rocky Flats Environmental Technology Site (RFETS or Site) in Golden, Colorado. Results can be used for developing decisions about contaminated groundwater and for the Comprehensive Risk Assessment (CRA) that is part of the Remedial Investigation/Feasibility Study for the Site. The model estimates maximum, or long-term, groundwater VOC concentrations that may discharge to surface water at the Site.

### Modeling Scope

The modeling scope included the following steps:

- collected, synthesized, and reviewed all historical VOC data;
- developed a flow and transport model of historical conditions to determine appropriate parameter values; and
- developed a flow and transport model to predict long-term (or probable maximum) groundwater VOC concentrations for a proposed closure configuration that could discharge to surface water.

The modeling scope included simulation of saturated zone transport only within unconsolidated material and weathered bedrock that define the Upper Hydrostratigraphic Unit (UHSU) at RFETS. VOCs in the UHSU do not migrate downward through the much lower permeability unweathered bedrock of the Lower Hydrostratigraphic Unit (LHSU) and into the underlying regional Laramie-Fox Hills Aquifer (RMRS, 1996).

Simulation of VOC fate and transport within the unsaturated zone or surface streams was not considered. Surface water impacts from groundwater VOCs were not modeled or assessed. The scope also did not include the simulation of the fate of any contaminants other than VOCs. Rather than simulating the fate and transport of total VOCs in groundwater, individual VOCs were modeled because differences in their chemical properties cause them to transport at different rates. Finally, this study did not evaluate the performance of current groundwater collection systems.

VOCs associated with the PU&D Yard Plume, south of the Present Landfill, were not considered in this modeling. They are addressed in a separate report.

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## Modeling Approach

The approach used to develop flow and transport models involved several steps. First, historical VOC groundwater concentration data were collected and analyzed. Then Historical Release Report (HRR) information was evaluated and incorporated into a database/Geographical Information System (GIS) so this information could be further assessed in conjunction with available hydrologic and hydrogeologic information to identify possible groundwater VOC sources. Using the GIS, observed groundwater VOC concentration data were spatially associated with historical release information to identify likely sources. In general, VOCs detected in the greatest number of groundwater sample locations exhibited the highest concentrations. A total of 19 VOC-impacted areas, referred to as plume signature areas (PSAs), were identified where at least one source explained a group of associated groundwater concentration sample locations. PSAs represented an attempt to identify approximate, but distinct, source-plume areas. As such, PSA delineations were uncertain due to uncertainties in VOC source information and the complexity of historical groundwater flow pathways.

Further data analysis suggested that most PSAs probably have already intercepted groundwater discharge areas. Moreover, relatively steady VOC concentrations in time observed at most sample locations suggested that PSAs have probably reached stable configurations, though some areas may still be developing (i.e., 903 Pad area). The steady well concentration trends also suggested that VOC sources have probably reached steady concentrations in time. Non-aqueous phase liquid (NAPLs) VOC sources typically produce long-term, steady, dissolved phase concentrations in groundwater.

Primary VOCs considered in this study were tetrachloroethene (PCE), carbon tetrachloride ( $\text{CCl}_4$ ), and their daughter products. Successive daughter products of PCE include trichloroethene (TCE), cis-1,2-dichloroethene (cis-1,2-DCE), and then vinyl chloride (VC). Successive daughter products of  $\text{CCl}_4$  include chloroform ( $\text{CCl}_3$ ) and then methylene chloride ( $\text{CCl}_2$ ). The occurrence of both parent and daughter product VOCs within most PSAs suggested that biodegradation occurs at RFETS, though an independent study suggested rates are variable but low throughout the model area. Additional evidence suggested that TCE occurred as a source in several areas, though it is probably also a degradation product of PCE.

Draft CRA surface water preliminary remediation goals (PRGs), which define the relative risks associated with each VOC, were used as the basis for determining whether individual PSAs would be modeled. The total number of PSAs modeled was reduced to nine for PCE, 10 for TCE, and seven for  $\text{CCl}_4$ .

## **Modeling Results - Current Conditions**

The current distribution of VOCs for each PSA was evaluated using a groundwater flow path analysis and a sensitivity analysis of reactive transport for the PCE and CCl<sub>4</sub> degradation chains. Iterative groundwater flow path analysis was conducted for eight different model areas (at least one PSA in each model). The results confirmed initial assumptions about possible VOC source locations, the number of sources, timing of sources, and groundwater pathways and travel velocities. Results showed that it was reasonable to assume groundwater sources were introduced approximately 30 to 50 years ago. HRR information supported this conclusion. Flow path analysis resulted in 22 different source areas that explained concentration distributions in the 19 separate areas.

Several conclusions were made from the reactive transport sensitivity analysis conducted for each PSA. Many factors affected the fate and transport of VOCs from source release time to present. The most important included:

(1) hydraulic conductivity; (2) depth of source introduction; and (3) biodegradation rates. Factors such as sorption, dispersion, source concentration, diffusion, and porosity affected the fate and transport less significantly. The range of effective source concentrations and source depths, determined through this modeling, reproduced the range of historical time-averaged concentrations within each PSA for both parent and daughter VOCs.

Modeling showed that three-dimensional groundwater flows were important in supporting a detailed flow and transport model for the Site. Groundwater flows downward in upper mesa areas, but then flows upward near the bottom of hillslopes or streams due to the hillslope structure. This is important in the conceptual model because slower flow rates from sources allow for more efficient degradation (mostly within bedrock) before it eventually emerges at stream areas. Because evapotranspiration (ET) dominates near-stream hydrology at RFETS, model results indicated increasing amounts of VOCs in groundwater were lost via ET near streams. This loss to ET was significant because it helped attenuate VOCs in groundwater within most PSAs before discharging as baseflow to streams, seeps, ponds, or overland flow.

## **Model Results – Proposed Closure Configuration**

Proposed land configuration modifications were simulated in the local-scale integrated flow model for the Industrial Area (IA). The integrated modeling produced a three-dimensional flow field for reactive transport modeling of the closure configuration and was used to identify areas where groundwater discharged to the land surface. Discharge frequency and rates were also calculated by the model, but the model did not simulate groundwater discharge to streams along North Walnut, Woman Creek, or South Walnut in the B-pond area. The Site-Wide Water Balance (SWWB) model was used to predict actual discharge locations, rates, and frequency in these areas, but the original SWWB



closure scenario configuration was modified to reflect the current proposed land configuration.

Simulated closure-condition groundwater flow velocities changed little from the current configuration. This was because hillslope morphology (surface and bedrock topography) strongly controls groundwater flow directions at RFETS. The largest flow direction changes occurred near buildings with deep foundations where footing drains were assumed to be deactivated (Buildings 371, 771, 881, and 991); along South Walnut Creek east of Building 991; and where the proposed channel was re-engineered to eliminate roadways, fenced areas, and associated culverts. Local flow directions near the current Mound Groundwater Collection System changed notably due to this proposed reconfiguration.

Simulated closure-condition groundwater levels increased throughout the model area due to the proposed land reconfiguration. In some seep areas, groundwater discharged to three of four modified IA streams (drainage between Buildings 371 and 771, the drainage along South Walnut south of Building 991, and in the drainage west of Building 371). Average conditions indicated that the discharge occurred only in the drainage between Buildings 371 and 771.

Sensitive transport model parameters were adjusted to produce a range of simulated concentrations that bracketed the distribution of available time-averaged VOC concentrations within each PSA. This same range of parameter values was then used to simulate multiple closure configuration reactive transport model simulations and to estimate a range of possible groundwater concentrations at groundwater discharge areas. Simulations were run long enough to produce steady concentrations at groundwater discharge areas.

In four of the eight PSAs modeled, at least one of the closure-condition simulations produced long-term groundwater concentrations for TCE or  $\text{CCl}_4$  that were above the draft surface water PRGs at groundwater discharge areas. These PSAs include:

- the East Trenches area;
- the Oil Burn Pit/Mound area;
- Building 771; and
- Ryan's Pit/903 Pad area.

Of these areas, only Building 771 had average groundwater concentrations (for all closure-condition simulations) below draft PRGs at groundwater discharge areas.

Closure-condition integrated flow modeling results indicated that groundwater discharged to several areas. Groundwater discharged to the surface drainage west of Building 771 due to shallow bedrock and the Arapahoe Formation "No. 1

Sandstone" (herein referred to as Arapahoe Sandstone) in the area. Although the Arapahoe Sandstone is only present as shallow, discontinuous lenses throughout the model area and has no connection to the much deeper regional confined aquifer (i.e., Laramie/Fox Hills Aquifer), it is more permeable than the surrounding claystone/siltstone matrix and controls local groundwater flows. For a typical climate, some simulated groundwater discharged into the South Walnut Creek drainage north and down-gradient of the Mound Groundwater Collection System. The discharge area increased during precipitation events. Southeast and down-gradient of the Ryan's Pit area, groundwater discharge was also simulated to the SID and Woman Creek, but only during larger precipitation events. The integrated flow model did not simulate groundwater discharge to south Walnut Creek, though this probably occurs, for example to Pond B-2.

In general, simulated results indicated that only parent compounds  $\text{CCl}_4$  and TCE were above draft PRGs at groundwater discharge areas. All other daughter products and PCE did not. For the other PSAs not listed above, simulated groundwater VOC concentrations in groundwater discharge areas for closure simulations were below the draft PRGs. This is due to a combination of the following:

- slower groundwater velocities in bedrock (caused by typically unsaturated unconsolidated material in upper hillslope areas);
- the simultaneous combined effect of attenuation processes (such as biodegradation, sorption, volatilization, and ET loss) reduced VOC concentrations in groundwater discharge to surface areas; and
- loss through volatilization (not simulated with the reactive transport model but results in conservatively high concentrations).

In most PSAs, the dominant attenuation process appeared to be low rates of biodegradation, though ET loss is more significant in the eastern PSAs (i.e., southeast of the 903 Pad area and in the East Trenches area). Although, parameter values for each closure configuration model reproduced historical time-averaged concentrations, some combinations underestimated concentrations while others over-predicted concentrations. The latter case likely over-predicted long-term closure concentrations at groundwater discharge areas. As such, a single run should not be considered to be an accurate representation of closure concentrations, or even the most reasonable. Rather, the range of predicted output should be used in assessments.

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## Acronyms and Definitions

°C	Degrees Celsius
°F	Degrees Fahrenheit
af	Artificial Fill
msl	average mean sea level
Arapahoe Sandstone	Refers to the Arapahoe Formation No. 1 Sandstone, described in more detail in EG&G (1995a, and 1995b) as sandstones that occur only within the UHSU weathered bedrock material. This sandstone is discontinuous and lenticular.
BZ	Buffer Zone
CCl <sub>2</sub> or CH <sub>2</sub> Cl <sub>2</sub>	Methylene chloride (also dichloromethane, or methylene dichloride)
CCl <sub>3</sub> or CHCl <sub>3</sub>	Chloroform (also trichloromethane)
CCl <sub>4</sub>	Carbon tetrachloride (also tetrachloromethane)
CDPHE	Colorado Department of Public Health and Environment
cf	Cubic feet
cis-	Refers to the "cis" isomeric configuration of an organic compound.
CRA	RFETS Site Comprehensive Risk Assessment
DCA	Dichloroethanes in general, including: 1,1-DCA and 1,2-DCA. 1,1-DCA is the more common isomer in RFETS groundwater and is the first daughter of the natural dechlorination of 1,1,1-TCA.
DCE	Dichloroethenes in general, including: cis-1,2-dichloroethene (CHCl=CHCl); trans-1,2-dichloroethene; and 1,1-dichloroethene (CH <sub>2</sub> =CCl <sub>2</sub> ). All three DCE isomers are potential first daughters of the dechlorination of TCE. Cis-1,2-DCE is the most abundant daughter produced by biodegradation of TCE, and cis-1,2-DCE is the second daughter of the PCE decay chain. A further complication is that 1,1-DCE is also a potential first daughter of 1,1,1-TCA.
discretization	The representation of a model domain into a group of discrete cells, or elements.

DNAPL	Dense Non-aqueous Phase Liquid. Refers to halogenated hydrocarbon solvents that are denser than water. DNAPLs like carbon tetrachloride, have a limited solubility in groundwater, and are sometimes found as a separate liquid phase in an aquifer, or monitoring well.
DO	Dissolved oxygen concentrations measured in groundwater samples at the time of sampling.
DOE	United States Department of Energy
EPA	United States Environmental Protection Agency
ER	Environmental Restoration
ET	Evapotranspiration
$f_{oc}$	The fraction of organic carbon by weight in a soil.
ft	Foot or Feet
FY	Fiscal Year
GIS	Geographic Information Systems
GW	Groundwater
IA	Industrial Area
IHSS	Individual Hazardous Substance Site, numbered for identification.
in	Inch(es)
Kaiser-Hill	Kaiser-Hill Company, LLC.
$K_b$	"Bulk attenuation rate constant" determined for a specific time interval from a plot of natural log of analyte concentration versus distance along a contaminant flowpath. Positive rates indicate that natural attenuation is occurring along the flowpath. The units of $K_b$ in this report are "per year".
$K_d$	The "partition coefficient" or "distribution coefficient" in ml/g, is the equilibrium ratio of the mass of a chemical sorbed on a solid phase, to its concentration in aqueous solution. Commonly used as a simple approach to modeling chemical adsorption by soils.
$K_h$	Horizontal Hydraulic Conductivity
KH	Kaiser-Hill Company, LLC.
km	Kilometer(s)

$K_{oc}$	The distribution coefficient in ml/g, describing the adsorption of an organic chemical from aqueous solution to a substrate of organic carbon in soil. Used with $f_{oc}$ to estimate $K_d$ values for CAHs.
$K_s$	Soil Hydraulic Conductivity
$K_{sat}$	Saturated Hydraulic Conductivity
$K_v$	Vertical Hydraulic Conductivity
LHSU	Lower Hydrostratigraphic Unit
LNAPL	Light Non-aqueous Phase Liquid. LNAPLs are less dense than water and often float on top of the groundwater table. Gasoline and jet fuel are examples of LNAPLs.
m	Meter(s)
m/s	Meter(s) per second
m/yr	Meter(s) per year
$m^2$	Meter(s) squared
$m^3$	Cubic meters
$m^3/s$	Meter(s) cubed per second
MCL	Maximum Contaminant Level
mg/L	Milligram per liter
MIKE 11	Surface water modeling computer software
MIKE SHE	Integrated ground and surface model computer software
mm	Millimeter(s)
MODFLOW	U.S.G.S. Groundwater Modeling computer software
MSL	Mean Sea Level
N/A	Not Applicable
NAPL	Non-aqueous Phase Liquids. They may be mixtures of chlorinated solvents, or mixtures of solvents and hydrocarbons of unknown density.
PCA	Perchloroethane, e.g. 1,1,1,2-PCA, a potential parent of 1,1,2-TCA.
PCE	Tetrachloroethene, or perchloroethylene, $CCl_2=CCl_2$ , was an important solvent used in industrial operations at RFETS, and is the parent molecule of its decay chain. One solvent brand used at RFETS was called "Perclene".
PET	Potential Evapotranspiration
PRG	Preliminary Remediation Goal

PSA	"Plume signature area", a local area of RFETS under which the groundwater contains detectable concentrations of one or more chlorinated solvents. This term is defined by Prucha et al. (2003). Each of the more than two dozen PSAs identified at RFETS has been assigned an integer identification number.
Q	Flow rate
Qal	Valley Fill Alluvium
QC	Colluvium
QC	Quality Control
Qp	Piney Creek Deposits
Qrf	Rocky Flats Alluvium
RFCA	Rocky Flats Cleanup Agreement
RFETS	Rocky Flats Environmental Technology Site
SID	South Interceptor Ditch
Site	Rocky Flats Environmental Technology Site
SW	Surface water
SWD	Soil and Water Database
SWWB	Site-Wide Water Balance
S <sub>y</sub>	Unconfined Storage Coefficient
SZ	Saturated Zone
TCA	Trichloroethanes: 1,1,1-trichloroethane (CH <sub>3</sub> -CCl <sub>3</sub> ), and 1,1,2-trichloroethane (CHCl <sub>2</sub> -CH <sub>2</sub> Cl). 1,1,1-TCA is the more common TCA isomer in groundwater at RFETS, and is the parent of a decay series. It decays mainly to 1,1-dichloroethane, or to acetic acid (acetate ion), but a fraction may decay to 1,1-dichloroethene.
TCE	Trichloroethene, CHCl=CCl <sub>2</sub> , is a manmade industrial solvent that was used at RFETS. It can also occur from the dechlorination of PCE. TCE concentrations in groundwater can be the result of its release to the environment or from degradation of PCE.
trans-	Refers to the "trans-" isomeric configuration, usually of halogen atoms on opposite sides of a carbon double bond.
ug/L	Microgram per liter
UHSU	Upper Hydrostratigraphic Unit
USGS	United States Geological Survey

UZ	Unsaturated Zone
VC	Vinyl chloride, or chloroethene, $\text{CH}_2=\text{CHCl}$ , is produced naturally by dechlorination of any of the DCE isomers. VC can be thought of as the third daughter of PCE decay, or the second daughter of TCE decay.
VOC	Volatile Organic Compound
WY	Water Year

## 1.0 INTRODUCTION

This report summarizes the development, application, and results of subsurface flow and transport models used to simulate the fate and transport behavior of dissolved volatile organic compounds (VOCs) at the Rocky Flats Environmental Technology Site (RFETS or the Site).

The flow and transport model was developed as a management tool to support decisions made about VOC-impacted groundwater at the Site, and the Comprehensive Risk Assessment (CRA) that is part of the RFETS Remedial Investigation/Feasibility Study (RI/FS). Within the CRA scope, human and ecological risks are determined from VOCs in groundwater reaching the ground surface discharge areas (herein assumed to include seeps, streams, or ponds). As a result, it was necessary to determine the most probable ranges of maximum VOC concentrations where groundwater discharges to seeps or surface water. Accurate estimation of concentrations using a model was not possible due to uncertainties associated with groundwater VOC sources, such as locations, depths, concentrations, and timing.

The complex three-dimensional groundwater flow and transport processes at RFETS precluded using simple analytical methods to predict maximum concentrations. As a result, a fully-distributed numerical flow and transport model was developed to predict possible ranges of maximum concentrations given source uncertainties.

The main objectives of the flow and transport modeling conducted for the CRA are summarized in Section 1.1. The modeling scope associated with the integrated flow and transport modeling is presented in Section 1.2. The report outline is described in Section 1.3, and limitations are described in Section 1.4.

### 1.1 Modeling Objectives

Three modeling objectives were identified. These are described below in order of importance:

- estimate a range of possible long-term concentrations for VOCs of concern in groundwater at surface discharge areas (i.e., seeps, streams, or ponds) for a proposed Industrial Area (IA) closure configuration;
- estimate areas where groundwater, impacted by VOCs, may discharge to the surface; and
- develop an integrated hydrologic flow model of the IA that may be used to assess groundwater and surface water response of proposed closure configuration modifications and integrated hydrologic response to potential groundwater collection system designs.



Although current groundwater collection systems were included in the flow and transport models to reproduce observed conditions, they were omitted from long-term closure simulations. The numerical resolution of models developed in this study was too coarse to adequately capture the local flow and transport dynamics associated with these systems. As such, the models developed were not intended to be used to evaluate the performance of these systems. Higher concentrations were estimated at groundwater discharge locations when the systems were left out of the model.

## 1.2 Modeling Scope

The modeling scope described here was developed to meet the project objectives outlined above. The following discussion outlines what is, and what is not included in this modeling scope.

This study considered the VOC transport in groundwater from assumed source areas to estimated groundwater discharge areas associated with streams, seeps, and ponds. VOC transport in surface water, or in the unsaturated zone, was not considered in this study because VOC concentrations in groundwater at discharge areas are conservatively high compared to the VOC concentrations in surface water in those areas. Once VOCs in groundwater discharge to surface water, concentrations likely decrease rapidly due to volatilization and degradation by sunlight.

The fate and transport of VOCs were simulated long enough to estimate steady-state groundwater concentrations at potential groundwater discharge areas. These long-term simulated concentrations represent maximum concentrations assuming source concentrations remain constant over time. This is reasonable for chlorinated VOCs, typically introduced into the environment as Dense Non-Aqueous Phase Liquids (DNAPLs), that are denser than water and flow downwards to lower permeability layers. Typically, DNAPL sources represent long-term sources of dissolved VOCs in groundwater that support specifying constant source concentrations in long-term simulations. In some instances, it will take many years for the dissolved contaminants to reach these discharge locations, especially with closure modifications to the Site.

This modeling scope considered both hydrologic effects and subsequent VOC fate and transport in groundwater due to proposed closure configuration modifications. Information on the proposed closure configuration is current as of July 2003. It should be noted that other alternative closure configurations were not analyzed in this study. The flow and transport models developed here can be modified to evaluate effects of other alternatives.

Only VOCs and their degradation products detected within the IA were considered in the current study. For example, even though VOCs have been detected in the Present Landfill area, they were not modeled in this study. Transport modeling only considers VOCs and not radionuclides, metals, nitrates,

or other dissolved or free phase (non-aqueous phase) contaminants. Moreover, only the fate of a select number of VOCs, detected at the greatest spatial frequency in wells across the Site and which are above current CRA-proposed surface water preliminary remediation goals (PRGs), were simulated. Generally, concentrations for these selected VOCs also exhibited the highest concentrations compared to other VOCs.

Total combined VOC concentrations in groundwater at discharge areas were not simulated. Each VOC moves in groundwater at different rates due to differences in chemical properties. As such, each VOC was modeled individually to simulate their fate and transport.

Although an integrated flow model of the IA was developed to meet the modeling objectives, only saturated zone flow and transport of VOCs was considered. The CRA modeling only required that the groundwater concentrations at discharge areas be estimated. As such, only a saturated zone flow model was required to simulate the advective-dispersive transport. Very little information exists on VOC sources, their form and distribution, and/or occurrence within the unsaturated or saturated zones. Without detailed information on VOC source configurations in the saturated zone, possible source concentrations must be determined through modeling. As a result, there was no need to consider flow and transport within the unsaturated zone or surface flow systems.

The integrated hydrologic flow model was developed with an appropriate spatial discretization to simulate VOC transport and hydrologic effects of RFETS closure. If the model is to be used as a design tool in the future, (e.g., for groundwater collection systems), grid refinement will be necessary to improve localized prediction accuracy. Furthermore, the hydrologic impacts of closure configurations using the Integrated VOC Flow Model can only be evaluated within the model area, a smaller area than covered by the Site-Wide Water Balance (SWWB) model. The smaller size permits higher grid resolution. It does not include features such as ponds, or the extents of Woman and Walnut Creek from the IA to Indiana Street to the east for reasonable computational efficiencies.

It is typical in flow and transport modeling that the quality and quantity of data are limited given the complexity inherent within most systems. As such, the model was used to predict a range of concentrations at groundwater discharge areas, rather than a single value that is subject to input uncertainty. This range reflected the uncertainty in all of the factors controlling transport of VOCs at RFETS.

### **1.3 Key Modeling Factors and their Uncertainties**

This section describes key modeling factors and their associated uncertainty. This was essential for developing reliable predictions to meet objectives stated in Section 1.1. To estimate VOC concentrations in groundwater at discharge areas

for the assumed closure configuration, suitable saturated zone flow and transport computer codes had to be used. Developing reliable models depended on the accuracy of all model inputs. As such, the range of uncertainty in model inputs was assessed so that uncertainty in model predictions (i.e., discharge area concentrations) could be determined.

The conceptualization of flow and VOC transport in RFETS groundwater is complex and dependent on many factors. Although, much of the current VOC concentration distributions in groundwater can be attributed to advective transport (groundwater flow), other important transport processes contribute to the currently observed three-dimensional concentration distributions. Each factor that controls either the flow, or VOC fate, and transport has uncertainties due to quality (i.e., sampling errors) and/or quantity (insufficient data) limitations. In addition, information on the system stresses, response (i.e. climate variability), land configuration changes with time, or water level variability also have uncertainty caused by measurement error and have quantity limitations (i.e., data gaps).

These factors influenced the overall quality of the conceptual flow and transport model, which was based on the most accurate understanding of the system. Important processes considered in developing the conceptual flow and transport model included the following:

- Groundwater Flow:
  - infiltration-recharge from surface system to the groundwater;
  - evapotranspiration (ET) from vegetation;
  - groundwater flow - pathways and velocities;
  - groundwater discharge to the surface in the form of baseflow to streams or seeps;
  - hydraulic influence of subsurface features at IA resulting from Site operations; and
  - groundwater collection systems currently in operation.
- Fate and transport processes:
  - degradation of dissolved VOCs due to biological activity;
  - dispersion controlled by the flow and subsurface heterogeneity;
  - sorption and desorption from the porous medium;
  - molecular diffusion;
  - volatilization; and
  - ET.

Estimating reliable ranges of groundwater VOC concentrations in discharge areas depended on two important factors: (1) VOC source information; and (2) observed groundwater VOC concentration data. Information on VOC sources was important for simulating long-term fate and transport, but it was limited both in quantity and quality at RFETS. This is true of most sites. As a result, several assumptions are made about VOC sources to model their fate and transport. Some of these include the following:

- concentration of dissolved constituents in the groundwater at the possible locations;
- depth at which the dissolved mass flux enters the groundwater;
- volume over which the source is active;
- date(s) of source release; and
- type of source (i.e., single or multiple VOCs, slug, or continuous source).

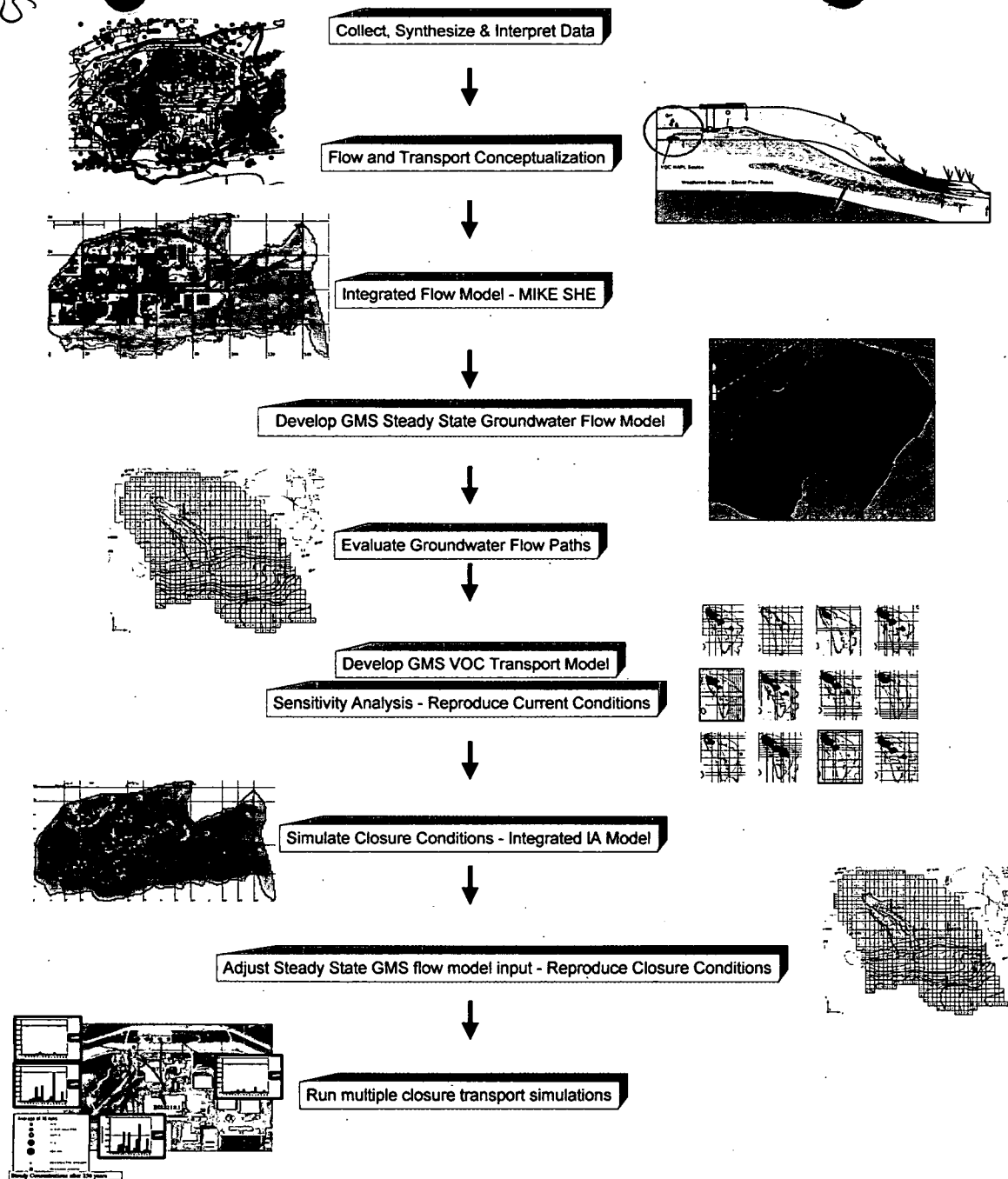
Plume characterization was another important element affecting fate and transport modeling. Time-averaged concentrations in RFETS wells were, in many cases, due to multiple sources (i.e., commingling of plumes) or due to variations in source concentrations over time. The relationship between sources and concentrations in each well was uncertain. As such, our approach was iterative and required assumptions associating observed well concentrations with assumed source areas. Every attempt was made to justify the source locations, but these assumptions lead to uncertainty.

Uncertainty associated with these assumptions strongly influenced the accuracy and reliability of model predicted concentrations. In fact, the uncertainty was large enough at RFETS, that it precluded the development of a more traditional "calibrated" flow and transport model to simulate highly accurate fate and transport of VOCs in groundwater. Instead, a sensitivity analysis was used (as described in Section 1.4) to produce a range of possible groundwater VOC concentrations under the proposed closure configuration that bound the range of model parameter uncertainty.

#### **1.4 Modeling Approach and Report Organization**

The key steps taken in the development of the integrated flow and VOC fate and transport model for the IA are summarized on Figure 1.1. The first step, described in Section 2.0, was to obtain, synthesize, and interpret available hydrologic, geologic, and chemical (constituent concentration) data. This step was critical in defining where gaps exist with respect to data quantity and quality.

25



- Geology
- Hydrology
- Chemical Data
- Saturated Zone Flow and Transport – important
- Local scale features important to transport
  - Bedrock surface
  - Hillslope structure
  - Utilities

- Calibrate key system parameters
- Simulate integrated flow conditions accurately

- Forms the basis for Reactive Transport Simulations - RT3D

- Confirm Source Locations
- Confirm Source Timing
- Confirm Pathways

- Identify key parameters affecting transport/fate
- Estimate range of key parameters reproduce observed trends
- Determine effective Source concentrations
- Determine appropriate source depths
- Confirm source locations

- Calculate closure condition heads and discharge

- Forms the basis for Reactive Transport Simulations - RT3D

#### PREDICT:

- range of Cmax in groundwater at surface discharge locations
- approximate extent of discharge locations

Figure 1.1. Modeling approach

Best Available Copy

In the second step, a detailed and consistent conceptual flow and transport model was developed for the IA and eastern area (Section 3.0). The IA conceptual flow model is similar to the SWWB model, but it describes the fate and transport of VOCs in the IA groundwater. The MIKE SHE code was used to develop a more refined integrated hydrologic flow model of the IA than that developed for the SWWB. A refined flow model was needed to simulate transport more accurately. The development of this model was similar to that described in the SWWB Modeling Report (Kaiser-Hill, 2002).

An integrated hydrologic flow model of the IA and the adjacent eastern area, more refined than the regional SWWB model, was developed to better resolve local-scale flow conditions. Model development and results for simulation of current conditions Water Year 2000 (WY2000) are described in more detail in Section 4.0.

Steps taken in the flow and transport code selection process are summarized in Section 5.0. These steps involved defining model needs, selection criteria, and the selection process. The Groundwater Modeling System (GMS) developed by the United States Department of Defense (DOD) and supported by the Department of Energy (DOE) was selected.

The development and modeling results of individual steady-state GMS MODFLOW groundwater flow models are described in Section 6.0 for individual plume areas where VOC concentrations were above currently proposed CRA goals. The parameter estimation code, PEST, available in the GMS software, was used to help automate the flow parameterization. The number, locations, and source release times of VOCs into groundwater in individual plume areas were evaluated using the particle tracking software, MODPATH, developed by the USGS. Given the complex system and flow parameter uncertainties, sensitivity of key flow model parameters was bracketed over a reasonable range in which observed groundwater level distributions and discharges could be reasonably reproduced.

The development and modeling results of reactive transport models to simulate the fate and transport of VOCs in groundwater are also described in Section 6.0. In this section, effective source area concentrations, source depths, source locations and key transport parameters were identified. In addition, ranges of values for key model input parameters (that adequately reproduce observed concentration distributions) were estimated for each plume area. The RT3D code, developed by the Pacific Northwest National Laboratory (<http://bioprocess.pnl.gov/rt3d.htm>), was used to model the reactive transport of observed VOC degradation chains, along with other attenuation processes including sorption, diffusion, dispersion, and ET. RT3D is an extension of the more popular MT3DMS multi-species transport computer software.

To simulate groundwater conditions for the proposed closure configuration, various model input parameters were modified based on details provided by Site

personnel. Details of this modeling are presented in Section 6. Results of these simulations were used to identify discharge areas for typical and wet-year climate, which in turn were used to assess the potential of discharge of VOCs into surface waters.

Proposed closure configuration simulation results from the integrated flow model were also used as the basis for simulations using the GMS MODFLOW and RT3D codes. Long-term VOC fate and transport simulations for various closure configuration runs are described in more detail in Section 6.0. A range of long-term, maximum groundwater VOC concentrations at discharge areas are estimated in this section. The ranges of concentrations simply reflect uncertainty in key flow and transport model input parameter values. Results of multiple runs are then compared against currently proposed surface water PRG values. This information, in combination with groundwater discharge areas calculated using the integrated flow model, provides a basis for the CRA analysis of VOCs discharging from groundwater to surface water.

Summary and conclusions are presented in Section 7.0, and references are listed in Section 8.0.

## 2.0 DATA COMPILATION AND ANALYSIS

The collection and compilation of geologic, hydrologic, and chemical data to support delineation of current VOC sources and associated plumes are described in this section. Large amounts of data required database development and careful, iterative spatial analyses using various GIS techniques. The original data required filtering and analysis for the purposes of this study.

### 2.1 Geologic Data

Well geologic data locations (as of August 2002) are presented on Figure 2.1. The symbols indicate the lowest formation penetrated by the well. Some areas, such as the IA and Solar Ponds areas, contain many geologic data points whereas others to the east and southeast of the 903 Pad area have less data and more uncertainty about subsurface geology.

#### 2.1.1 Weathered and Unweathered Bedrock Depths

The three geologic units at the Site from the surface down are the unconsolidated deposits (alluvium and colluvium), weathered bedrock (mostly claystone with Arapahoe Sandstone lenses), and unweathered bedrock. In this study, the term Arapahoe Sandstone refers to the Arapahoe Formation "No. 1 Sandstone" that is discontinuous and occurs only within the UHSU (see Section 3.2 for more discussion). Maps of the depth to weathered bedrock (contact with the overlying unconsolidated deposits) and the depth to unweathered bedrock are shown on Figure 2.2 and Figure 2.3. The interpolated bedrock surfaces were prepared for the SWWB model (Kaiser-Hill, 2002).

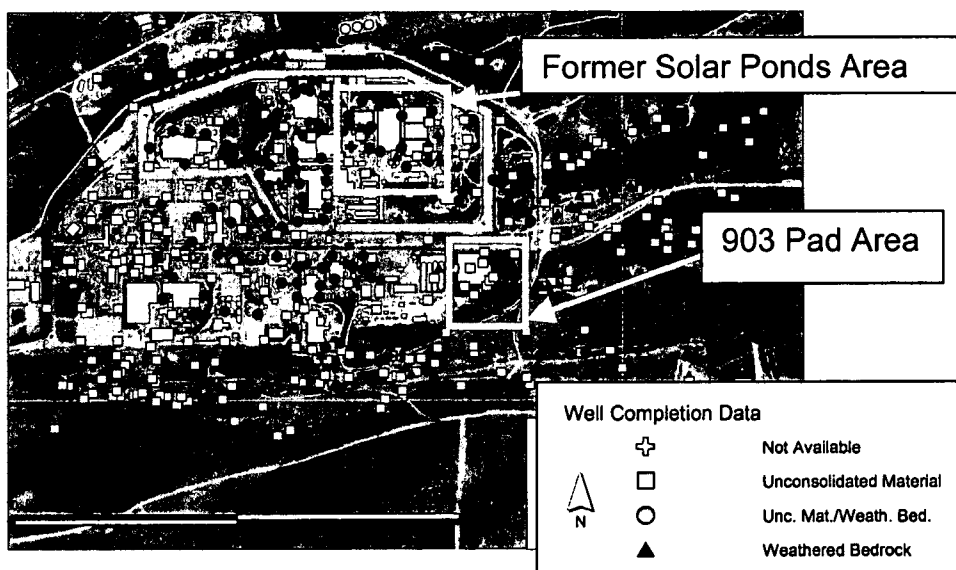


Figure 2.1. Locations of geologic borehole information. The symbols indicate the lowest formation penetrated.





Figure 2.2. Depth to weathered bedrock (unconsolidated material thickness).



Figure 2.3. Depth to unweathered bedrock (base of the model).

## 2.2 Hydrologic Data

Groundwater level (head) data from quarterly water level measurements during WY2000 were used to constrain integrated flow modeling results. These data also were used to establish appropriate initial conditions for groundwater flow modeling. This time period was selected because of the quantity of groundwater level data. Gradients and flow directions modeled using these water levels were later used to evaluate contaminant transport with respect to possible source locations (Section 6). Figure 2.4 shows both continuous and quarterly monitored well locations where groundwater levels were collected during April, 2000.

Local drainage systems in the unconsolidated material affect the natural groundwater flow and made it difficult to accurately define groundwater levels in these localized areas without additional data. Near-stream areas were most important for calibrating seasonal groundwater discharge response within each VOC plume area. The lack of groundwater level information during early Site operations (i.e., 1950 to 1985) precluded reproduction of historical groundwater flow conditions at the Site.



Figure 2.4. Locations of groundwater level data for April, 2000.

## 2.3 VOC Chemical Data

Site-wide water quality analysis results are compiled in the RFETS Soil Water Database (SWD). Based on the SWD, nearly 200 organic contaminants were

detected at the Site. Information on contaminant releases at RFETS was documented in a series of Historical Release Reports (HRR) prepared for the Site (DOE, 1992; DOE, 1993; DOE, 1994; DOE, 1995; Kaiser-Hill, 1996; Kaiser-Hill, 1997; Kaiser-Hill, 1998; Kaiser-Hill, 1999; Kaiser-Hill, 2000). At least 366 documented releases were reported, though, not all were associated with VOCs. A spatial GIS database of the documented releases and available historical VOC concentration data was developed to assist in developing a set of plumes and associated sources (Sections 2.3.1 and 2.3.2). The number of specific VOCs selected for modeling was systematically reduced to a manageable subset to assess the basic characteristics associated with historical and current plume distributions and movement (Section 2.3.3) and subsequent modeling (Section 6.0). Plots of groundwater VOC concentrations with time at sample locations through RFETS were constructed to identify temporal trends in plume behavior, or possible temporal trends in near-source concentrations (Section 2.3.4). Finally, the selected contaminants were compared with their draft surface water PRGs (Section 2.3.5) to determine which were most likely to impact groundwater concentrations at surface discharge areas.

### **2.3.1 VOC Source Database Development**

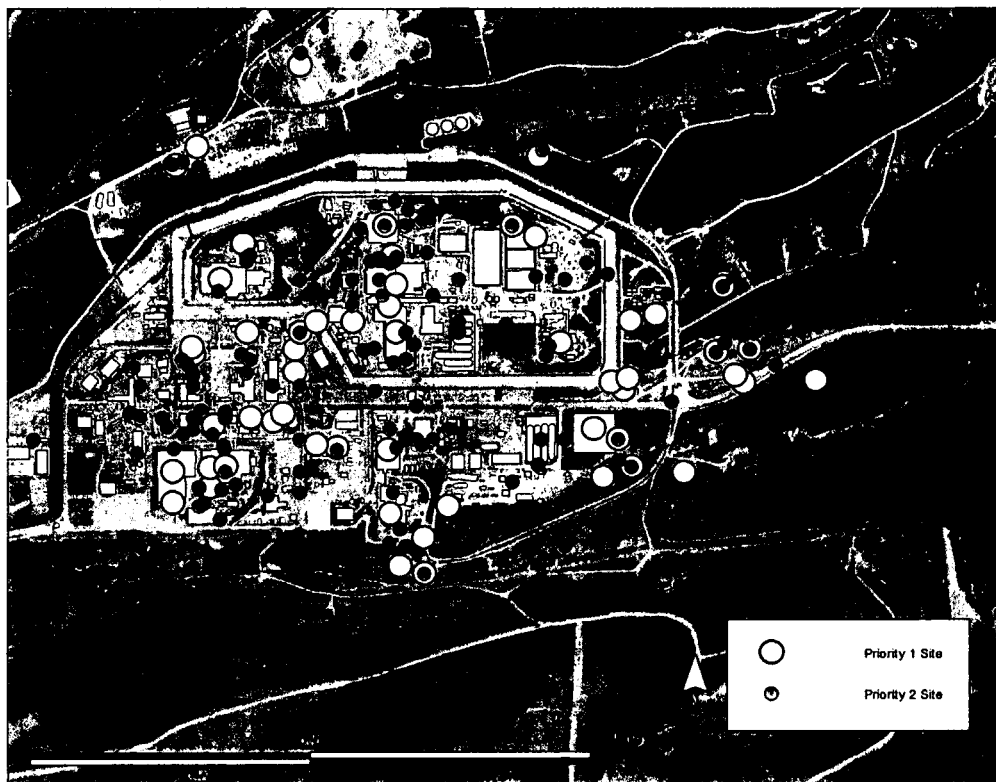
Historical records of known or suspected chemical releases to the environment are documented in the HRR and summarized in Appendix A. These documents have been compiled annually or quarterly since 1992, and they represent the best known information of environmental releases at RFETS. Contaminant releases documented in the HRR date back to 1952, when Site operations began. Updated reports include new releases, as well as any additional information gathered regarding previous releases. In nearly all cases, there is no direct evidence indicating that HRR sources reached Site groundwater and can be tied to observed groundwater contamination. Therefore, for the purpose of this modeling effort, all sources in the HRR were considered potential VOC sources.

The HRR source information was entered into a GIS database to facilitate its use for this modeling project. The HRR and subsequent update reports were reviewed for any mention of a potential or verified release of volatiles, organics, solvents, chemicals, transformers, fuel, oil/grease, nitrates, PCBs, or process waste. These releases were entered separately into the database along with specific information about release timing, concentration, volume, composition, and cleanup.

The database entries were spatially referenced in GIS using Site coordinates reported in the HRR. Locations for many of the releases were uncertain (to within 500 feet). To locate possible VOC sources more accurately, Subject Matter Experts (SMEs) with expertise in the HRR information extensively reviewed the database. While uncertainty remains about exact locations of historical releases, the GIS database provided a SME-reviewed summary of published information that was readily queried and easily displayed.

The 366 entries in the database were classified as either Priority 1, 2, or 3 releases. A release was categorized as a Priority 1 release if a large volume (> 100 gallons) was documented, or if the release was considered significant by Site SMEs. Smaller releases (< 100 gallons) were categorized as Priority 2 releases. Very small releases considered insignificant (i.e., 0.5 gallon ethylene glycol spill in parking lot, cleaned up immediately), were classified as Priority 3 releases. Priority 1 and 2 releases were scrutinized during the review process. Priority 3 releases were not reviewed extensively given that the associated release volume was small and likely did not result in groundwater sources. Figure 2.5 shows the potential Priority 1 and Priority 2 release locations.

Despite efforts to compile all available information on releases into an all encompassing database, many significant data gaps and uncertainties remained. This release information was considered only as a guide in locating potential sources in the transport modeling effort. The significant uncertainties are recognized as a limitation in model development.



**Figure 2.5. Priority 1 and Priority 2 release locations. A complete map correlating release sites with HRR information is included in Appendix A.**

### **2.3.2 Groundwater VOC Database and GIS**

A digital database was developed to support both temporal and spatial analysis of the available groundwater VOC information at RFETS. The database was also used to develop groundwater VOC concentration time-series for analysis discussed in Section 2.3.4. Useful statistical information associated with time-varying groundwater concentrations were calculated in the database and subsequently used for spatially analyzing the distribution and trends of VOCs in groundwater using GIS techniques. Information from the digital database, the HRR release database, and approximate groundwater flow directions were systematically used to infer groundwater VOC source areas and the associated area, or areas of impacted groundwater.

The database was developed from an initial subset of 136,411 UHSU lab analytical records from the SWD database (provided by the SWD database manager). This initial subset included groundwater sample results for 36 analytes (including VOCs and supporting analytes such as Iron and Manganese), along with detection limits, Quality Control (QC) codes (duplicates), dilution amount, result value, and validation qualifiers for each sample. About 41,500 samples were lab detects. Non-detect sample results were retained in the database so that areas with no samples above the detection limit could be used to delineate areas with detectable concentrations. Duplicate sample results varied somewhat from the "real" sample results, but they were retained for averaging purposes.

The initial subset was refined using Microsoft Access to select only the highest risk VOCs, listed in Section 2.3.3. This subset of high risk VOC samples was exported to Microsoft Excel where time series for each VOC of each well were graphed along with detection limits to determine if there were any discernible trends and to generally evaluate the data consistency (Section 2.3.4). The dataset of high risk VOCs was then queried to select all samples that could be classified as "detect". Samples rejected by verification or validation were excluded as were samples where the result and detection limit were equal. Summary statistics were performed to determine the average concentration, minimum and maximum concentration, standard deviation, and number of detect and non-detect samples for each VOC at each sample location. Sample locations with at least one "detect" sample were included in the summary, and only the detect samples were used in the summary statistics. Non-detect samples were excluded because they are reported below the detection limit. A separate summary dataset of sample locations with all samples at or below the detection limit was compiled to help delineate areas with higher concentrations. The summary statistics for each of the sample locations was included in the GIS database to facilitate spatial/conceptual assessment.

### 2.3.3 VOCs Selected for Analysis

An initial subset of 10 VOCs (Table 2.1) was selected from the nearly 200 organic chemicals observed at the Site for further temporal analysis based on spatial distribution, frequency of sample detects, and sample concentration. This subset was cross-checked with the Site Water Program's list of VOCs of interest (as defined by inclusion in the former annual VOC composite mapping of the 2001 RFCA Annual Groundwater Monitoring Report) and all were on the list. These chemicals are all parent or daughter products of the PCE, TCA, and  $\text{CCl}_4$  degradation chains.

Table 2.1. 10 VOCs selected from 200 chemicals in SWD for further evaluation (spatial lots and temporal trends).

Selected VOCs	Acronym
TETRACHLOROETHENE	PCE
TRICHLOROETHENE	TCE
cis-1,2-DICHLOROETHENE	cis-1,2-DCE
VINYL CHLORIDE	VC
1,1-DICHLOROETHENE	1,1-DCE
CARBON TETRACHLORIDE	$\text{CCl}_4$
CHLOROFORM	$\text{CCl}_3$
METHYLENE CHLORIDE	$\text{CCl}_2$
1,1-DICHLOROETHANE	DCA
1,1,1-TRICHLOROETHANE	TCA

### 2.3.4 Temporal Concentration Trends

The absence of increasing or decreasing trends at most sample locations suggests that most plumes are relatively stable configuration, implying that the source is also constant. If a plume was growing, the concentration at locations down-gradient from the source would increase with time. However, the usefulness of the sample dataset to detect trends was constrained by the limited time range over which sampling has taken place. Sampling was not initiated until

1986, long after most of the releases occurred in the 1950s and 1960s, apparently after most plumes had stabilized. If a plume was shrinking (i.e., a slug source), near-source sample concentrations would likely decrease with time.

The variability in sample concentrations (typical ranges between one and 1.5 orders of magnitude) is attributed to seasonal variations in groundwater levels (Figure 2.6). Clearly, other factors include daily variation, well completion, sampling, and sample analysis. This causes groundwater fluxes, and therefore, solute fluxes through source zones to vary in time. As a result, sample locations down-gradient of source zones reflect seasonally varying concentrations, but they have relatively steady long-term concentrations. Seasonal variability in sample concentrations with time could be attributed to variable spatial and temporal distribution of infiltration from ground surface (i.e., runoff, or precipitation) through sources within the vadose zone as well. Finally, it could be due to changes in source concentrations with time, or changes in source NAPL architecture, though, this probably would not account for the apparent seasonal variability in concentrations (i.e., variability might be more abrupt or not cyclic).

PCE chain concentration data for two wells (Figure 2.7) are shown on Figure 2.8 and Figure 2.9 as an example of data that shows no increasing or decreasing trend.

Steady (i.e., 1986 to 2003) concentrations also suggest VOC sources are likely constant, possibly indicating the presence of NAPL sources or back diffusion from stagnant zones. If sources do not have NAPL, or source volumes were small, a greater number of sample locations would probably exhibit decreasing concentrations with time.

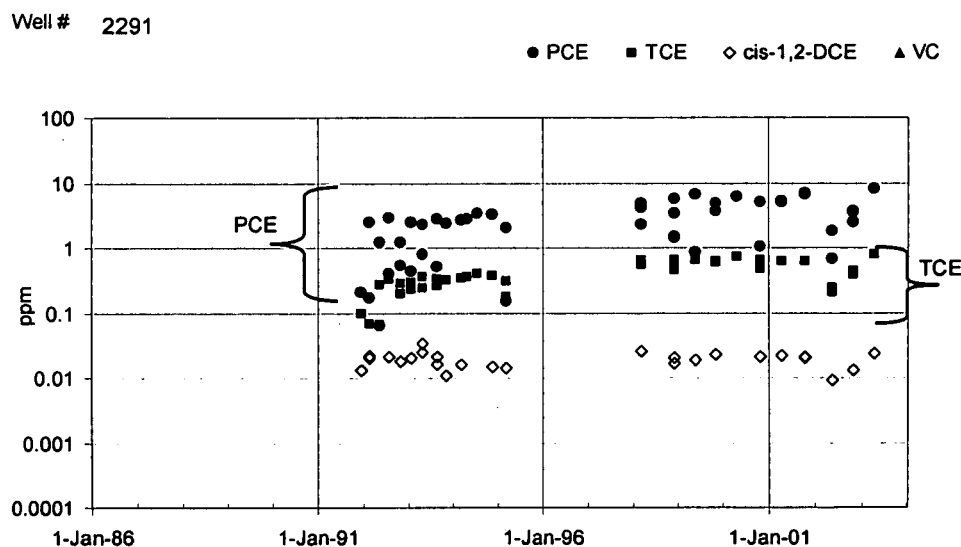


Figure 2.6. Concentration plot showing variability in sample results. Note that results vary about one order of magnitude.



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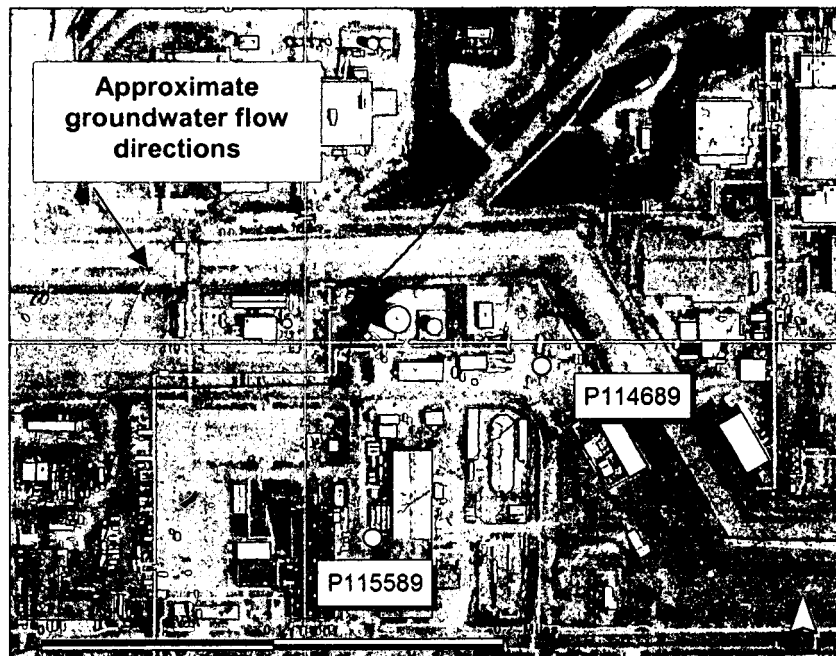


Figure 2.7. Well locations for data shown on Figures 2.9 and 2.10.

Well # P115589

● PCE ■ TCE ◇ cis-1,2-DCE ▲ VC

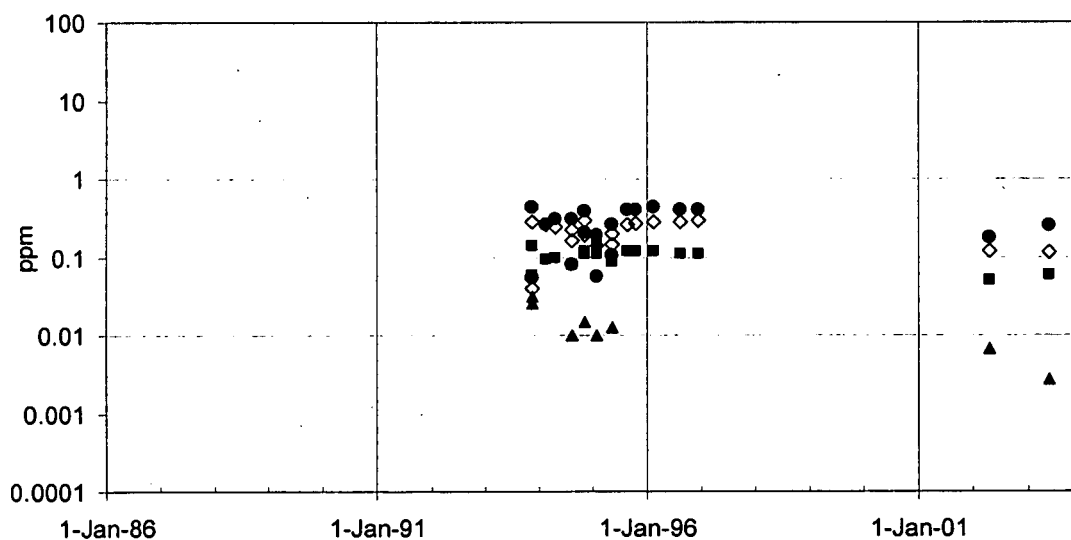


Figure 2.8. Sample concentration data (log scale) for well P115589 (location shown on Figure 2.7).

37

Well # P114689

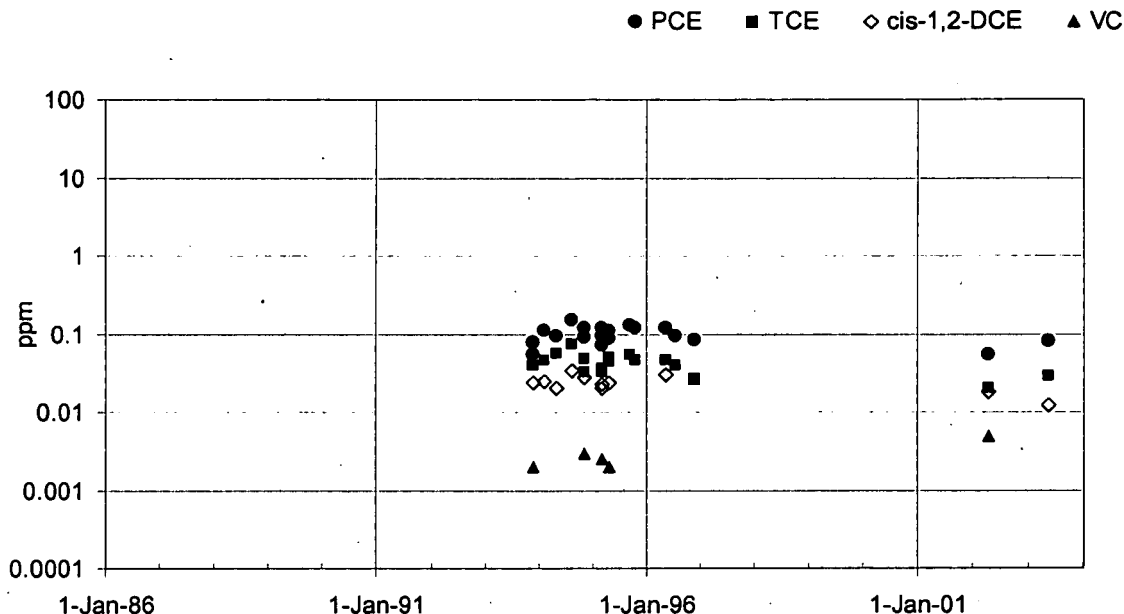


Figure 2.9. Sample concentration data (log scale) for well P114689 (location shown on Figure 2.7).

Well 3586 (located down-gradient of the Mound Groundwater Collection System) is an example of a well that shows a decreasing trend (Figure 2.10 and Figure 2.11). The concentration plot (Figure 2.11) suggests a localized TCE source that is diminishing over time and completely degrading to the daughter products (cis-1,2-DCE and VC). Small amounts of PCE were present with only one sample slightly over the detection limit. This contrasts with other wells [2291 (Figure 2.12) and 897 (Figure 2.13)], which are near and far from the primary source in PSA 5 (Figure 2.10), that show no trends and have a higher concentration of PCE with successively lower concentrations of the daughter products TCE and cis-1,2-DCE.

Sample concentrations with time at most locations show no clear signs of increasing or decreasing beyond typical seasonal sample variability. For example, a sample taken after the wet season might be diluted by increased recharge, thereby giving a lower result than a sample taken during the dry season. Also, fluctuating groundwater levels can also produce variability in concentrations observed in sample locations, down-gradient from source locations. A cumulative density plot shown on Figure 2.14 summarizes the difference between maximum and minimum log concentrations for all VOC samples considered in this study for CCl<sub>4</sub>, PCE, and TCE. Results confirmed that this difference for greater than 96% of the samples at the Site varies less than two orders of magnitude in time. In other words, most sample

concentrations with time showed little to no clear increasing, or decreasing trends with time.

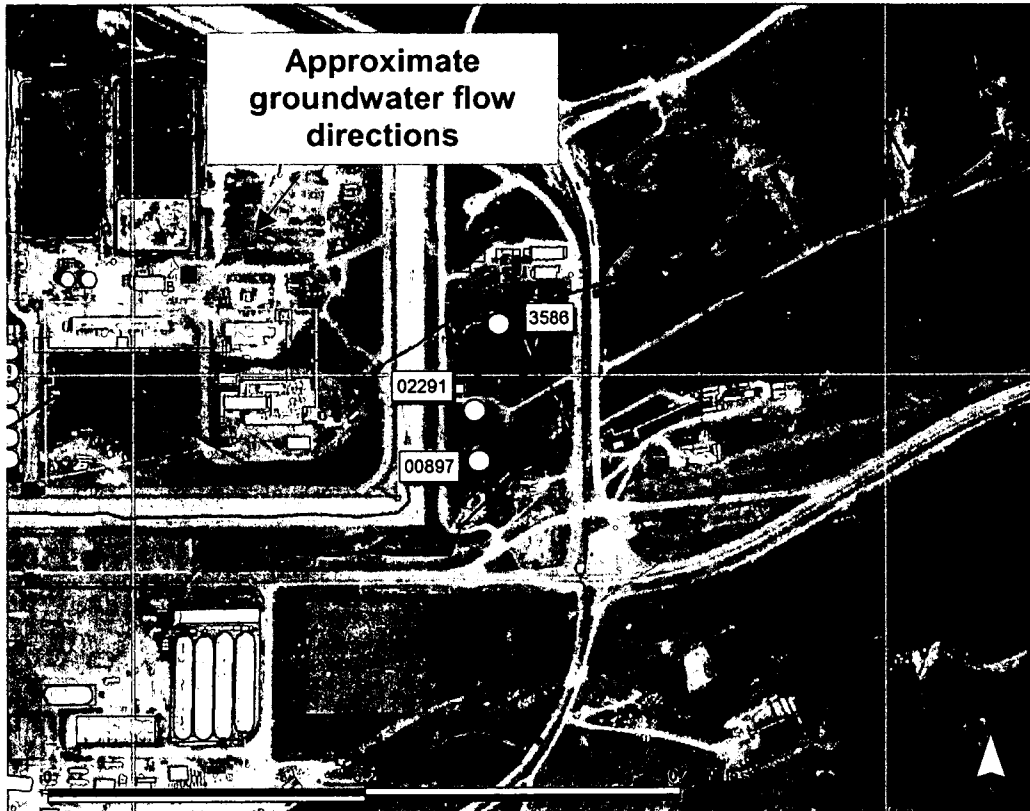


Figure 2.10. Well locations for data shown on Figures 2.11 through 2.13.

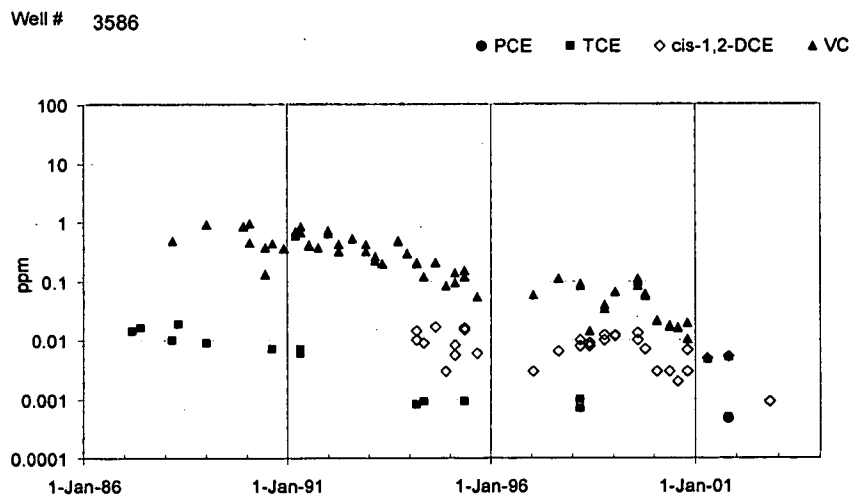


Figure 2.11. Sample concentration data (log scale) for well no. 3586 (location shown on Figure 2.10).

Well # 2291

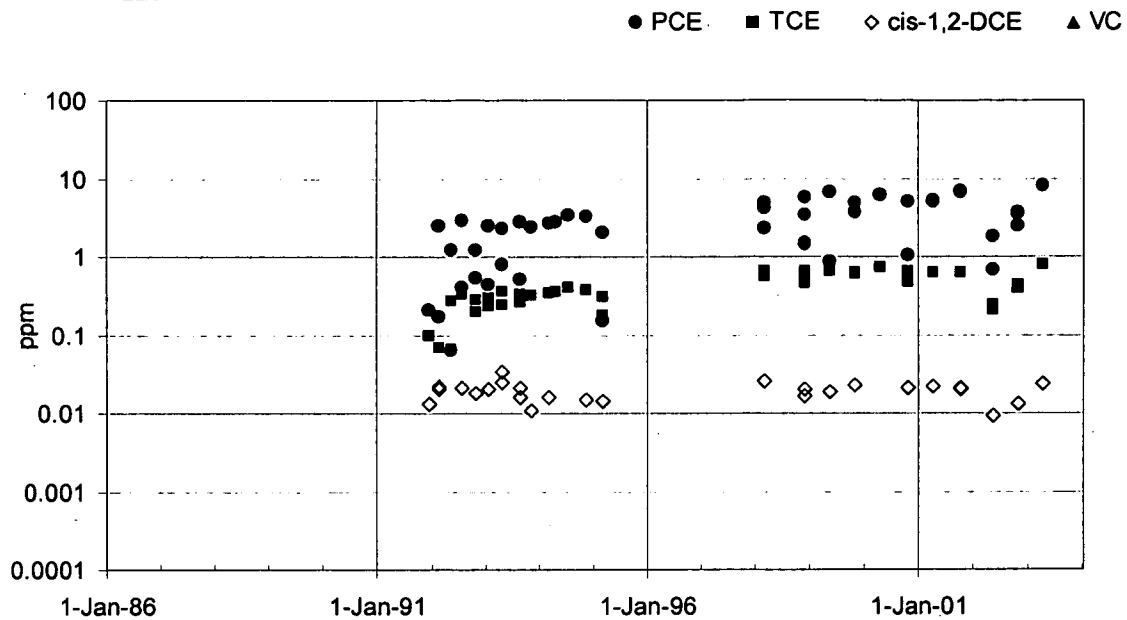


Figure 2.12. Sample concentration data (log scale) for well no. 2291 (location shown on Figure 2.10).

Well # 897

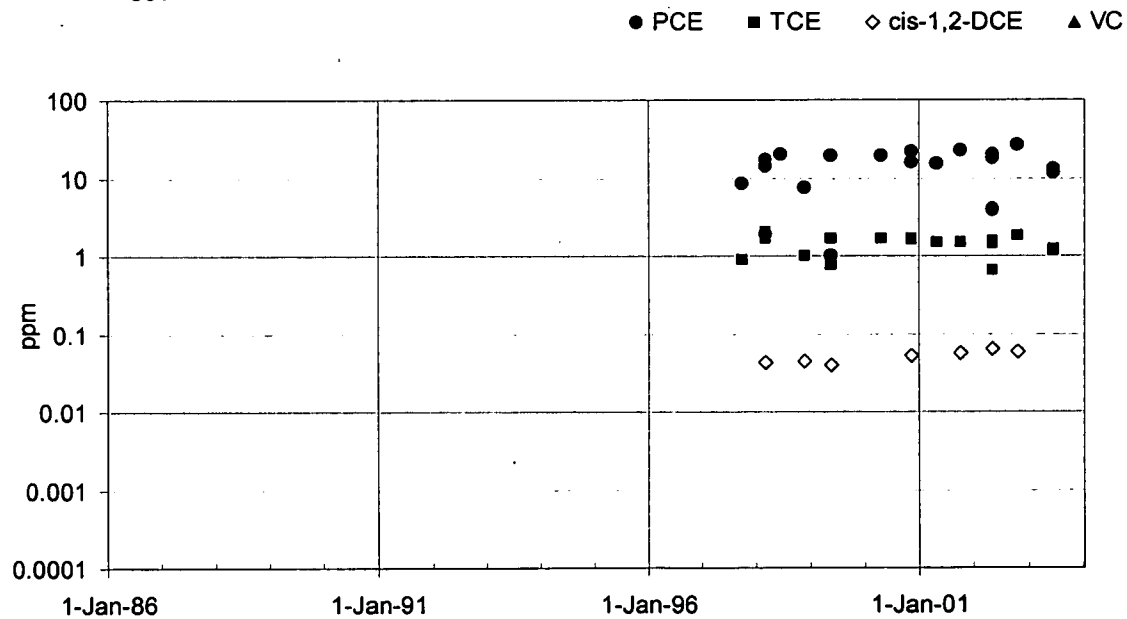
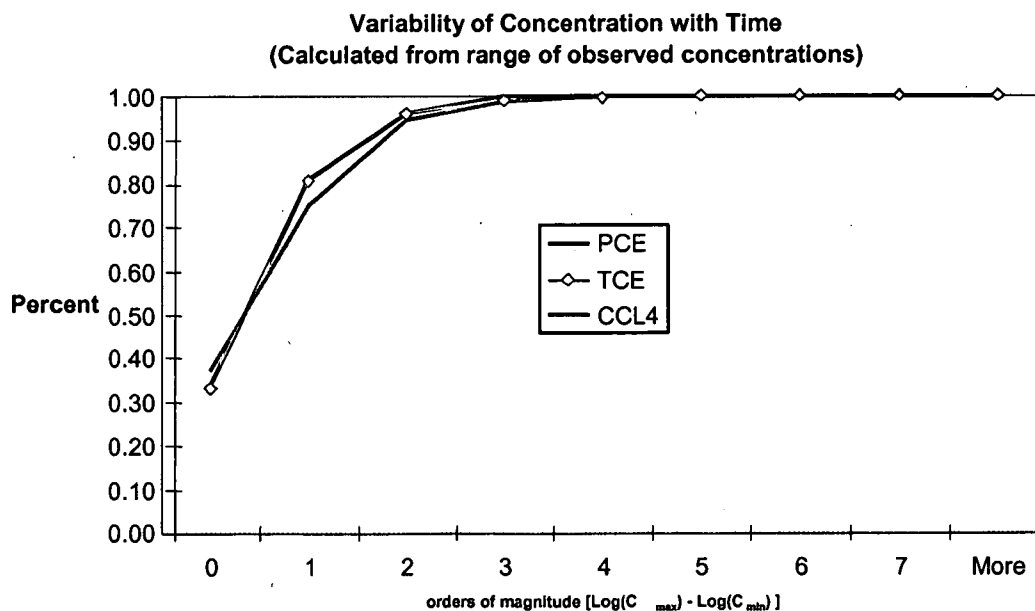


Figure 2.13. Sample concentration data (log scale) for well no. 897 (location shown on Figure 2.10).

Instead of attempting to reproduce the observed variability of concentrations in time at all sample locations, all historical concentration information, including recent 2003 well "snapshot" sampling data, were temporally-averaged. This approach was considered valid given the lack of clear increasing or decreasing trends in time (Figure 2.14) for most samples. An additional benefit to using time-averaged concentrations is that considerably more sample locations are available than would have been using only current "snapshot" data. This information is important because it is then used to constrain plume areas (described in Section 3.0), and parameter values in the reactive transport modeling discussed in Section 6.0.

Figure 2.15 shows the number of samples detected in time for TCE and associated degradation products. Results clearly demonstrate that individual years of data vary significantly (i.e., more than 100%). This is largely due to different historical sampling objectives. Moreover, it is important to recognize that the same wells sampled, for example, in 2003 were not, in many instances, the same wells sampled in earlier, like in 1995. Clearly, using data from only one year did not provide as clear a picture of where VOCs exist in groundwater as using all historical information. Finally, once VOCs were detected in a sample for the first time, subsequent sampling events typically showed detectable levels. This observation, along with the apparent steady concentrations, support using average time-averaged concentrations for delineating plume areas, and as performance criteria for later reactive transport modeling (see Section 6.0).



**Figure 2.14. Cumulative plot of the number of sample locations whose concentration range falls within one or more orders of magnitude.**

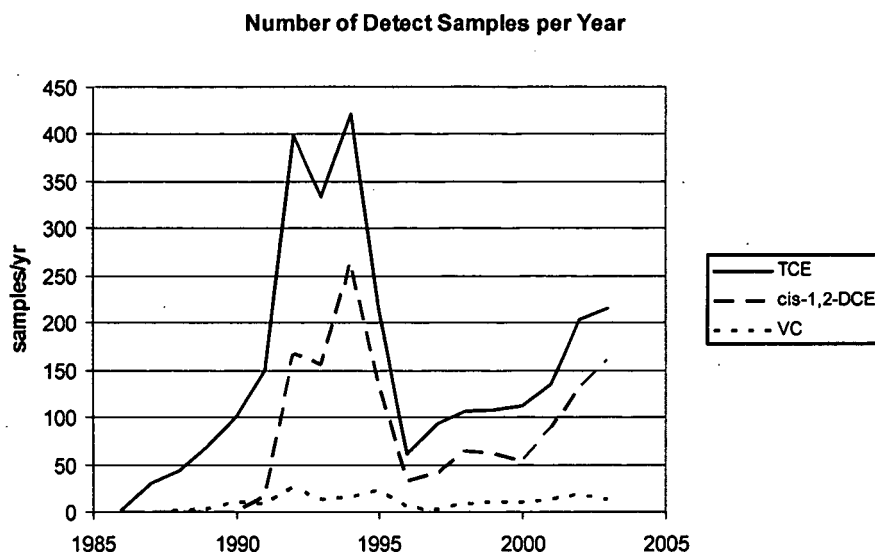


Figure 2.15. Plot of detected VOCs within the study area over the sampling time range.

### 2.3.5 Modeled VOCs

For each of the 10 VOCs considered (Table 2.1) maximum historical concentrations at all sampling locations (within each PSA) were used as the basis for determining whether individual PSAs fate and transport were evaluated further with modeling. Maximum historical concentrations in time were compared against proposed draft surface water PRGs (Table 2.2). Only PSAs in which maximum historical VOC concentrations were higher than draft surface water PRGs were modeled. If maximum historical concentrations were above draft surface water PRGs, it was assumed that inferred source area groundwater was capable of producing groundwater VOC concentrations in groundwater discharge areas that were also greater than the PRGs. As a result, if no historical maximum groundwater sample concentrations were above PRGs, they were not modeled.

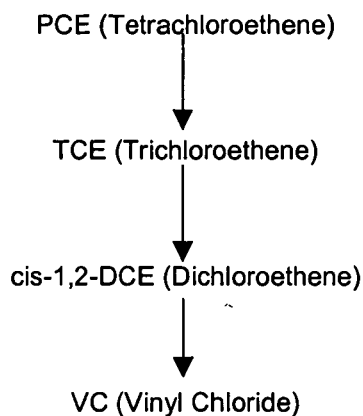
Although the criteria used here reduced the total number of PSAs modeled, it was considered reasonable. For example, the combined attenuation effects of other processes such as biodegradation, dispersion, sorption and evapotranspiration act to reduce downgradient VOC concentrations before groundwater discharges to surface water. In addition, time-series analysis (Section 2.3.4) also confirmed that most sample location groundwater VOC concentrations are relatively stable in time, suggesting that it is not likely that the mostly decades-old inferred sources will produce higher VOC concentrations at existing sample locations in the future.

Therefore, only the PCE and CCl<sub>4</sub> degradation chains were considered for further fate and transport modeling evaluation. These chains are illustrated on Figure 2.16.

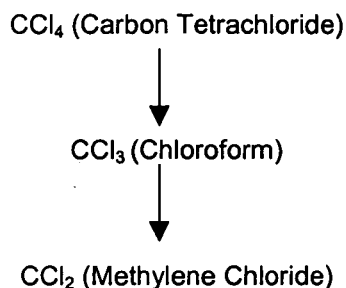
**Table 2.2. Comparison of maximum observed data with PRGs (at hazard quotient = 0.1) (Chromec, 2002).**

VOC	Max Observed Groundwater (mg/L)	Solubility (range of published values, temperature dependent) (mg/L)	Surface Water PRG (mg/L)	Observed above PRG?	Surface Water Standards (mg/L)	Selected for modeling?
PCE	528, 120 (well 0174)	130 – 400	1.46	Yes	0.0008	Yes
TCE	220 (well 3687,7391)	1000-1100	0.19	Yes	0.0027	Yes
Cis-1,2-DCE	2.9 (well 8891)	3500	18.25	No	0.07	Yes
VC	1.2 (well 33502)	1100-2790	0.105	Yes	0.002	Yes
CCl <sub>4</sub>	100 (well 6691)	800	0.58	Yes	0.00025	Yes
Chloroform	64 (well 6691)	8000	20.3	Yes	0.0057	Yes
Methylene Chloride	43 (well 3687)	13000	10.1	Yes	0.0047	Yes
1,1,1-TCA	20	950 – 1500	568	No	0.2	No
1,1-DCA	5	5000	203	No	3.6	No
1,1-DCE	48 (well 0974)	2500	0.13	Yes	NA	No

**PCE/TCE Degradation Chain**



**Carbon Tetrachloride Degradation Chain**



**Figure 2.16. Degradation chains for PCE and CCl<sub>4</sub>.**

The results of this comparison are shown in Table 2.2. Three of the 10 VOCs (TCA, DCA, and cis-1,2-DCE) did not pass the screening process. Cis-1,2-DCE was modeled even though its maximum concentration is below its surface water PRG, because it degrades to VC (as part of the PCE degradation chain) which was observed above its PRG. One of the chemicals, 1,1-DCE, was eliminated because of its limited occurrence (there are only eight wells with more than one sample greater than its draft surface water PRG) and that it occurs in areas already modeled with the PCE degradation chain. Based on these results, only the chemicals in the PCE and CCl<sub>4</sub> degradation chains were modeled. The time-averaged sample locations and concentration ranges for the seven VOC analytes in the PCE and CCl<sub>4</sub> degradation chains are shown spatially in Appendix B, Figures B-1 through B-7.



### 3.0 CHARACTERIZATION AND CONCEPTUALIZATION OF FLOW AND TRANSPORT

Many factors affect the fate and transport of VOCs in groundwater at RFETS. The purpose of this section is to describe key factors and how they affect VOC fate and transport.

Key processes controlling plume migration include: advection, degradation, sorption, ET / volatilization, and dispersion. As groundwater flows past a contaminant source, the contaminant dissolves into the groundwater at a rate dependent on the solubility of the chemical. The plume front advances down-gradient primarily by advection from the source. Factors affecting advection are described in Section 3.2. The remaining processes generally attenuate plumes, creating high concentration distributions near the source that diminish away from the source. These processes were systematically evaluated for VOCs throughout the IA and to the east for the first time at RFETS in the context of plumes and sources within a groundwater flow field. In this approach, an attempt was made to associate likely plume areas to possible VOC sources. This is described in more detail in Section 3.3.

#### 3.1 Approach

A description of the VOC plume characteristics and their sources required a detailed spatial and temporal evaluation of all available groundwater observed VOC concentration data coupled with the major factors influencing fate and transport. At RFETS, the fate and transport of VOCs in groundwater within the IA is complex and likely affected by many factors. Past studies and interpretations of available data suggested that the most important factors controlling VOC transport at RFETS include:

- advective transport with groundwater flow, the dominant mass transport process in groundwater flow systems. It is a strong function of groundwater flow directions and velocities that are controlled by the heterogeneity of the formation and influenced by many factors (Section 3.2);
- source locations, depths, release times, and effective concentrations; and
- other fate and transport processes associated with fate and transport of dissolved constituents (e.g., dispersion, biodegradation, and sorption).

Four steps were taken to understand observed distributions of VOCs in groundwater at RFETS. First, the groundwater flow field was determined so that advective transport directions and velocities could be evaluated. Second, areas impacted by VOCs were clearly identified. This step required evaluation of all time-varying groundwater concentration data, mostly from wells throughout the IA. The third step was to identify the most probable sources for the observed

VOC distributions. In this step, HRR information was used to infer groundwater VOC sources. Finally, areas in which groundwater was impacted by VOCs were then associated with these inferred VOC sources (Section 3.3).

Previous work at the Site [Annual Rocky Flats Cleanup Agreement (RFCA) Groundwater Reports] involved plotting the distribution of Tier I and II VOC concentrations in composite VOC maps. This approach does not utilize both groundwater flow paths and inferred VOC source areas in delineating plume concentration distributions and extents. In addition, no attempts were made to define areas with groundwater VOCs down to detection levels. As a result, these composite plume maps could not be used to as the basis for simulating fate and transport from inferred VOC sources.

Previous work did not evaluate the fate and transport of individual VOCs. Differences in chemical properties of individual VOCs cause differences in their fate and transport behavior. Temporal and spatial characteristics of the individual VOCs provided basic information on sources, areas where degradation occurs, degradation rates, and differences in transport (typically resulting from differences in chemical properties of individual VOCs). Finally, observed concentration distributions of individual VOC daughter compounds served as constraints on several parameters in reactive transport modeling.

A step-wise approach was developed to evaluate the spatial distribution of individual VOCs in groundwater using available time-averaged groundwater VOC concentration data. A discussion of areas where groundwater is impacted by VOCs is described in Section 3.3 below. A new term, Plume Signature Areas (PSAs), was introduced to refer to the distinct areas of impacted VOCs as suggested by time-averaged historical observations. A PSA represents a single plume or a number of plumes associated with a single source or a combination of sources in the same vicinity. General characteristics of PSAs are summarized in Section 3.3.2, while characteristics of VOC sources are described in Section 3.3.3.

### **3.2 Factors Affecting Groundwater Flow**

Advective transport of contaminants with the groundwater is usually the most important factor in VOC transport modeling. The groundwater flow field must be accurately simulated to effectively model the transport of VOCs to discharge areas. The conceptual model developed for the integrated SWWB flow model (Kaiser-Hill, 2002) was adequate for explaining most factors that affect flow within the IA, but not those affecting VOC fate and transport. This section focuses on the factors that affect transport flow paths and velocities under current conditions and under the proposed closure configuration. Key factors include the following:

- subsurface pipelines;

- subsurface utilities and utility corridors;
- subsurface structures;
- groundwater collection systems;
- unconsolidated material types and their distribution;
- weathered bedrock surface;
- unweathered bedrock surface;
- Arapahoe Sandstone distribution;
- impervious areas (buildings/pavement); and
- vegetation distribution.

These factors are described in detail below.

### **Subsurface pipelines**

Subsurface pipelines occur primarily in the more industrialized parts of the IA. They remove groundwater from the local areas, with the exception of the water supply pipelines. Subsurface pipelines are more important in areas of higher pipe densities (i.e., western IA and Central Avenue) and less important in lower density pipe areas, like the 903 Pad area. For example, flow modeling in the Building 771 area showed that subsurface drains impacted the local groundwater flow regime and likely the contaminant distribution observed in the area. Proposed deactivation of these subsurface pipelines will cause a local increase in water levels, which may change local VOC concentrations.

### **Subsurface Utilities**

Backfill material associated with subsurface utilities may also influence local flow and hence advective transport. The backfill material (whether the excavated material or coarse sand/gravel) at the base and around utilities is believed to be more permeable than surrounding unconsolidated or bedrock material. As such, if water levels rise into this material, they can behave as preferential pathways for flow and VOC transport. The quantity and locations of available wells near known utility corridors was insufficient to clearly demonstrate their effects on transport.

### **Building Footing Drains**

Building footing drains currently act as strong hydraulic controls around deeper building basement structures. However, in the proposed closure simulations, footing drains were assumed inactive. As a result, subsurface building basement

structures may act as barriers to flow, rather than as local groundwater discharge points. Local groundwater VOC concentrations may change as a consequence of the deactivation of footing drains, for example, around Building 771.

### **Groundwater Collection Systems**

The current gravity drainage systems were designed to collect contaminated groundwater in their vicinity. The proposed land reconfiguration modification will not likely impact the operation of these systems. No new systems were considered in this modeling effort.

### **Unconsolidated Material Distribution**

Unconsolidated materials within the Integrated VOC Flow Model area are predominantly Rocky Flats Alluvium (Qrf), Artificial Fill (af), and to a lesser extent, colluvium, and landslide deposits (Qc and Qls, respectively). Little information was available on af properties, but it is assumed that af is roughly similar to Qrf (with possible screening of larger rocks – this would result in possible higher storage). Closure conditions assumed that the af material is used in re-graded topographic areas (see Section 6.1.4). The hydraulic properties of these materials strongly control groundwater velocities and pathways. The distribution of surface soils also controls the spatial and temporal distribution of recharge and ET throughout the integrated VOC Flow Model area.

The seasonal fluctuation of groundwater within the unconsolidated material is another important factor that strongly controls the transport of VOCs. The higher hydraulic conductivity of the unconsolidated material relative to the underlying weathered bedrock allows groundwater to flow more rapidly in the unconsolidated material. However, in areas where the groundwater level fluctuates above and below the unconsolidated material and weathered bedrock interface, average groundwater flow velocities and paths become complex. At times when the high conductivity unconsolidated material is saturated, flow and transport are relatively rapid. However, when the unconsolidated material is unsaturated, flow is restricted to the lower conductivity weathered bedrock. This condition of variable saturation occurs mainly in hillslope areas, particularly to the north, east, and south of the IA (within the integrated VOC Flow Model domain).

### **Depths to Weathered Bedrock and Unweathered Bedrock**

Groundwater flow paths are strongly influenced by the conductivity of the different geologic units at the Site. Accurately estimating locating the contacts between the unconsolidated material, weathered bedrock, and unweathered bedrock was essential to developing an accurate flow model layers. Simulations of groundwater flow direction are directly dependent on the accuracy of the mapped units because of the contrast in hydraulic conductivity values for the three units. Maps of the depth to weathered bedrock and the depth to unweathered bedrock are shown on Figure 2.2 and Figure 2.3.

The weathered bedrock (claystone/siltstone) hydraulic conductivity is from about 10 to 300 times lower than the overlying unconsolidated material. Because of its relatively high conductivity, the majority of flow occurs within the unconsolidated material when it is saturated. In some locations (such as the IA plumes near Building 771, Building 991, and the 903 Pad) the unconsolidated material is less than 5 feet thick and on average, unsaturated. This occurs in the east-central part of the IA, where groundwater flows to the east from the 903 Pad area in a relatively thick, saturated zone of unconsolidated material along the topographic high before diverging down-gradient to the north and south.

The interpolated weathered bedrock surface was prepared for the SWWB model (Kaiser-Hill, 2002). The 903 Pad area flow divergence is consistent with the bedrock surface, and it is also influenced by the Arapahoe Sandstone in the area.

### **Arapahoe Sandstone Distribution**

The shallow distribution of Arapahoe Sandstone within the UHSU has a strong influence on local groundwater flow paths and velocities. The modeled distribution of sandstone is shown on Figure 3.1, while the areas that subcrop the unconsolidated material are shown on Figure 3.2. The dark areas on Figure 3.2 represent areas where claystone/siltstone weathered bedrock material overlies the Arapahoe Sandstone (Arapahoe Sandstone does not subcrop the unconsolidated material). The discontinuous sandstone occurs in several areas impacted by VOCs, specifically the 903 Pad area, central IA, former western Solar Ponds area, the Mound Groundwater Collection System area, and Building 771. Its high permeability, compared to the surrounding weathered bedrock siltstone/claystone matrix, causes groundwater to preferentially flow towards it, and within it. The degree to which these act as preferential conduits depends on its discharge at seeps. The presence of numerous seeps throughout the Site is attributed to its subcropping beneath thin unconsolidated material along hillslopes (EG&G, 1995a). Discharge at seeps cause the shallow sandstone lenses to dewater, which in turn causes local groundwater to flow towards and within the sandstone.

The occurrence and configuration of the Arapahoe Sandstone within the UHSU weathered bedrock also impacts the local fate and transport of VOCs in groundwater. Although the Arapahoe Sandstone occurs within each of the primary VOC plume areas, it subcrops (occurs in the upper weathered bedrock) near the Building 771, Building 991, Mound System/Oil Burn Pit, East Trenches, and former Solar Pond areas (Figure 3.1 and 3.2). VOC sources introduced in areas where Arapahoe Sandstone subcrops (e.g., the East Trenches) probably results in deeper sources (i.e., at base of Sandstone) because the permeability of the sandstone is similar to the unconsolidated material. In addition, the distribution of Arapahoe Sandstone, where continuous, causes preferential pathways for VOCs, because the sandstone permeabilities are much higher than the surrounding UHSU weathered bedrock (claystone/siltstone) material.

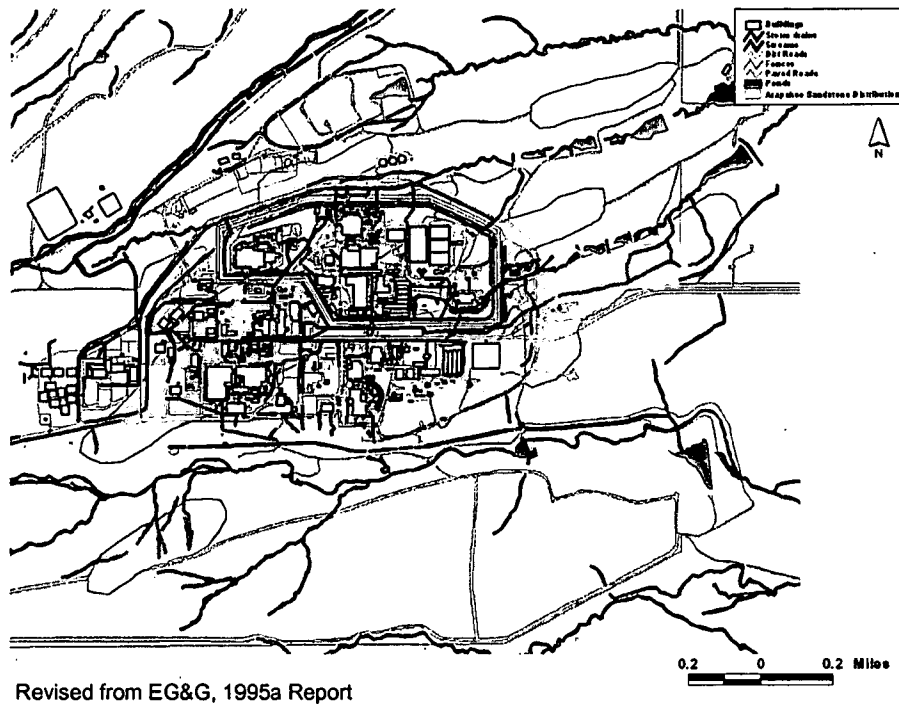


Figure 3.1. Distribution of the Arapahoe Sandstone.

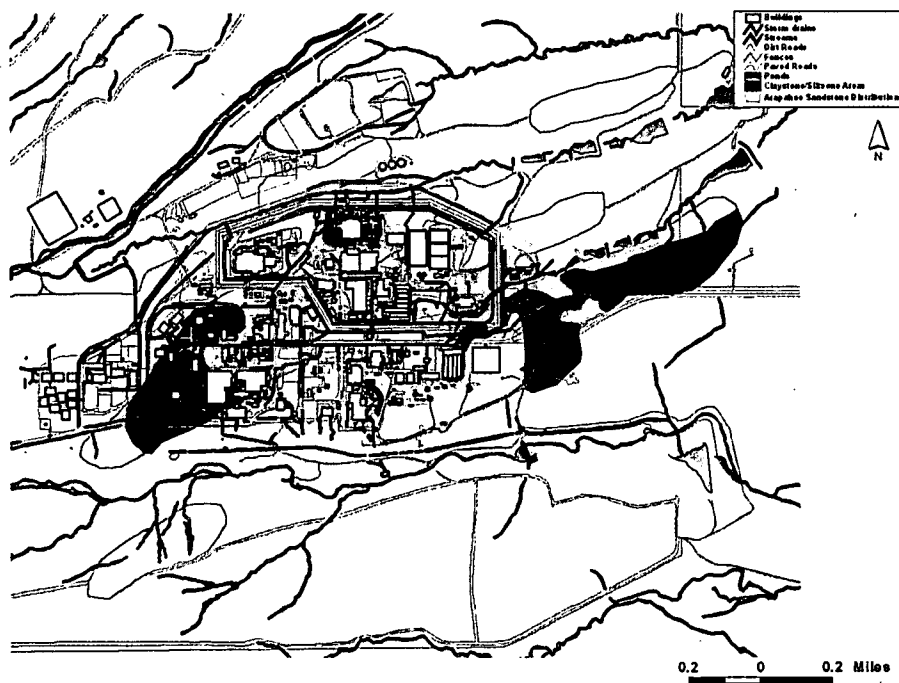


Figure 3.2. Subcropping Arapahoe Sandstone areas.

## **Impervious Areas**

Impervious areas are important under current conditions because they prevent direct recharge, but promote much higher localized recharge where runoff occurs in adjacent pervious areas and limit direct ET losses. This is an important aspect of the integrated model in predicting accurate groundwater flow paths. The seasonality of this recharge process likely causes significant local divergence of flows from "recharge mounds" at the edge of impervious areas, which may be one cause of the macro-dispersion of plumes.

## **Vegetation**

Vegetation distributions within the IA control local ET rates and hence the net recharge to a given area. Near streams, ET is the dominant groundwater discharge mechanism. The SWWB modeling estimated that more than 90% of the annual precipitation is lost to ET. Additionally, the rate of ET loss increases as groundwater depths decrease or as you approach stream areas. The loss of VOCs to ET may be a dominant process at RFETS. This process alone may limit the migration of VOCs to adjacent stream areas as baseflow. In arid/semi-arid areas like RFETS, ET strongly controls the water levels at stream areas, and if it is strong enough, no baseflow ever occurs.

## **Faults**

Geologic fault lineaments across the Site have only been inferred and not specifically studied or mapped (EG&G, 1995). With the currently available information, these faults do not appear to influence groundwater flow or VOC transport at the Site.

### **3.3 Plume and Source Characteristics**

Developing a good understanding of the basic characteristics of dissolved VOC distributions and their possible sources was essential to constructing a realistic and acceptable flow and transport model of the IA at RFETS. The extent to which this can be done depends largely on the quality and quantity of available data. At RFETS, although source data and time-averaged concentrations are inherently uncertain, it is still possible to prepare meaningful interpretations by synthesizing information from available data. Several steps taken to describe the basic characteristics of recent VOC distributions and their sources are discussed below.

#### **3.3.1 Plume Signature Areas**

PSAs are defined as distinctly separate areas where groundwater is impacted by VOCs. Within these areas, there is at least one probable VOC source, though in most cases there are several. In many cases, multiple source locations (or a single source location with multiple source releases in time) produce co-mingling

groundwater plumes with complex concentration distributions that are generally indistinguishable from each other with available groundwater well water quality information. The PSA shape, extent, and concentration distributions are developed using available groundwater flow paths, groundwater VOC concentrations (from wells), and HRR information.

PSAs were developed for several purposes summarized below:

- clarification of the spatial distribution of VOC-impacted groundwater within the IA;
- evaluation of parent-daughter VOC spatial relationships;
- identification of individual VOC distribution trends;
- evaluation of individual VOC distributions in groundwater given different chemical properties (different chemical properties allow each VOC to travel and degrade differently in groundwater);
- identification of possible locations where dissolved VOCs may have been introduced from entrapped NAPL or back diffusion from stagnant zones (referred to as source locations);
- understanding of constraints on flow and transport model parameterization using PSA concentration distribution and extents;
- temporal analysis of concentration trends within specific PSAs emphasizing basic source concentration characteristics. For example, constant concentration trends in time in all observation wells within a given PSA suggest that source concentrations are likely to remain constant; and
- development of initial concentration distributions in closure configuration simulations to predict a range of maximum groundwater discharge concentrations.

PSAs were developed for the 10 most spatially frequent VOCs detected in groundwater throughout the IA using the GIS-database described in Section 2.3.3. Of these VOCs, only seven remained after screening with draft surface water PRGs (Section 2.3.5). The seven remaining VOCs represent the degradation chains for PCE and CCl<sub>4</sub>, suggesting that these VOCs are likely the most common VOCs in source areas.

The extent of each PSA was defined using plots of historical time-averaged groundwater sample concentrations (Appendix B, Figures 1-7) in conjunction with available groundwater level information and inferred groundwater flow directions (Section 2). The delineation of a PSA is based on the following: (1) a continuous area with average concentrations at or above 10 ug/L and with at least one sample location with an average concentration above the draft surface water PRG; (2) a potential source location; and (3) groundwater flow directions based



on integrated modeling of WY2000 data. Figure 3.3 and Figure 3.4 show boundaries only for PSAs that are modeled. PSAs in which historical maximum VOC concentrations are less than the surface water PRGs (included on Figures 1 to 7 in Appendix B) are not modeled.

Initially, an attempt was made to differentiate areas with detectable levels of VOCs from areas with no detectable levels for any sampling event. In this step, major PSA areas were identified. In some PSAs, it was difficult to associate observed VOC concentrations at some data locations because of plume commingling of multiple sources. More careful evaluation of historical releases, discussions with SMEs, and spatial evaluation of concentration distributions within preliminary PSAs allowed for further refinement of individual shapes and likely source areas. In developing the PSAs, a GIS database was constructed that also supported transport modeling (average and maximum concentrations, number of data points, potential source locations, estimated travel distances, and estimated distances from groundwater discharge areas). Significant data gaps and uncertainty were also noted in preparing the PSA maps.

Although PSAs represent the best understanding of VOC groundwater distributions, external boundaries were based on professional judgment. As such, the spatial shape and extent of PSAs are only used to evaluate the spatial characteristics VOC distributions within the IA. Ultimately, flow and transport modeling was used to refine the extents and pathways for VOCs. In the transport modeling discussed later in Section 6.0, only the concentration data at observed locations are used to constrain calibration of flow and transport parameter values. General PSA characteristics are discussed next in Section 3.3.2.

### **3.3.2 PSA Characteristics**

The data used to develop PSA boundaries provides significant conceptual insight into VOC contamination of groundwater at the Site. General characteristics of the individual VOC PSAs are described here.

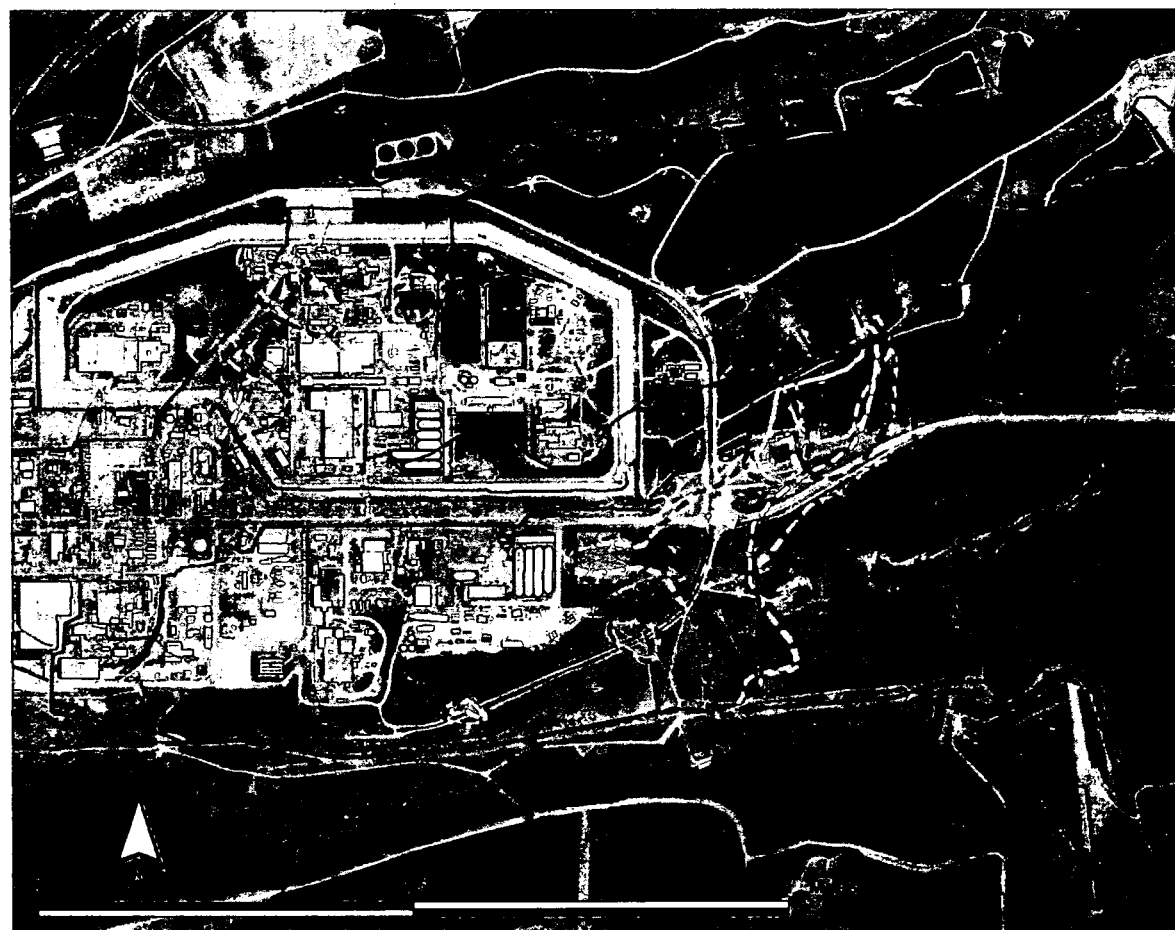
Biodegradation is likely, given consistently observed daughter product distributions, and appears to occur through most of the PCE and CCl<sub>4</sub> chains. All PSAs with PCE signatures contain TCE and DCE, and most contain VC. As further evidence of biodegradation, all of the PSAs being evaluated contain cis-1,2-DCE (considered to be an indicator compound for biodegradation). In some PSAs, degradation to lower chain compounds is limited. For example, VC did not occur in all areas with detectable levels of PCE or TCE. This suggests that local conditions may not be anaerobic enough to fully degrade parent compounds to lower VOC compounds. Alternatively, conditions may be highly aerobic, causing the VC to be completely degraded from the system.



#### Explanation

- PCE-Chain Approx. PSA Boundaries (10 ug/L)
- TCE-Chain Approx. PSA Boundaries (10 ug/L)
- cis-DCE Approx. PSA Boundaries (10 ug/L)
- VC Approx. PSA Boundaries (10 ug/L)
- GW Flow Directions
- Remediation System
- Buildings
- Streams (pre-closure)
- Ponds

Figure 3.3. PCE/TCE Chain. General PSA locations are based on the presence of analytes at or above the draft surface water PRG. Boundaries are based on a conservative value of 10 ug/L time-averaged well concentration. VC is present in most PSA areas, but is not displayed because of its limited spatial extent.



#### Explanation

- CCI4-Chain Approx. PSA Boundaries (10 ug/L)
- CCI3 - Approximate PSA Boundaries
- CCI2 - Approximate PSA Boundaries
- GW Flow Directions
- Remediation System
- Buildings
- Streams (pre-closure)
- Ponds

Figure 3.4. CCI<sub>4</sub> Chain. General PSA locations are based on the presence of analytes at or above the draft surface water PRG. Boundaries are based on a conservative value of 10 ug/L time-averaged well concentration.

Areas identified as PSAs are generally reasonable given groundwater flow paths, concentration distribution, historical release information, and probable groundwater discharge areas. Detectable levels of VOCs are found mostly within unconsolidated material, though this may be somewhat biased by less sampling done within the weathered bedrock. Concentrations increase in the bedrock relative to the unconsolidated material within some areas of the Site (East Trenches area), probably due to the occurrence of Arapahoe Sandstone.

Most PSAs appear to extend to groundwater discharge areas (i.e., streams, seeps, or ponds). Concentrations are generally low in these areas (<100 ug/L). The lack of discernable changes in concentration trends with time suggests increasing ET losses as groundwater nears stream areas may limit its movement. Stream VOC data is generally lacking to confirm this point.

Groundwater VOC distributions do not appear to be strongly affected by subsurface drains. For example, if drains were to affect transport significantly, concentration distributions would likely reflect linear trending high-density subsurface drain features. PSA shapes and concentration distributions do not clearly show this trend. Though, in some instances such as building footing drains, VOCs have clearly been detected. Moreover, it is difficult to fully assess this with limited well resolution around these features. Subsurface utility trench effects on the distribution of VOCs are also difficult to assess with the resolution of available well data. However, no strong correlations are evident between high-density subsurface utility corridors and PSA distributions.

VOC source areas occur mostly within upper, mesa areas where Rocky Flats Alluvium is prevalent. In most source areas, multiple VOCs are present. This implies that source releases involved multiple VOCs, rather than a single VOC parent product. HRR information also supports this finding. For example, VOC daughter products were probably introduced as the primary source, though this is less likely for degradation products, such as cis-1,2-DCE.

Individual PSA extents and their likely source locations suggest that VOCs have traveled similar distances. This implies several things:

- groundwater VOCs were introduced at similar times;
- hydraulic properties for hydrogeologic units (unconsolidated material and weathered bedrock) are similar; and
- similar attenuation processes act on the plumes.

Differences between individual VOC PSA extents did not appear significant. This suggests that differences in the transport of different VOCs due to their chemical properties may be less important than advective transport (i.e., groundwater flow paths and velocities).

Sampling result information for the VOCs is presented in Table 3.1. All VOCs except VC was detected in more than 150 wells with PCE and TCE detected in 327 of 621 wells.

Most well records did not indicate a clear increasing or decreasing concentrations in time. While concentrations vary in these "no-trend" wells, this variability is likely due to seasonality, or slow system response to periodic recharge events.

**Table 3.1. Number of wells with at least one sample above detection limit for analytes of interest. All listed analytes were detected in all PSAs modeled.**

VOC	Number of Wells Showing Detection	Number of Wells Sampled
PCE	327	621
TCE	327	621
Cis-1,2-DCE	201	557
VC	44	620
1,1-DCE	152	621
CCl <sub>4</sub>	218	621
CCL <sub>3</sub>	257	621
Methylene Chloride	265	619

### 3.3.3 VOC Source Characteristics

The GIS database of HRR releases (Appendix A) contains 363 entries. Of these potential sources, 15% are identified as Priority 1 releases, 70% are identified as Priority 2 releases, and 15 % are identified as Priority 3 releases. As discussed in Section 2.3.1, most solvent releases were only indicated generally, without identifying the specific type or the composition of spilled solvent or mixtures. In the few cases where specific solvents were noted, the following six VOCs were identified: PCE, TCE, cis-1,2-DCE, CCl<sub>4</sub>, Chloroform, and Methylene Chloride (six of the seven VOCs being considered for modeling).

HRR release information did not provide information on the composition of VOCs. In most cases, release volume, spill area (source configuration), and duration are uncertain and therefore had to be evaluated through modeling.

For each of the PSAs which passed the screening process, the number of potential VOCs sources within or adjacent to the PSA area varied significantly (from 0 to 15). Priority 1 and Priority 2 release locations are shown previously on Figure 2.8. The source release time periods in the database range from 1951 to 1998. The database indicates that most of the larger releases occurred in the earlier years of Site operation.

### **3.4 Data Gaps**

Thirteen wells were added to address data gaps identified following initial delineation of individual VOC PSAs (Figure 3.5). Four of the wells are located in the Ryan's Pit area southeast of the 903 Pad. Two wells are located west of the Mound area and were used to sample near the former Oil Burn Pit. One well is located in the East Trenches area down-gradient of a potential source with no other nearby control. Three wells were located in the PSA 13 area and were used to assess the presence of VC in this area.

Not all of the sampling information obtained from these new wells was used to constrain modeling results. However, where available, results of sampling were used to improve conceptual flow and transport and subsequent modeling efforts. Specifically, information from the Mound Groundwater Collection System area and south of Building 371 were used in this study.

### **3.5 Flow and Transport Conceptual Model**

The conceptual flow and VOC transport model is described in this section. It was developed using information derived from the characterization of groundwater flow (Section 3.1) and plume and source distributions (Section 3.2). Conceptualizing the flow and transport emphasized the essential features and processes that must be incorporated into the numerical model. Key features and processes that influence the fate and transport of VOCs to groundwater discharge areas are graphically illustrated on Figure 3.6. A brief description of each process and feature is described in terms of how it impacts VOC transport.

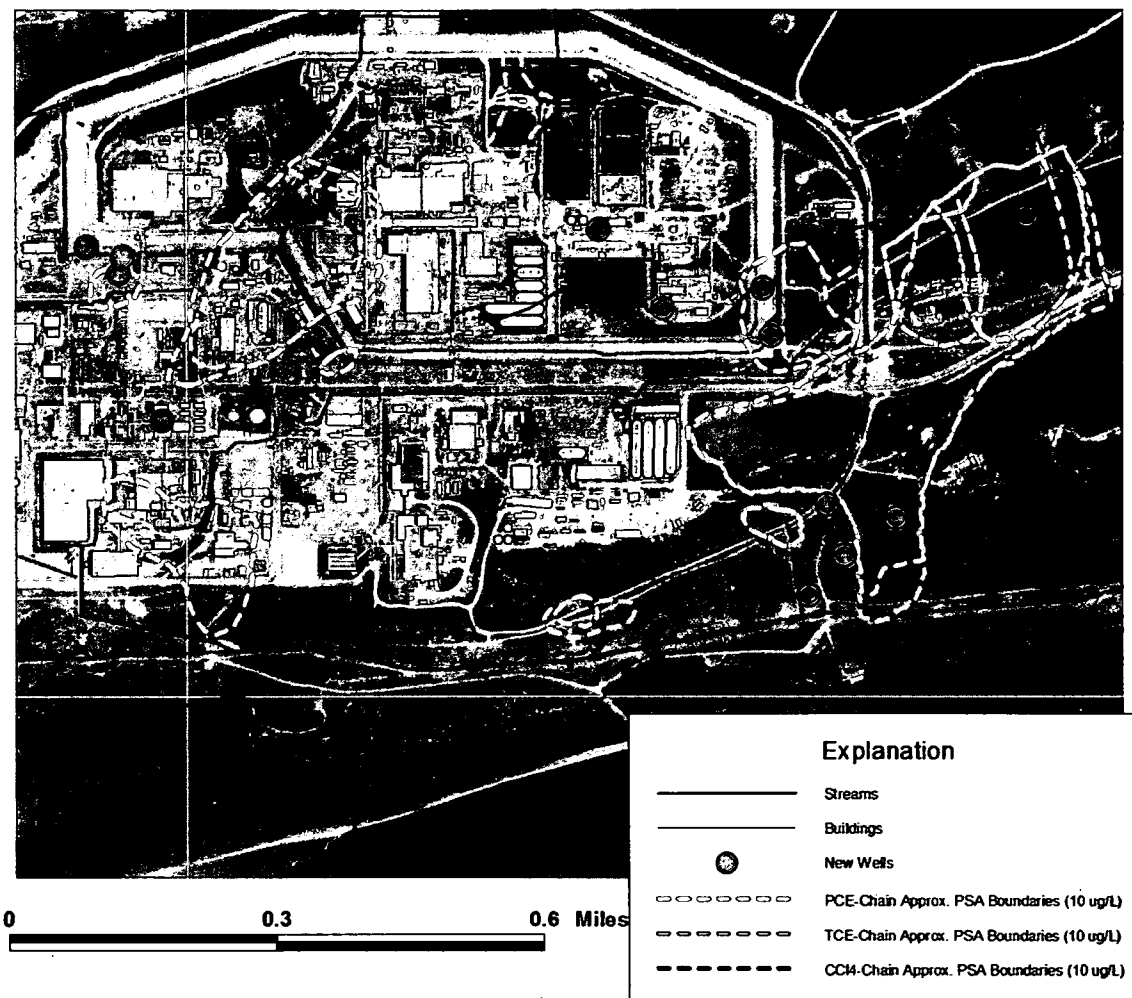


Figure 3.5. New wells installed to address data gaps.

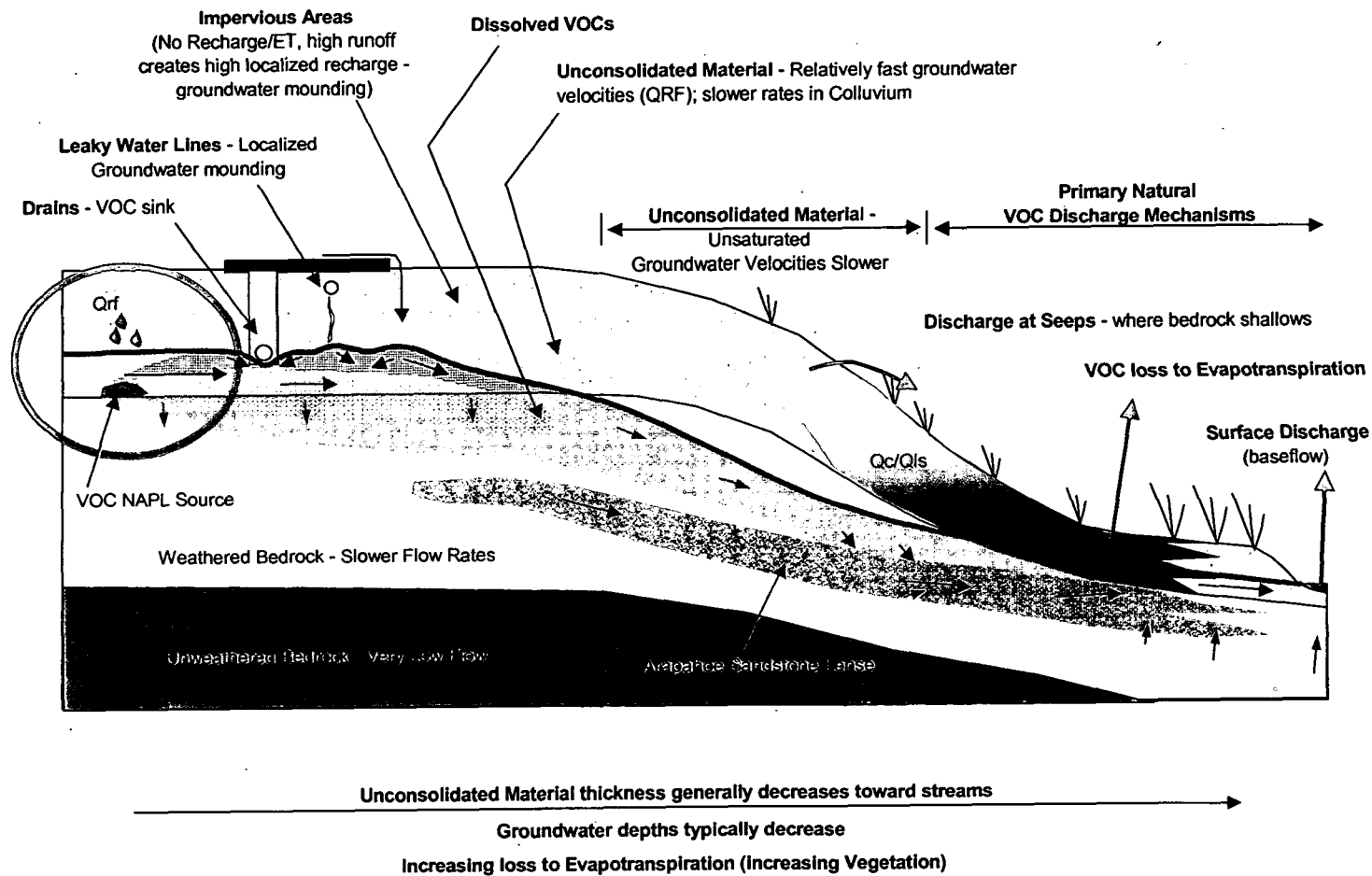


Figure 3.6. Conceptual flow and transport model.



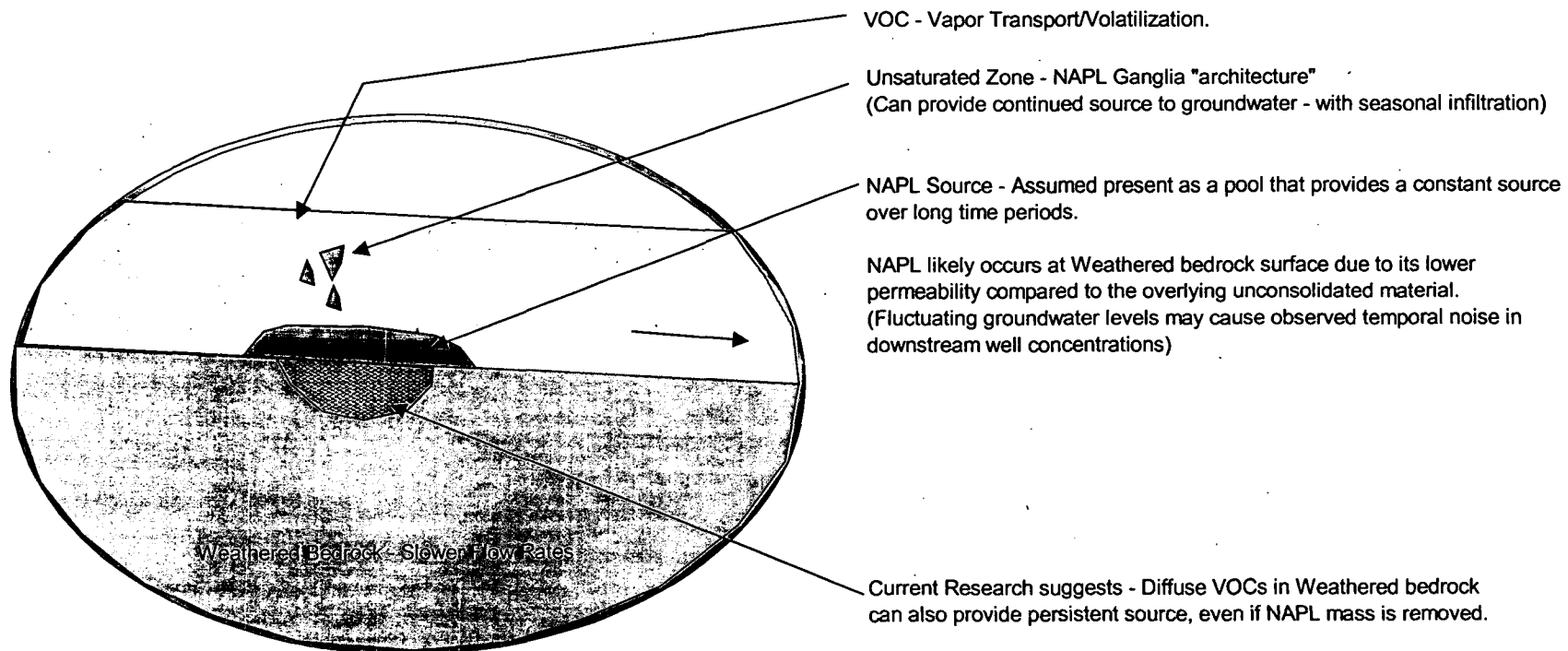


Figure 3.6. (continued) NAPL conceptual diagram.

The conceptualization of groundwater flow was extensively documented in the SWWB modeling (Kaiser-Hill, 2002). Key Site features and subsurface hydrogeologic factors affecting flows were described in detail in Section 3.1. Most of these features are described on Figure 3.6. Groundwater flow within the IA at RFETS is dominated by the hillslope structure. Direct precipitation, infiltration, localized run-on from impervious areas, or leaky water supply lines recharge groundwater throughout the upper mesa areas of the model domain. This generally causes a slight downward vertical gradient from the unconsolidated material into underlying weathered bedrock. Although groundwater flows from upper mesa areas to nearby stream areas, some water is lost via ET even in areas well below the root zone (i.e., caused by unsaturated zone moisture deficits within the root zone).

As groundwater moves within the model domain, it can be affected by subsurface drains in two ways: (1) actual discharge (or recharge) to pipelines like storm or sanitary sewers; or (2) by subsurface utility trenches which may preferentially route water within associated permeable backfill material. As groundwater flows from mesa areas down through hillslope areas, levels can decline into the weathered bedrock due to seasonal fluctuations, thinning unconsolidated material, increased ET effects, and increased lateral drainage (i.e., increased hydraulic gradients). This causes groundwater velocities to decrease due to lower bedrock hydraulic conductivities. In some areas, the presence of Arapahoe Sandstones causes groundwater to flow preferentially within these deposits. As groundwater nears lower hillslope areas, or near-stream areas, vertical gradients typically reverse and are directed upwards. Groundwater is either lost to ET or baseflow, though previous integrated modeling indicates that the majority of loss is via ET along lower hillslope areas where groundwater levels are shallow. Groundwater can also discharge to the surface at surface seeps during higher precipitation periods.

The occurrence of VOCs is probably due to the presence of NAPL caused by either spills, or leaks from pipelines, or buried wastes. NAPL source occurrence in the RFETS hillslope conceptual flow model is described on the second diagram for Figure 3.6. Due to the greater densities of NAPLs compared to groundwater NAPLs migrate vertically downward until reaching a relatively stable pool, typically caused by a lower conductivity material. At RFETS, the weathered bedrock contact likely acts as such a contact, though, locally, greater NAPL source volumes might be driven into the weathered bedrock to greater depths. Recent research also indicates that where high NAPL volumes exist above hydrostratigraphic contacts, the NAPL can migrate into the underlying lower permeability material via diffusion.

As the NAPL travels from the source area, it is also possible that residual amounts remain in the unsaturated zone. As groundwater passes through NAPL areas, dissolved concentrations develop within the groundwater at the NAPL/water interface. Seasonal fluctuations in groundwater levels can increase the flux of dissolved VOCs passing through NAPL source zones. In addition,

groundwater passing through residual NAPL zones within the unsaturated zone may also provide a persistent source of VOCs into the groundwater.

Once VOCs are dissolved in the groundwater, they become subject to different fate and transport processes. These processes include advective transport, degradation, adsorption, dispersion, diffusion, groundwater discharge, and plant uptake (or transpiration). Much of the current VOC distributions throughout the IA are due to advective transport. Advection is typically a dominant process controlling VOC distribution and movement in groundwater. This emphasizes the need to accurately reproduce groundwater flow paths and velocities.

VOCs in groundwater are also subject to dispersion effects. The dispersion of VOCs occurs in three dimensions. Typically longitudinal dispersion is greater, along the principal advective pathway. It accounts for differences in hydraulic properties within the primary hydrostratigraphic units, which are assumed homogenous at the cell-scale. In addition, seasonal fluctuations of localized mounding within the IA due to runoff from paved areas probably causes increased macro-dispersion of VOCs.

VOCs can also be adsorbed onto the porous medium through which they travel. This causes concentrations in the groundwater to decrease, though over time, adsorption rates may decline and effectively this process may only retard the transport of higher concentrations from constant sources. Diffusive processes are typically small, at the scale of the VOC plumes. Their effects can become larger relative to dispersive effects in lower velocity areas, like weathered bedrock.

Biodegradation through anaerobic degradation (reductive dechlorination) is yet another way that concentrations of individual VOCs can be reduced as the VOCs in groundwater migrate towards discharge locations. At RFETS, however, available information suggest that biodegradation rates are likely low (Kaiser-Hill, 2004). Despite this, PSA characteristics (Section 3.2.3) strongly suggest that it does occur. Therefore, as parent VOCs (i.e.,  $\text{CCl}_4$ , PCE, or TCE) migrate towards discharge points, their concentrations decrease, while daughter product concentrations tend to increase. Available data suggest that daughter products, such as VC, probably aerobically degrade. This implies it is possible that a VOC chain can entirely degrade before it discharges to the ground surface.

As groundwater nears stream areas, the effect of ET increases dramatically, due to shallower groundwater levels and increased vegetation. It is likely that VOC losses via ET are also significant, though direct evidence is limited (i.e., VOC concentrations within the vegetation canopy have not been measured). Moreover, ET losses are only high during warmer months, implying that groundwater discharge concentrations could be higher during colder months depending on local flow conditions.

#### 4.0 INTEGRATED FLOW MODEL DEVELOPMENT

The development and application of an integrated groundwater-surface water flow model for current and a proposed closure configuration is described in this section. This integrated flow model was developed to simulate local flow conditions in PSA-specific areas, using a smaller model area and more refined grid than the original SWWB model (Kaiser-Hill, 2002). It was only necessary to model the saturated zone flow as accurately as possible to meet objectives as discussed in Section 1.1. This is because the distribution and fate of VOCs in groundwater at RFETS is strongly controlled by advective transport (i.e. see Section 3.3.2 on PSA characteristics). The original SWWB model, developed to evaluate Site-wide hydrologic response to proposed closure, encompassed a larger area, but grid discretization (200-foot by 200-foot) was too coarse for simulating fate and transport.

Advective transport of VOCs in groundwater at RFETS is strongly dictated by local flow dynamics within the UHSU. SWWB flow modeling showed that local groundwater flow paths and velocities at RFETS are complex and depend on many factors and processes. For example, surface flow and unsaturated zone recharge dynamics caused by seasonal changes in climate produce complex three-dimensional saturated zone flow paths. This in turn strongly influences the movement and distribution of VOCs. As a result, it was necessary to develop a saturated zone flow model that explicitly incorporated the complex external stresses, had the capability of simulating the three-dimensional flow, and predicted areas and rates of groundwater discharge.

The integrated hydrologic code MIKE SHE, used to develop the SWWB model (Kaiser-Hill, 2002), is a powerful tool for simulating the complex three-dimensional saturated zone flow. This computer code couples the surface-subsurface flow dynamics and also considers external stresses like the spatial and temporal distribution of climate factors. Available system response data that capture the integrated behavior of the system (i.e., time-varying stream flows, groundwater levels, and surface groundwater discharge with time) can constrain model parameter values more effectively than traditional single process models (i.e., MODFLOW simulation of only saturated zone). As a result, the integrated model produces more confidence in parameterization of key model input such as hydraulic conductivity.

A more localized and refined fully distributed, integrated hydrologic model was constructed for the area impacted by VOCs using the MIKE SHE code (Kaiser-Hill, 2002). This model is herein referred to as the "integrated VOC flow model" because it differs from the former SWWB Model. Its development serves two purposes. First, it is used to simulate current and closure configuration three-dimensional saturated zone flows that are used in subsequent VOC transport predictions. It also represents a tool that can be used to evaluate the hydraulic and hydrologic impacts of land configuration modifications and their affect on VOC transport.

Although some of the input for the SWWB model could be used directly in the integrated VOC flow model, most required direct conversion from the original GIS coverages. This conversion consisted of an automated comprehensive, multi-attribute spreadsheet algorithm. The spreadsheet was developed to streamline development of more refined integrated modeling input datasets (i.e., building-scale models). The spreadsheet algorithm generated model input in a consistent and easily manipulated format. This is important given the substantial amount of data that must be read into and out of the integrated code. The algorithm permitted changes to key parameters, such as specific geologic materials (i.e., Rocky Flats Alluvium or Colluvium), or specific drain leakage coefficients (i.e., sanitary, storm, footing drain, or water supply) to be incorporated directly into model input quickly and consistently.

The model boundary and discretization used in the current and proposed closure configuration integrated flow models are described first, in Section 4.1. Next, the development, application, and results of integrated flow modeling of current conditions are described in Section 4.2. The development, application, and results of integrated flow modeling of the closure configuration are described, in Section 4.3.

#### **4.1 Model Boundary and Discretization**

The integrated VOC flow model developed here differs from the original SWWB model in two ways. First, the areal extent of this model is smaller than the SWWB model. The model boundary and grid discretization of the IA flow model are shown on Figure 4.1. Only groundwater areas currently impacted by VOCs, or areas possibly impacted due to the closure configuration, were considered. Although VOCs in the PU&D Yard area were not included in this model, a separate integrated flow model developed for the Present Landfill system was used as the basis for simulating the three-dimensional groundwater flow in this area (Kaiser-Hill, 2004).

The integrated VOC flow model is also discretized differently than the SWWB model. Instead of using a 200-foot by 200-foot grid to represent groundwater flows, a more refined grid of 60-foot by 60-foot is used. The increased grid refinement better resolves local features, which was necessary to more accurately simulate transport. This is important because the transport of VOCs is sensitive to how accurately hydrologic features, such as building basements, pavement, drains, and other features are numerically represented. In addition, a more refined grid permits more accurate specification of source locations and effective source concentrations that can significantly affect transport of VOCs.

The vertical discretization of the saturated and unsaturated zones of the current model is the same as in the SWWB modeling (Kaiser-Hill, 2002). The saturated zone is divided into four model layers; the upper two layers represent the unconsolidated material, while the lower two represent the weathered bedrock. Two layers were defined for each of these UHSU hydrostratigraphic units to

account for variable depths to subsurface utility corridors and building basements, and for the distribution of Arapahoe Sandstone within the weathered bedrock. The first saturated zone layer was defined as deep, as possible to better represent effects of ET from the unsaturated zone, but is adjusted locally to represent drain inverts. The third layer is adjusted locally to account for the presence of Arapahoe Sandstone, which can immediately subcrop the unconsolidated material, or be entirely embedded within the siltstone/claystone matrix.

## **4.2 Integrated Flow Model – Current Conditions**

### **4.2.1 Model Configuration**

The distributions of hydraulic properties in the integrated VOC flow model are mostly derived in the same fashion as for the SWWB model (Kaiser-Hill, 2002). Various spatial GIS datasets for the subsurface pipelines, utilities, surficial geologic materials, vegetation, pavement, and building coverages, and of Arapahoe Sandstone distributions were used as the basis for developing equivalent numerical representations for input to the model. A series of GIS techniques were used to convert the spatial information into a set of regularly spaced points. This information was then incorporated into the spreadsheet, where a set of algorithms are used to convert this information into appropriate MIKE SHE model input arrays.

Although, hydraulic properties for the different geologic material (i.e., unconsolidated material or bedrock) were not adjusted much from the SWWB model, the grid refinement required hydraulic conductivities associated with local subsurface drain cells to be refined. In addition, drain leakage coefficients also required adjustments in response to the refined grid.

The surface channelized flow routing also required adjustment in response to the grid refinement. The simulated channel network for the integrated VOC flow model is also shown on Figure 4.1. For example, although the SID is included in the model, Woman Creek to the south is not explicitly defined. Instead, effects of groundwater discharge to Woman Creek are simulated using constant head boundaries. The surface routing at the eastern end of Central Avenue and surface routing from the Building 774 area east to a storm drain that flows north into Walnut Creek were extended to better represent channelized flow in these areas.

### **4.2.2 External Stresses**

- External stresses imposed on the integrated flow model are similar to those applied on the SWWB model. These are summarized in more detail in the SWWB modeling report (Kaiser-Hill, 2002). External stresses in the model include: (1) spatially uniform, but temporally variable, temperature with time

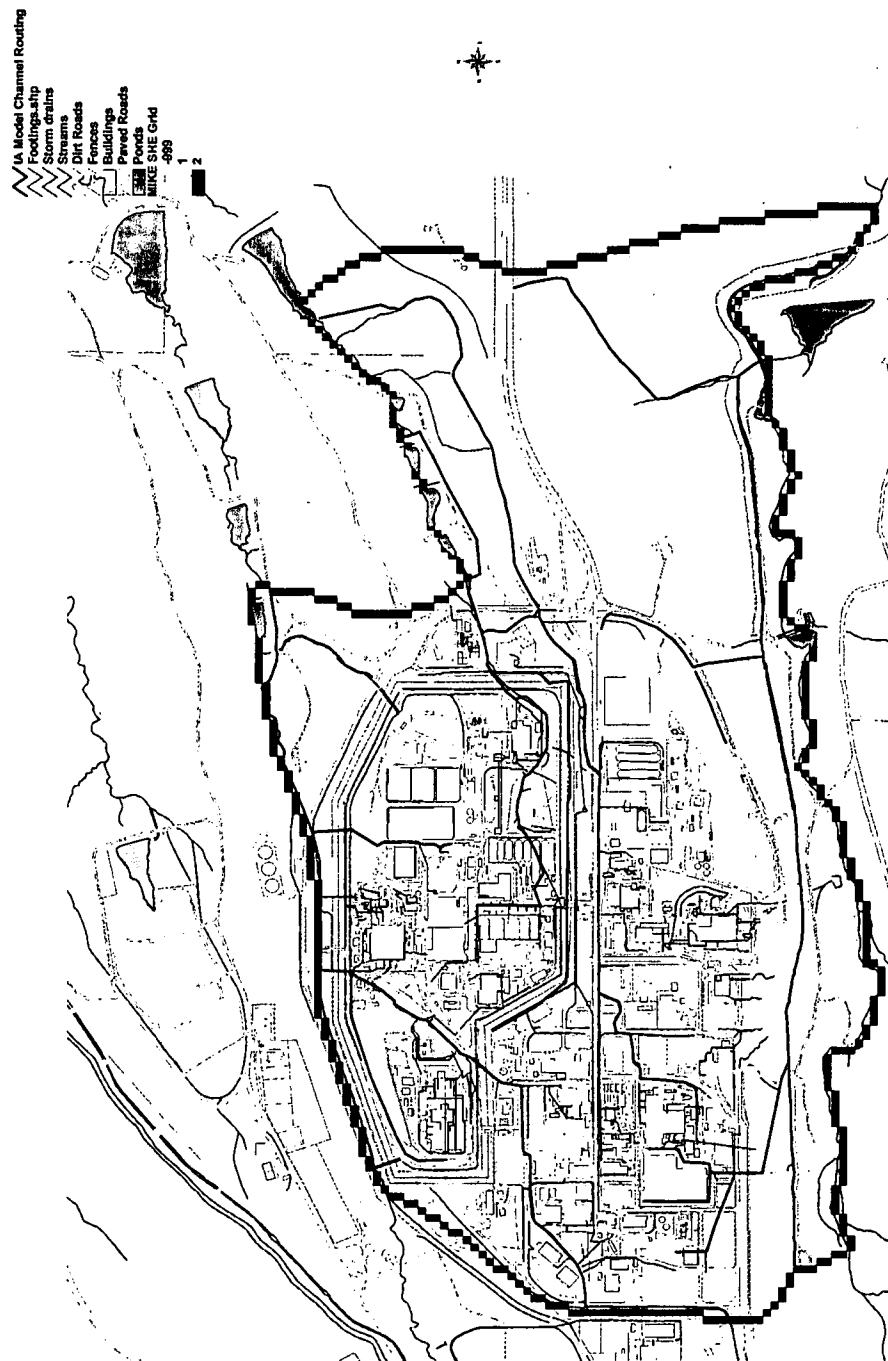


Figure 4.1 Integrated VOC flow model boundary and discretization.

(hourly); (2) potential ET (hourly); and (3) spatially and temporally variable precipitation (15-minute intervals).

Appropriate lateral boundary conditions are applied to the overland surface flow boundary and saturated zone. Overland flow boundaries were chosen such that no significant sources of overland flow are possible across them. As such, a zero-flux boundary condition is imposed along these overland flow boundaries.

Boundary conditions for all four layers of the saturated zone were specified as constant head. These boundaries for the integrated VOC flow model were specifically located to minimize impacts to flow calculations in internal areas of interest and to also represent realistic external boundaries (or external stresses). For example, constant head cells along Woman Creek to the south coincide with this feature because it represents a groundwater flow divide. Boundary conditions were simulated here as constant head to represent streams as discharge points for both groundwater flow and VOC transport. Simulating the effects of stream flow explicitly in MIKE 11 was not considered feasible or efficient as this would have required introducing upstream inflows and modeling overland flows south of Woman Creek.

One implication of specifying the stream boundary condition as constant head is that the flow and transport model developed here can not be used to accurately predict the flux of VOCs as baseflow to the stream. This is because it is not possible to distinguish between baseflow and ET loss in near-stream areas. Still, this boundary condition is reasonable for meeting project objectives (i.e., estimating maximum groundwater VOC concentrations in groundwater discharge areas) because stream-bed elevations (used as constant head elevations in the model) are always lower than the up-gradient (uphill) heads.

Constant head boundary conditions were also imposed along the western edge of the saturated zone using available data. This is reasonable to account for the lateral groundwater inflow. The SWWB modeling showed that this influx is small in comparison to the localized vertical recharge and ET loss. In addition, only limited inflow occurs due to the highly divergent saturated flow field in the western IA. Along the eastern boundary, groundwater flows are generally no-flow. Specifying constant head cells was also appropriate here because inflow and outflow is minimal due to no-flow conditions.

#### **4.2.3 Model Performance Criteria**

Several types of system response data were used to constrain the integrated model performance. These included the following:

- quarterly monitored well water level data;
- continuously monitored well water level data;



- groundwater collection system annual average discharge;
- approximate average annual storm discharge;
- approximate average annual sanitary discharge;
- approximate average annual footing discharge:
  - specific buildings;
  - entire IA; and
- average annual surface stream flow at gauging locations.

#### **4.2.4 Model Results**

In general, the performance accuracy of the integrated VOC flow model in simulating flow is better than the SWWB model for the same area. This is due in part to the ability to better resolve Site features that affect local hydrologic conditions using a refined grid in the numerical model. Simulated groundwater depths generally are within a meter of observed average annual depths<sup>1</sup>. The performance of the integrated flow model for WY2000 is summarized in comparisons between simulated and observed average annual groundwater levels and discharge information. Figures 4.2 and 4.3 show simulated discharge and groundwater levels performance, respectively.

Simulated drain discharges reasonably reproduce observed discharges. Observed discharge rates are only approximate, but believed to be representative of system flows.

Initially, local-scale single-column unsaturated/saturated zone flow models were developed to more efficiently parameterize unsaturated zone parameters such as, the unsaturated hydraulic conductivity – pressure relation, the moisture retention function, field capacity and wilting point, and ET parameters [including the crop coefficient (Kc), Root Depth Function with time (RDF), Leaf Area Index (LAI) with time, and empirical crop growth functions C1, C2, and C3]. Results from these simulations reproduced observed groundwater fluctuations with time reasonably well, though in some, local grid effects prevent accurate reproduction. In general, though, average annual groundwater levels and key recharge events are represented well with the model.

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<sup>1</sup> It should be noted that only wells with complete quarterly measurements were used to compute average annual heads. Heads can vary by 2 to 3 meters seasonally, and without all four quarters, the average head would be biased.

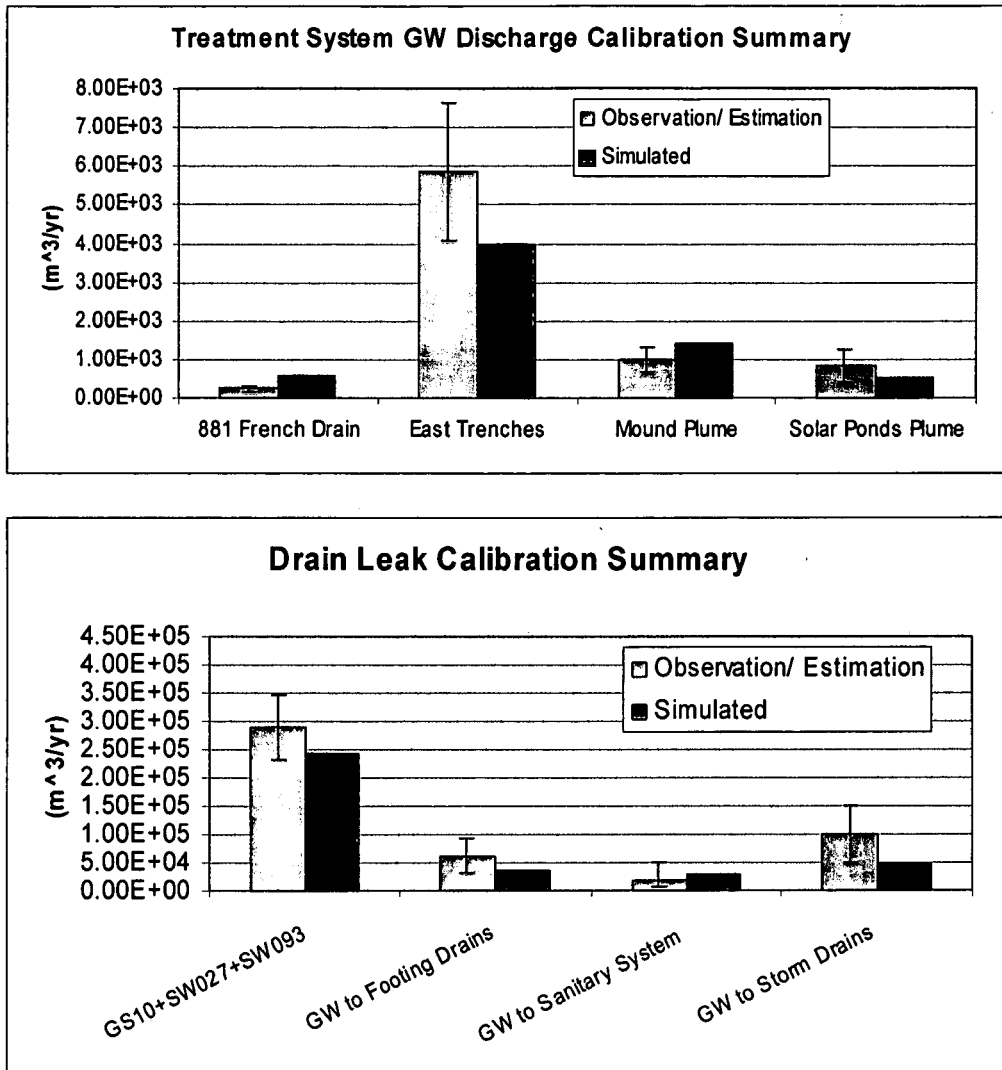


Figure 4.2 Comparison of simulated and observed annual drain and groundwater collection system discharge.

70

Multiple sub-scale models were developed within the integrated VOC flow model to improve the overall computational performance. This permitted more efficient adjustment of parameter values to improve model performance against observed information. For example, a sub-scale model of the Building 444 area, GS10 drainage, SW027 drainage, SW093 drainage to Walnut Creek, and the 903 Pad area and east were prepared. Modeling showed that all of these areas could be parameterized reasonably well to reproduce observed data, though the 903 Pad area proved more difficult. This is due in part to the added complexity of Arapahoe Sandstone in the area. It is also difficult to simulate due to the deeper and more divergent flow paths in the area, where groundwater is directed either north into South Walnut Creek (B-Pond series), or south into the South Interceptor Ditch (SID), or Woman Creek.

### **4.3 Integrated Flow Model – Proposed Closure Configuration**

This section describes the MIKE SHE integrated modeling of the proposed closure configuration. The purpose of this was to generate a new saturated zone flow field using typical climate (WY2000) and to identify areas where groundwater discharges to surface water.

#### **4.3.1 Approach**

Using an approach similar to that implemented in the integrated hydrologic modeling in the SWWB modeling (Kaiser-Hill, 2002), hydrologic effects resulting from the currently proposed closure configuration modification were simulated using the refined integrated flow model. The closure configuration modifications were similar to the SWWB-reported modifications, but differ in several ways. Specific modifications were described in detail in Section 4.3.2 below.

For the CRA, the most important simulated system response from the integrated hydrologic model of the proposed closure configuration was where groundwater discharges to the surface. Discharge rates were also important, but the frequency of groundwater discharge was more important. For example, areas where groundwater discharges to the surface intermittently (i.e., only after large precipitation events) or where it discharges continuously were identified. A typical climate sequence (WY2000) was used to simulated average groundwater levels in the integrated model.

#### **4.3.2 Closure Configuration Modifications**

Specific closure configuration modifications are described in this section. Several changes were necessary to the integrated flow model and are summarized below.

The closure configuration land surface topography provided was somewhat different than the previous regrade topography. By comparison, this revised

surface grading plan reduces the overall amount of fill material, but still included both cut and fill areas within the IA. Two notable borrow areas were located west and north of the Building 371 area. The surface topography of the two borrow areas located north and west of Building 371 actually occurs below the top of weathered bedrock, exposing the bedrock in these areas.

The western end of the SID was removed to accommodate regrade of the entire surface over the Original Landfill area. SMEs indicated that an 18% slope should be used, but currently only a ~11% slope exists. Therefore, only the SID feature was graded over, keeping an approximate 11% slope.

Surface routing based on the new regrade and associated drainage plan also changed in several areas. Areas where surface routing has been modified were shown on Figure 4.4. Four new stream channels were included in the new regrade and drainage plan within the internal part of the IA. These effectively removed surface flow in the former western part of the South Walnut Drainage (upstream of GS10 gage), and the southern unnamed stream that extends into the IA within the North Walnut Creek Drainage (upstream of the SW093 gage). No surface drainage features occur in the southern IA (i.e., former SW022 drainage), the SID, or Woman Creek.

All building footing drains, storm drains, and sanitary drains were removed from the simulation. In the present configuration, these actively remove groundwater throughout the IA where they occur. Leaky water supply lines were removed.

The increased hydraulic conductivity associated with the trenches for all utilities (nine utilities described in Section 4) was not modified. Although some of these may be disrupted (i.e., sanitary lines), given the density and interconnection of these subsurface trenches, it was assumed these disruptions would have only limited impacts (i.e., only very local effects).

The four groundwater collection systems (Solar Ponds Groundwater Collection System, 881 Hillside Groundwater Collection System, the East Trenches Groundwater Collection System, and the Mound Groundwater Collection System) were included for the integrated flow modeling results shown here, but were not included in VOC transport simulations.

Subsurface basement structures associated with four building areas were simulated as low conductivity material. These areas were associated with the Buildings 881, 371, 991, and 771. Much of the basement slabs and walls remained in these areas. The removed portions were determined by the newly regrade surface. All basement material was removed to at least 3 feet below grade. Because of the 60-foot grid resolution, flows calculated within these former building footprints (above the remaining slabs) were not considered in this modeling. Hydrologic effects on flows within and external to building basement flows were evaluated in several individual high-resolution grid models and are described in the Groundwater Interim Measures/Interim Remedial Action Report (IM/IRA) (under development).

Vegetation was assumed to be xeric-type in all former paved, or building areas, while the pre-existing native vegetation remains in all other areas (principally along hillslopes).

Where fill material was placed to adjust to the revised proposed closure configuration topography, it was assumed to be similar hydraulically to the Rocky Flats Alluvium. In all other areas, the pre-existing soils remained.

The WY2000 climate sequence (i.e., the 15-minute precipitation series, the hourly ET series, and the hourly temperature series) was used to constrain the integrated system response for the closure configuration. This climate was assumed to represent a typical year because the total annual precipitation was close to the long-term annual precipitation at the Site (~15 inches/year). Simulating the typical climate sequence was also considered the most appropriate for establishing long-term average groundwater flow paths and groundwater levels to drive long-term transport simulations. A wet-year or dry-year climate was inappropriate because groundwater flows, flow directions, and groundwater levels under these conditions would not be representative of long-term average conditions.

#### **4.3.3 Model Results**

Three consecutive years were simulated to stabilize initial conditions. Results from the final year were used to produce average model output. Several types of output were obtained from the model; the average annual groundwater levels were of most interest because they were used as both initial and boundary conditions for the GMS MODFLOW groundwater flow closure configuration simulations (described in Section 6.1.2). Simulated closure configuration groundwater flow directions, the change in groundwater flow directions from current conditions, and average annual ET are also discussed below.

Simulated average and minimum annual groundwater levels were illustrated on Figures 4.5 and 4.6. Results showed that simulated groundwater depths in several areas were above ground surface for average annual conditions (Figure 4.5). These areas occur in the drainage between Buildings 771 and 371 in the

north western part of the IA, along the drainage immediately northeast of the Solar Ponds trench (SW091 gage), along the re-engineered South Walnut Creek drainage immediately south of Building 991, and in isolated locations along Woman Creek. These shallow groundwater areas increased in size in the minimum annual groundwater depth plot (Figure 4.6). For the latter case, levels increased only during, or immediately after, large precipitation events (i.e., typical annual spring recharge period).

The simulated change in average annual groundwater levels from current to the closure configuration were illustrated on Figure 4.7. Results showed that water levels changed less than half a meter over much of the central IA, and most of the eastern model area, where proposed closure configuration modifications are minimal. Groundwater levels in the western central IA generally decreased more than 0.5 meters (due mostly to removal of leaky water supply lines). Areas where groundwater increased more than 0.5 meters occurred near all of the buildings with deeper footing drains (i.e., Buildings 771, 371, 881, and 991) and in the central IA where groundwater had discharged to the higher density of storm and sanitary drains (deactivated in the closure scenario).

Changes to the South Walnut Creek drainage and a new routing scheme for the Central Avenue drainage that merges into South Walnut Creek east of the Mound Groundwater Collection System trench caused notable changes in groundwater levels in this area, both increased and decreases.

Simulated closure configuration groundwater flow directions were plotted for both the unconsolidated and upper weathered bedrock layers in the model (i.e., two and three) on Figures 4.8 and 4.9, respectively. Groundwater flow vectors, uniformly sized, show simulated flow directions from the center of each model grid cell. In some areas, flow directions were meaningless because the cell for that layer was desaturated (i.e., the water table was below the bottom of the layer). Results confirm that flows were largely similar between the unconsolidated material and weathered bedrock. They varied in local areas due to local variations in hydraulic conductivities, mostly in the unconsolidated areas associated with utility trenches (more variability in higher density utility areas) or areas where subsurface building structures remain. The flow vectors demonstrate that the simulated closure configuration groundwater in both the unconsolidated and weathered bedrock layers generally flows toward nearby streams.

The change between simulated current and the proposed closure configuration groundwater flow directions for the upper weathered bedrock was shown on Figure 4.10. Results confirmed that flow directions remained largely unchanged, except for some areas. In areas where more significant land reconfiguration occurs, groundwater flow directions differ.

Simulated areas where groundwater discharges to either surface streams, or the ground surface (overland flow that entered streams) were presented on Figure 4.11. This plot showed simulated annual discharge rates to streams, or overland flow. Three of the four modified internal surface streams indicated discharge, while the fourth (Central Avenue) routing that merges with South Walnut Creek east of the Mound Groundwater Collection System does not appear to produce groundwater discharge. As groundwater flow nears streams, an increasing amount was lost via ET as shown on Figure 4.12. Discharge for each cell ranged from approximately 60 m<sup>3</sup>/year to nearly 450 m<sup>3</sup>/year, with the highest amounts along stream areas. The implication of this was that as groundwater (with VOCs) nears streams, increasing amounts of dissolved phase VOCs in groundwater were lost via ET.

## 5.0 FLOW AND TRANSPORT MODEL SELECTION

This section describes the approach used to select the flow and transport code(s) to meet project objectives described in Section 1.0. Three primary steps were involved in selecting an appropriate code: (1) identifying specific modeling needs (model specifications); (2) defining selection criteria; and (3) identifying and selecting the appropriate code. Each of these steps is described briefly in Sections 5.1 to 5.3.

Code selection methodology was derived from the following sources: (1) work done in the SWWB model (Kaiser-Hill, 2002); (2) a Foster Wheeler Environmental Corporation document entitled "RBCA Fate and Transport Models: Compendium and Selection Guidance" prepared for American Society for Testing and Materials (ASTM), dated November 1998; and (3) a DOE report entitled "Computer Code Selection Criteria for Flow and Transport Codes" (Mann et. al., 1999).

### 5.1 Flow and Transport Modeling Needs

As described in Section 3, the flow system at RFETS within the IA is complex and characteristic of shallow semi-arid hydrologic system. The industrialized nature of the IA also has a notable impact on the surface/subsurface flow system. The conceptual flow/transport models described in Section 4 dictate which key features are necessary components of a flow and transport code so that reliable, long-term VOC fate and transport predictions can be calculated to meet project objectives.

Several features at RFETS impact groundwater flow pathways and velocities and must be considered in a groundwater flow model that supports transport modeling. Some of the more important features include:

- subsurface drain flows (storm, sanitary, building footing drains, and groundwater collection systems);
- preferential flow within utility trenches;
- three-dimensional subsurface basement effects;
- impervious areas; and
- spatial distributions of saturated zone material, unsaturated zone soil properties, vegetation coverage, and topographic details (i.e., changes from current to the proposed closure configuration surface).

Important flow processes that need to be simulated include integrated surface-subsurface flow processes, including: (1) spatial and temporal recharge



distributions; (2) stream-groundwater interactions; (3) surface ponding; (4) surface discharge at seeps; and (5) ET.

Key transport processes required to simulate transport at RFETS include three-dimensional advection and dispersion, anaerobic and aerobic degradation, sorption, diffusion, volatilization, and plant uptake (or ET).

## 5.2 Code selection criteria.

	Criterion	Action
1	Ability to simulate processes identified in the conceptual model (transport, decay, and reactive processes)	Reviewed code user manuals
2	Compatibility with existing MIKESHE based water balance model	Reviewed manual
3	Ability to incorporate VOC source terms	Reviewed manual
4	Well documented	Reviewed documentation
5	Code validated	Reviewed documentation
6	Accessibility and support from developers	Discussed with developers
7	Availability of developer for training	Discussed with developers
8	Code demonstrated at field sites, DOE sites	Publications
9	Public domain code	Web site verification
10	Time needed to learn the code	Discussed with users/developers
11	Computational efficiency	Discussed with users/developers
12	Code Cost	Discussed with developers
13	Graphical User Interface	Evaluated Demo, reviewed manual

### 5.3 Selection process

Many flow and transport codes are available that are capable of simulating VOC fate and transport at RFETS. As a result, many of these could not be reviewed, or fully evaluated in this study. Instead, only the more appropriate codes were identified and for evaluation against the above criteria outlined in Section 5.1. Based on these criteria, GMS was selected as the best available tool. It had to have the following attributes: (1) utilizes a state-of-the-art graphical user interface (GUI); (2) use is well documented; (3) contains relatively thorough documentation; (4) publicly available; (5) free of cost; and (6) familiarity with the code and its code developers. As such, it met or exceeded selection criteria.

The GMS software developed by the DOD was selected for the transport modeling, primarily because of its well-documented application at DOD, DOE, and Environmental Protection Agency (EPA) sites. The RT3D reactive transport code (Clement, 1997) is a modification of the popular transport code MT3D developed by the USGS (Zheng, 1990). The RT3D code requires a three-dimensional flow velocity array as the basis for transport simulations. This is completed using the MODFLOW 2000 code included in GMS graphical user interface. Unfortunately, the MIKE SHE software does not provide this velocity array in a format that is readily compatible with RT3D. Furthermore, despite recycling the WY2000 climate sequence to obtain a quasi-stable velocity field, the MIKE SHE output still exhibited considerable seasonal flow dynamics which is inefficient for simulating long-term RT3D reactive transport models. As a result, the GMS MODFLOW 2000 code was used as the basis for generating the three-dimensional saturated zone flow field for the RT3D simulation. The conversion of the MIKE SHE model input to the GMS MODFLOW code is discussed in more detail in Section 6.0.

Capabilities of the GMS software include the following:

- Advanced graphical interface. This feature allows the modeler to incorporate all flow and transport information into a single graphical interface. It also provide a powerful interface for both input and output of a substantial amount of modeling data that greatly improves the overall analysis of computational simulations. The interface greatly aids in graphical, or direct spreadsheet specification of model input and grid generation. In addition, more advanced features allow model output from both the MODFLOW flow and RT3D transport models to be displayed readily, or in an animated mode, for presentations;
- The GMS software incorporates the USGS MODPATH particle tracking code, which is also a key feature employed in this study to evaluate groundwater flow velocities and pathways (i.e., VOC source location assessment); and
- The GMS software also meets the selection criteria.

## **6.0 GROUNDWATER FLOW AND TRANSPORT - HISTORICAL AND CLOSURE CONFIGURATION**

The development of steady-state groundwater flow and reactive transport models used to simulate the historical growth of the currently observed PSAs and their subsequent fate under proposed closure configuration conditions are described in this section. The overall modeling approach is described first in Section 6.1. The development of groundwater flow and transport models, application, and results are described in Section 6.2.

### **6.1 Modeling Approach**

The overall modeling approach, described first in Section 6.1, is more detailed than the project approach described earlier in Section 3.1. The approach used to convert integrated flow model input into equivalent MODFLOW steady-state flow model input is described first, in Section 6.1.1. The approach used in developing steady-state MODFLOW models and groundwater flow path analysis, using the particle tracking code, MODPATH, are described next in Section 6.1.2. The approach used to model reactive transport of historical PSA growth from inferred VOC sources, determined through flow path analysis, is described in Section 6.1.3. Finally, the approach used to simulate the eventual fate and transport of present PSA concentration distributions, from inferred VOC sources, under the closure configuration is described in Section 6.1.4.

Results of the groundwater flow and transport modeling are described in Section 6.2. PSA-specific areas are described first in Section 6.2.1. Section 6.2.2. summarizes general findings from the modeling for all PSAs.

#### **6.1.1 MIKE SHE Conversion to MODFLOW**

Model input from the integrated MIKE SHE flow model was converted into equivalent GMS MODFLOW flow models. Although some MIKE SHE model input arrays converted directly, others had to be approximated. Specific model input that could be converted directly included the following:

- the topographic surface;
- model layer surface elevations;
- model layer hydraulic conductivities;
- locations and invert depths of river, drains, and constant head boundary cells; and
- locations and recharge rates (leakage) of water supply lines.

Other input did not convert directly and required additional adjustment in the GMS MODFLOW model. These included the following:

- river conductance;
- drain conductance;
- recharge; and
- ET parameters.

Horizontal and vertical hydraulic conductivity arrays from the integrated flow model had to be slightly adjusted during MODFLOW flow model simulations to reproduce the distribution of time-averaged well VOC concentrations. The integrated flow modeling did not incorporate transport information to help constrain the hydraulic conductivity values. As a result, slight adjustments to the hydraulic conductivity values in the MODFLOW models were appropriate to improve groundwater velocities so that particle tracking distances were consistent with PSA extents. No adjustments were made to the integrated flow model hydraulic conductivity values. For consistency, the modified hydraulic conductivity values in MODFLOW were used for simulating later closure configuration scenario simulations (Section 6.2).

River and drain conductance values also had to be adjusted so that observed groundwater discharges to streams and drains within each PSA were reproduced. The locations of drain and river cells coincided with those specified in the integrated flow model.

Several recharge zones were defined within each PSA flow model based on unsaturated soil and vegetation zones defined in the integrated flow model. Zonal recharge values within each GMS flow model were also adjusted to reproduce observed information (groundwater levels and discharge) and to produce particle flow paths consistent with assumed PSA extents. The total simulated MIKE SHE model annual recharge to the saturated zone (within a given GMS model area) was used as a general constraint on the overall simulated steady-state GMS MODFLOW water budget.

ET in the MIKE SHE model was simulated for both the unsaturated and saturated zones. In MODFLOW, it is simulated as a net loss from the saturated zone. The total simulated MIKE SHE model annual ET loss from the saturated zone within each PSA flow model area was also used as a general constraint on the overall simulated steady-state GMS MODFLOW water budget.

### **6.1.2 Groundwater Flow Model Development**

Eight different steady-state PSA MODFLOW groundwater flow models were developed to improve computational efficiency, rather than attempt to simultaneously simulate the flow and transport of all PSAs in a single flow model.

These PSA model areas were also used as the basis for historical and proposed closure-condition reactive transport modeling. The rationale for selecting these boundaries is described further in Section 6.3. The eight model areas are defined as the following:

- PSA 2N (Northern extent of 903 Pad area and East Trenches area);
- PSA 2S (Southern extent of 903 Pad area and Ryan's Pit area);
- PSA 5 (Mound /Oil Burn areas);
- PSA 9 (881 Hillside area);
- PSA 10 (Building 444 area);
- PSA 12 (Central IA);
- PSA 14 (Building 771/IHSS118.1 area); and
- PSA 15 (Western Former Solar Ponds area).

#### **6.1.2.1 Model Boundaries and Boundary Conditions**

Spatial extents of each PSA groundwater flow model domain are shown on Figure 6.1. They are defined based on several factors. The extent of each model included the historical, current, and proposed closure configuration VOC distribution. It was large enough that external boundary conditions (i.e., along the edges) did not affect internal flow and transport calculations except at areas where groundwater discharges to the surface. The MODPATH code was used to confirm that model boundaries were appropriate by producing long-term particle tracks from suspected source areas that could then be evaluated using observed Site data.

In some instances, PSA model boundaries overlap as shown on Figure 6.1. These overlaps only reflect the need to have conservatively large model areas to avoid external boundary condition effects on internal calculation cells. Several PSA models (i.e., PSA 2N, PSA 2S, and PSA 12) included more than one PSA and multiple sources.

Site hydrogeologic features were used to develop initial model boundaries. For example, because source areas occur in uphill or mesa areas, they currently flow, or will eventually flow, down-gradient towards streams. As such, downhill boundaries were assigned at stream boundaries which is appropriate because they coincide with groundwater divides and represent groundwater discharge areas. In some cases, appropriate internal drainage boundaries were specified at stream locations, instead of constant head cells (i.e., PSA 12). Uphill boundary conditions were generally prescribed as constant heads, but in some

instances, were specified as no-flow conditions (where groundwater flow directions parallel the boundary). Hydraulic heads derived from the integrated MIKE SHE flow model were used for these constant head boundary cells. Simulated average annual heads, obtained from the integrated model, were also used as initial heads for the steady-state PSA flow models.

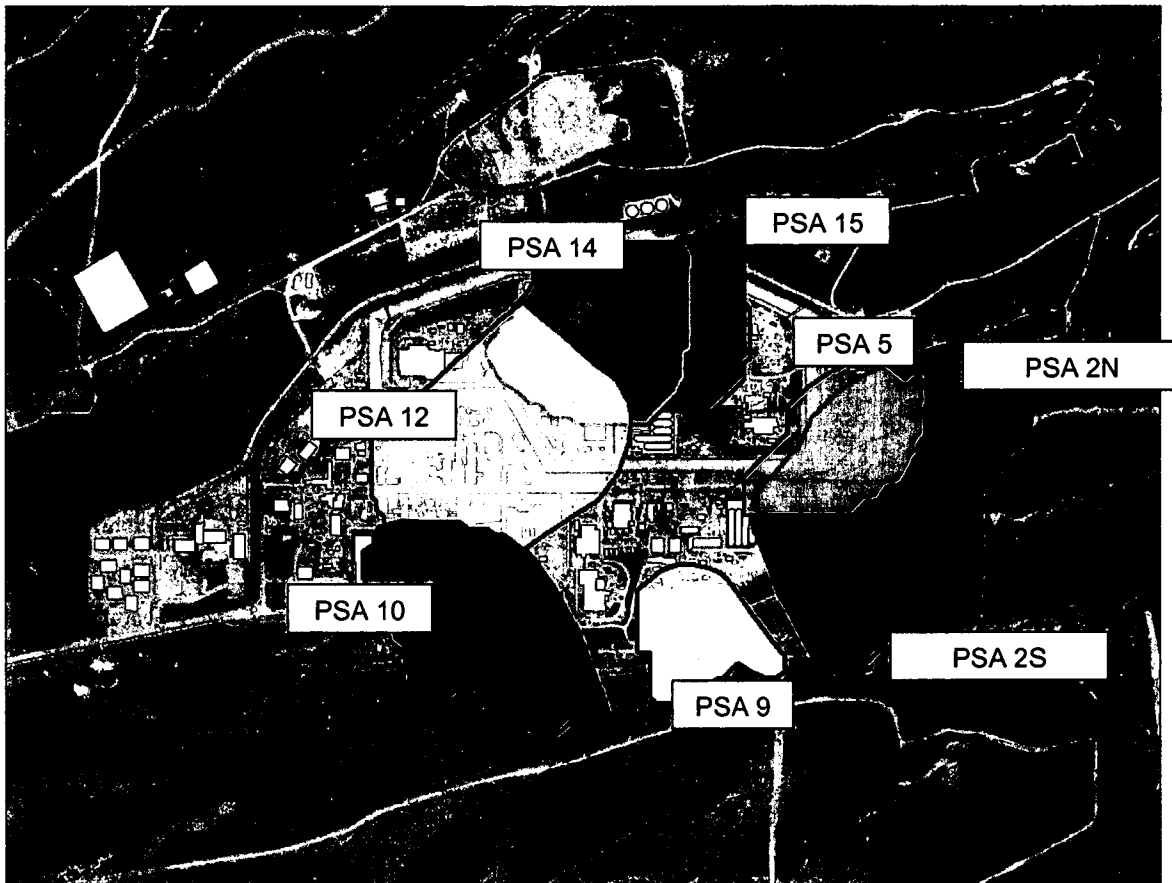


Figure 6.1. PSA locations.

Other spatial model boundary conditions assigned in each PSA flow model included river cells, leaky water supply lines (simulated using the MODFLOW well package), drain cells, spatially distributed recharge, and ET. Although locations of these boundary conditions coincided with the integrated model, some parameter values had to be determined through iterative model simulations (constrained by performance criteria).

Although the steady-state flow models could not reproduce the transient behavior simulated in the integrated MIKE SHE flow model, the simulated steady-state flow fields are appropriate for simulating long-term flow and transport. For example, transient system flow response, such as annual or seasonal recharge and ET variations, affect groundwater levels, but they generally do not significantly perturb steady-state groundwater flow paths or velocities within the

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IA. These flow paths and velocities are strongly controlled by the underlying hillslope morphology.

The GMS flow models were constructed using input from the integrated MIKE SHE flow model. Though much of the model input converted directly, some required modification.

#### **6.1.2.2 Parameter Estimation**

Parameterization of the flow model recharge rates was accomplished using the automated parameter estimation code, PEST (Watermark, 2003), included within the GMS software graphical user interface (GUI). Other parameter values (such as drain and river conductance) were adjusted manually, given their spatial complexity. Spatially distributed recharge values (within zones) were adjusted based on the performance criteria described in Section 6.1.2.3. Recharge was varied over six to eight different zones within each model. More permeable areas were assigned initial recharge values, while recharge in impermeable areas (i.e., building, or paved areas) was kept constant at zero. High and low recharge rates were specified for each recharge zone value to limit the possible values considered in the PEST simulations. These high and low values were determined based on ranges determined from the MIKE SHE flow model.

Hydraulic conductivity values for the unconsolidated material layers and weathered bedrock layers were manually adjusted in each MODFLOW flow model to improve the correlation between particle paths and concentration distribution, and between observed and simulated heads. Though the flow model had zoned conductivity distributions, only the conductivity of entire layers was varied by a multiplication factor that ranged from 0.5 to 5.0. The original distribution was preserved. Multiplication factors were limited in magnitude to retain consistency with the MIKE SHE model.

Simulated average annual groundwater flow conditions from the integrated flow model and observed average annual well water level information (quarterly water levels, WY2000) were used as model performance criteria. Simulated average annual groundwater discharges from the integrated flow model, combined with observed drain and building discharge, were also used to constrain the MODFLOW flow model parameter values.

#### **6.1.2.3 Model Performance Criteria**

Several types of criteria were used to constrain the performance of individual MODFLOW models. These included the following:

- integrated flow model results:
  - simulated mass balance for individual PSAs (i.e., net recharge, ET, stream baseflow, and collective PSA drain flows), and

- simulated average annual heads;
- average annual water levels (at wells);
- observed discharges from key buildings, or groundwater collection trenches (i.e., East Trenches Groundwater Collection System, Mound Groundwater Collection System, 881 hillside system, and Solar Pond Area Groundwater Collection System); and
- groundwater flow paths and velocities as inferred based on assumed source locations, release times, and PSA extents.

Simulated average annual heads and discharges from the integrated flow model were used to constrain the steady-state MODFLOW models for each PSA. Although the integrated flow model reproduced groundwater levels well (i.e., generally less than one meter residuals), in some instances they did not. As a result, average annual groundwater levels were also used to help constrain each MODFLOW flow model input parameter values.

Observed discharges were only available for certain building footing drains and subsurface drains (i.e., storm and sanitary). This information was used to the extent possible, given that individual PSA flow model boundaries cut across some drains. Cumulative approximate observed discharges (Kaiser-Hill, 2002) could not be used as criteria in the individual flow models. However, simulated annual integrated flow model groundwater discharge for individual drains (storm, sanitary, and groundwater collection trenches) was calculated and used to constrain the MODFLOW flow model parameter values. Other simulated annual discharges were used to constrain values included net recharge, net ET, net baseflow, and stream inflow over each model area.

Results of the MODPATH particle tracking analysis were also used as performance criteria to constrain input parameter values. Particles were introduced at inferred VOC source locations and allowed to travel between 30 to 50 years based on HRR information (Appendix A) and discussions with SMEs. Groundwater flow parameter values (such as saturated hydraulic conductivity, recharge rates, and ET rates) were adjusted so that particles traveled to PSA downstream extents and within lateral boundaries (as defined on Figures B-1 through B-7). Only parent VOC PSA distributions were considered in this analysis. Saturated hydraulic conductivities were only varied slightly from those obtained in the integrated flow model. This was appropriate as the information from transport was not included in development of the integrated flow model.

#### **6.1.2.4 Flow Path Analysis – Particle Tracking**

Once the individual GMS groundwater flow models were developed, the USGS particle tracking code, MODPATH, was iteratively used to assess the number, location, and introduction times of individual assumed VOC sources with each PSA. Particle tracking was used to confirm that transport from inferred VOC source areas produced reasonable pathways and velocities within PSA



boundaries (Figures B-1 through B-7, Appendix B). If particles migrated considerably further than the PSA boundaries, it was assumed that the porosity was too low, conductivity values were too high, or sources were introduced too early. Conversely, if particles only migrated short distances from assumed source areas, it was concluded that sources were not introduced early enough, porosity was too high, conductivity values were too low, or that sources were placed in the inappropriate hydrostratigraphic layers. Finally, if assumed source locations, depths, or numbers of sources resulted in poor VOC travel times or pathways, iterative analysis using particle tracking was used to assess additional scenarios.

### **6.1.3 Transport Modeling Approach – Historical Conditions**

The uncertainty associated with groundwater sources precluded using a traditional approach to modeling the fate and transport of VOCs. Instead a sensitivity analysis was conducted that involved several steps. First, parameters that most affected the fate and transport of VOCs in groundwater were identified. Next, an approximate range of parameter values was estimated that effectively bracketed the spatial distribution of time-averaged concentrations within each PSA. Finally, effective source concentrations were estimated at assumed source locations.

Individual reactive transport models were developed for each PSA using the three-dimensional steady-state groundwater flow fields simulated for each of the PSA MODFLOW models. The RT3D code was used to develop the reactive transport models. Based on comparisons between the draft surface water PRGs and maximum observed sample concentrations, only the PCE, TCE, and  $\text{CCl}_4$  parent VOCs were selected for transport modeling. Associated VOC daughter products were calculated internally by the reactive transport models.

The locations, depths, and release times of VOC sources, approximated through steady-state particle tracking analysis, were used in the reactive transport simulations. Although particle-tracking results reflected the advective transport of VOCs from assumed source areas, they did not consider the combined effects of degradation, sorption, dispersion, or diffusion on plume migration. As a result, the reactive transport modeling was used to confirm the MODPATH source locations, depths, and release times of VOC sources. Because source concentrations were unknown, effective values were iteratively determined through the reactive transport modeling.

The relative sensitivity of VOC transport to key model parameter values was evaluated iteratively in fourteen sensitivity runs summarized in Table 6.1. Key parameter values were adjusted in the reactive transport models for each PSA to reproduce the spatial distribution of time-averaged concentration data. Although limited Site-specific information was available for these parameter values (i.e., source location, chemical sorption, hydraulic conductivity, porosity, degradation rates, and source concentrations), sensitivity simulations were used to bracket

the range of values compared to published information. Results of sensitivity simulations indicated that ranges could be constrained.

During the sensitivity evaluation, reproducing time-averaged concentrations in wells down-gradient of the source was emphasized. Near-source concentrations probably do not reflect well-mixed groundwater concentrations (either too high, or too low). Successive model iterations were made until model results reasonably simulated observed average historical well concentrations.

#### **6.1.3.1 Transport Model Parameters**

The approach used in specifying key transport model parameters adjusted in the sensitivity simulations (summarized in Table 6.1) are described in this section. Information on inferred VOC sources, including locations, depths, timing, effective concentrations, and composition are described in Section 6.1.3.1.1. Rationale for specification of degradation, sorption and dispersion parameter values in the reactive transport modeling of the historical and the closure configuration is described in Sections 6.1.3.1.2 through 6.1.3.1.4, respectively.

##### **6.1.3.1.1 Inferred VOC Sources**

VOC source information represented the greatest uncertainty in transport modeling at the Site. Input parameters specified in the reactive transport model simulations for source information included source cell locations, source depth (unconsolidated material, or bedrock), source introduction timing, effective source concentration, and source VOC composition. HRR information, described in Appendix A, was used as the primary source of information for specifying this source information. This database was developed from the HRR, with additional input from Site SME, Nick Demos.

In most PSAs, information on likely VOC sources was limited (see Section 3.3.3). For example:

- the location of a possible source was not known with confidence to within several grid cells (60-foot by 60-foot model grid);
- the depth of source release was not recorded;
- the year the source was introduced at the Site was often based on the first time the spill was noted (non-conservative assumptions), or conversely, the date that the building or feature where the source was located was built (conservative assumption). It was often unclear how the source release date was determined;
- the concentration of the source was rarely noted, and when noted it was often estimated; and
- the source composition was often vague (i.e. "oil-grease" or "solvents") or not well documented.

**Table 6.1. Sensitivity simulation summary.**

Sensitivity Simulation	Rationale
Base case	This model simulation represented a base case. Parameters in subsequent models were varied from this model.
Source Depth	The contaminants move more slowly in the lower weathered bedrock layers because of the lower hydraulic conductivity.
Split source release between two layers	Produces significant difference in down-gradient concentrations. Depends on differences between permeabilities in unconsolidated material and weathered bedrock.
Low dispersivity	Produces conservatively high concentrations downgradient. Reduces lateral and vertical concentrations.
High dispersivity	Produces comparatively lower down-gradient concentrations. Simulated concentrations located off the primary flow path (lateral and vertical) were higher.
Low sorption	Produces higher down-gradient concentrations. Less time for degradation. Increased likelihood of higher concentrations reaching discharge locations. Larger area with higher overall concentrations for a constant source.
Low porosity	Produces lower down-gradient concentrations. Less time for degradation. Increased likelihood of higher concentrations reaching discharge locations. Larger area of high overall concentrations for a constant source.
High porosity	Produces lower down-gradient concentrations. Contaminants travel slower over a given time period. More time for degradation to attenuate transport.
Low source concentrations	Produces lower down-gradient parent product concentrations. Lower concentration of parent (source) also gives lower concentration of daughter compounds.
High source concentrations	Produces higher down-gradient parent product concentrations. Higher concentration of parent (source) also causes higher concentration of daughter compounds.
Low degradation rates	Produces higher parent product concentrations down-gradient. Daughter product concentrations are lower down-gradient. Can be less conservative with respect to daughter products. Differs by PSA and VOC.
High degradation rates	Produces lower down-gradient parent concentrations, but can increase daughter product concentrations. Depends on factors that control groundwater velocity and relative parent/daughter compound degradation rates. Differs by PSA and VOC.
Low hydraulic conductivity (MODFLOW model)	Produces lower down-gradient parent concentrations. Groundwater and contaminants travel slower and allow more time for degradation.
High hydraulic conductivity (MODFLOW model)	Produces higher down-gradient parent concentrations. Groundwater and contaminants travel faster allowing less time for degradation.

\*Note: The term "conservative" in Table 6.1 represents the case where VOC concentrations are simulated high.

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#### **6.1.3.1.1.1 Location**

Inferred source locations were determined initially based on their proximity to observed high concentrations in groundwater and using the HRR information (Appendix A). Priority 1 releases were considered as sources first because their source volumes were assumed to be the greatest. Priority 2 releases were considered next, and Priority 3 releases were not considered significant enough to model in this study. In the transport models, inferred VOC sources were generally specified within a single grid cell (60-foot by 60-foot) at constant concentrations. However, in several instances where the VOC source was inferred over an area larger than a single cell, multiple cells were used to specify the source. Source locations specified in the database were modified only in cases where the position was not specifically known. For example, a potential source associated with VOC activities in a large building was located in the center of the building in the database. In the RT3D model this location might be moved to a loading dock as assumed spill location in this area.

A spatial plot of inferred VOC source locations based on model results is shown on Figure 6.2 (PCE and TCE) and Figure 6.3 (CCl<sub>4</sub>). A summary of inferred VOC sources determined through modeling and from HRR information is included in Appendix B. The model-specified source cells and initial modeled concentrations are summarized in Appendix C.

#### **6.1.3.1.1.2 Depth**

Information on the depth of inferred VOC sources was largely unavailable. However, VOC sources were conceptualized (Section 3.5) as resulting from DNAPL releases at the surface or in the unsaturated zone that migrated downward and pooled above lower permeability material (assumed here to be the weathered bedrock). As a result, simulated VOC sources were specified either within the unconsolidated material, within the weathered bedrock, or both. If the MODFLOW model results indicated that the water table was below the weathered bedrock surface, the source was placed in the upper weathered bedrock model layer. If this layer was also unsaturated, the source was placed in the lower weathered bedrock layer. Source depth played a significant role in defining transport rates considering the contrast in conductivity between the unconsolidated material and weathered bedrock.

#### **6.1.3.1.1.3 Timing**

Inferred VOC sources were assumed to start based on HRR information and discussions with Site SMEs, if available. It was assumed that source start dates preceded the actual release times noted in the HRR. If this information was unavailable, inferred VOC sources were assumed to have been introduced into the groundwater system sometime between 1950 and 1970. These assumptions were evaluated through particle tracking analysis and reactive transport simulations using time-averaged concentration data.

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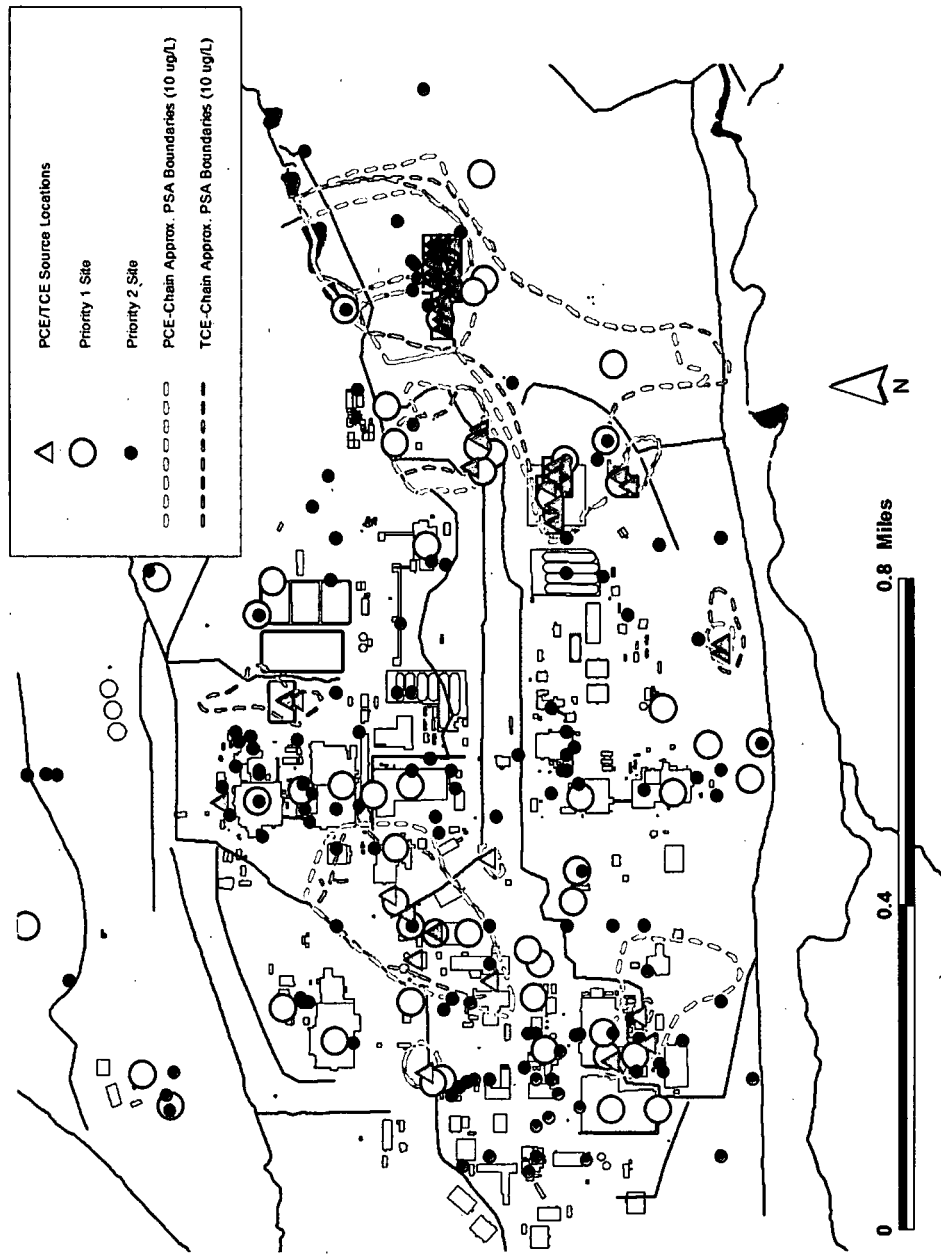


Figure 6.2. Priority 1 and 2 HRR releases with inferred PCE/TCE source locations.

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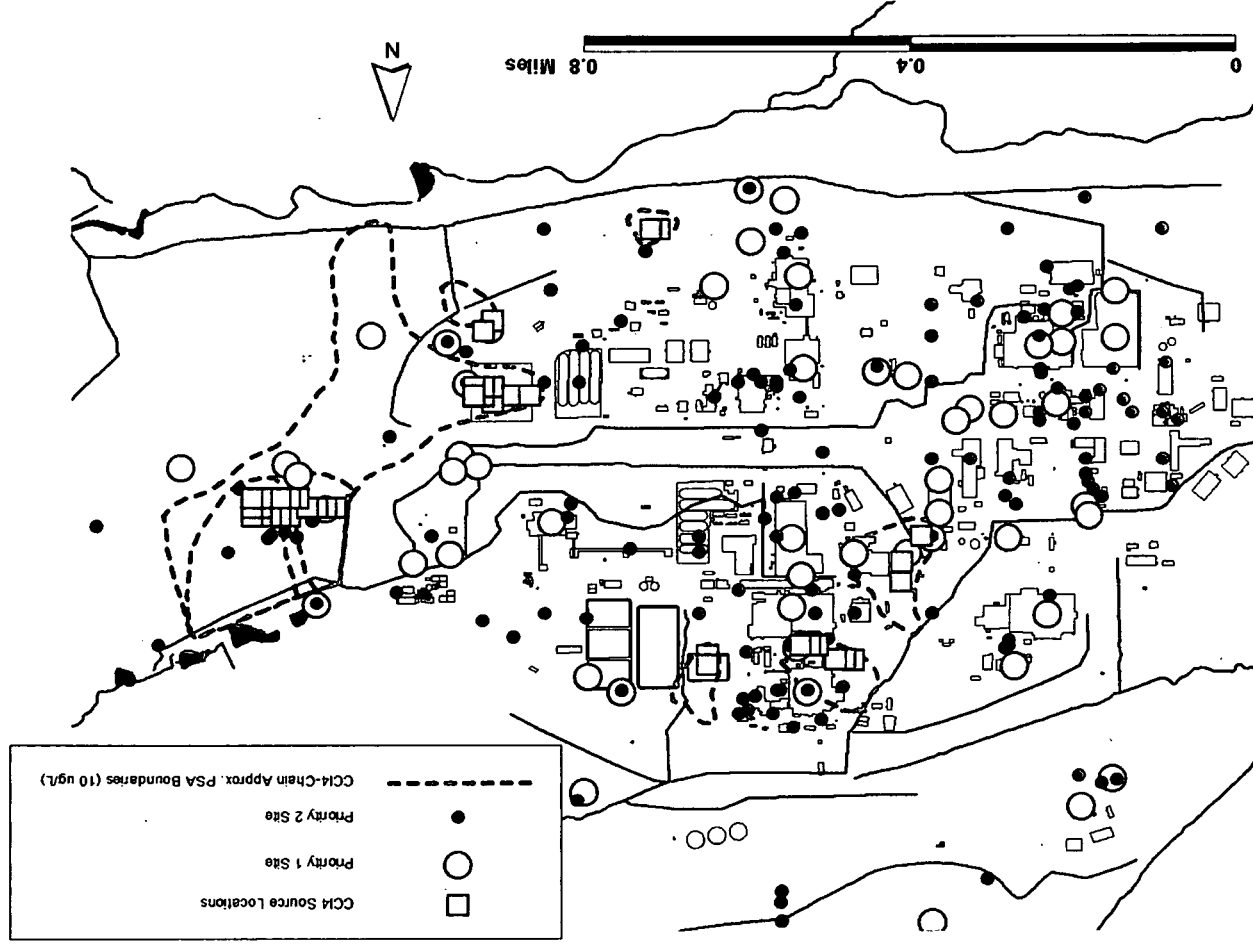


Figure 6.3. Priority 1 and 2 HRR releases with inferred  $\text{CCl}_4$  source locations.

#### 6.1.3.1.1.4 Concentration

Information on groundwater concentrations for inferred VOC sources was unavailable from either the HRR, or sample locations. As a result, a range of effective concentrations at inferred VOC source locations was determined by reproducing time-averaged concentrations using the reactive transport model. This was an iterative process in which effective source concentrations at inferred VOC source locations were first assumed, and then subsequently modified to reproduce the time-averaged groundwater VOC concentrations. Effective VOC source concentrations were kept constant through both historical and closure configuration scenarios given that most well concentrations with time are relatively constant (as discussed above in Section 2.3.4).

Although near-source well concentrations suggested where sources might be inferred, their time-averaged concentrations could not be used to determine effective source concentrations. Part of the reason for this was that near-source concentration information does not adequately represent average, well-mixed, concentrations within either the unconsolidated material or weathered bedrock. In other words, local heterogeneity, well-screening, or DNAPL architecture near inferred VOC sources can cause concentrations in nearby wells to be either higher, or lower, than the actual concentration perpendicular to the plume pathway. As a result, effective source concentrations could be either higher, or lower, than time-averaged concentrations at wells near inferred VOC sources.

#### 6.1.3.1.1.5 Composition

In most instances, the HRR did not provide information on the specific VOCs released into the environment. Instead, the occurrence and relative concentrations of VOCs at samples near inferred VOC sources were used, to the extent possible, to determine likely individual source VOCs (see Figures B-1 to B-7 in Appendix B). In most PSAs, however, individual source VOCs had to be assumed. As a result, particle tracking analysis and transport modeling using historical concentration information were both used to iteratively confirm initial assumptions about source VOCs.

The first step in the assessment of source composition was to identify which of the two degradation chains were observed in the PSA. Often both the PCE and CCl<sub>4</sub> chains were present. Next, parent compounds (PCE and/or CCl<sub>4</sub>) were introduced and simulated over a range of degradation rates. Where this approach could not reproduce observed time-averaged PCE sample concentrations, TCE was systematically co-introduced as a source. Some areas had much higher historical TCE (daughter) concentrations than that of PCE (a possible parent VOC) and it was reasonable to introduce TCE as well as PCE at the source to explain the observed concentration data. In addition, TCE use on the Site was documented. No CCl<sub>4</sub> degradation product sources had to be specified a model to reproduce observed concentrations.

#### 6.1.3.1.2 Degradation Rates

The range of degradation rates initially used in the transport simulations was based on published values from Aronson and Howard (1997) (Table 6.2). Subsequent reactive transport sensitivity simulations reduced the range of possible degradation rates compared to the published ranges (compiled from many different environments). Degradation rates were specified as constant within individual PSA models, but differed from model to model. This was consistent with a Site biodegradation study (Kaiser-Hill, 2003b) that concluded degradation rates likely vary across the Site depending on spatially varying factors such as redox parameters, dissolved oxygen (DO), and observed groundwater contamination. Available data was inadequate to justify varying degradation rates within individual PSA models.

Table 6.2. Published range of degradation rates.

VOC	Range of Degradation Rates Min – Max, Mean (day <sup>-1</sup> )	Reference(s)
PCE → TCE	0 – 0.41, 0.027	Aronson and Howard, 1997
TCE → DCE	0 – 0.19, 0.011	Aronson and Howard, 1997
DCE → VC	NA	NA
VC → Ethene	0 – 0.12, 0.018	Aronson and Howard, 1997
CCl <sub>4</sub> → Chloroform (CHCl <sub>3</sub> )	0 – 1.73, 0.34	Aronson and Howard, 1997
CHCl <sub>3</sub> → Methylene Chloride (CH <sub>2</sub> Cl <sub>2</sub> )	0.004 – 0.24, 0.08	Aronson and Howard, 1997
CH <sub>2</sub> Cl <sub>2</sub> → Methyl Chloride (CH <sub>3</sub> Cl)	NA	NA
CH <sub>3</sub> Cl → Methane (CH <sub>4</sub> )	NA	NA

#### 6.1.3.1.3 Sorption

A range of sorption ( $K_d$ ) values was calculated for each VOC based on published (EG&G, 1994) ranges of Site-specific soil parameters (organic matter content [ $f_{oc}$ ] and clay content) and VOC partitioning constants. A linear sorption isotherm was assumed.



The following equation was applied to calculate the sorption  $K_d$  values:

$$K_d = K_{om} + K_{min} = (f_{oc} \times K_{oc}) + K_{min}$$

Where:

$K_{min}$  = Partitioning coefficient for VOC between water and mineral surfaces

$K_{oc}$  = Partitioning coefficient for VOC between water and organic matter

$f_{oc}$  = Organic fraction

Initial transport simulations used the calculated range, summarized in Table 6.3, were later refined on a PSA-by-PSA basis with the transport simulation results.

Table 6.3. Range of sorption constants.

VOC	Minimum $K_d$ (L/mg)* $F_{oc} = 0.0004$ ; $K_{min} = 0$	Maximum $K_d$ (L/mg) $F_{oc} = 0.0022$ ; $K_{min}$ based on 80% smectite fraction of total clays
PCE	$1.5 \times 10^{-7}$	$3.8 \times 10^{-6}$
TCE	$5.0 \times 10^{-8}$	$3.0 \times 10^{-6}$
cis-1,2-DCE	$2.6 \times 10^{-8}$	$2.3 \times 10^{-6}$
VC	$8.0 \times 10^{-9**}$	$1.7 \times 10^{-6**}$
$CCl_4$	$1.8 \times 10^{-7}$	$4.0 \times 10^{-6}$
$CCl_3$	$1.9 \times 10^{-8}$	$2.5 \times 10^{-6}$
$CCl_2$	$2.8 \times 10^{-9**}$	$1.7 \times 10^{-6**}$
Methyl chloride ( $CH_3CCl$ )	$1.6 \times 10^{-9**}$	$1.0 \times 10^{-6**}$

\*Based on OU1 report (EG&G, 1994), minimum  $f_{oc}$  range and  $K_{oc}$  values also reported in the OU2 report. These  $K_{oc}$  value compared well with values compiled by Schwarzenbach, Gschwend, and Imboden (1993).

\*\* $K_{min}$  and  $K_{oc}$  values were not reported in the OU1 report (EG&G, 1994).  $K_{oc}$  values were assessed by relative comparisons of the octanol-water partitioning coefficients for these VOCs to determine an approximate  $K_{oc}$ .  $K_{min}$  values were similarly approximated for these VOCs based on  $K_{min}$  value trends for the rest of the degradation chain.

#### 6.1.3.1.4 Dispersion

Longitudinal dispersivity was specified in the reactive models based on approximate PSA extents. The transport model calculated a spatially varying dispersion coefficient based on the dispersivity and the velocity from the flow simulation (dispersivity multiplied by average linear velocity). Vertical and cross-flow components were entered as fractions of the longitudinal value. The range of dispersivity values applied to initial simulations was taken from Gelhar et al. (1985), which presents an empirical correlation between plume length and longitudinal dispersion. Observed PSA travel distances at the Site range from 50 to about 500 meters. Based on this, the correlation presented in Gelhar et al. (1985) suggests that the longitudinal dispersivity should be between one and 100

meters. This range was further refined for the Site to between 5 and 30 meters through preliminary reactive transport simulations.

The vertical and cross-flow components were specified as  $1/100^{\text{th}}$  and  $1/10^{\text{th}}$  of the longitudinal dispersivity, respectively. These values were based on the published values in Fetter (1998) and in an RT3D manual (Clement, 2003).

#### **6.1.4 Transport Modeling Approach – Closure Configuration**

For each PSA model, steady, long-term concentrations were estimated at groundwater discharge locations down-gradient of inferred VOC source areas identified through historical transport modeling. Because of the uncertainty in model input parameters that affect the fate and transport of VOCs in groundwater at RFETS, multiple simulations were run to generate a range of possible long-term, steady concentrations at possible groundwater discharge locations. To accomplish this, key flow and transport model parameter values affecting the long-term simulated groundwater discharge location concentrations were varied based on ranges determined in the sensitivity analysis of historical transport modeling (as described in Section 6.1.3).

For the closure configuration simulations, sixteen simulations<sup>2</sup> were run to estimate the range of groundwater discharge location concentrations. Simulations were conducted for both  $\text{CCl}_4$  and/or PCE/TCE-chain, depending on whether maximum observed concentrations were higher than draft surface water PRG values.

## **6.2 Flow and Transport Model Results**

Flow and transport modeling results are summarized in this section. PSA-specific modeling results are described first in Section 6.2.1, while more general findings are discussed in Section 6.2.2. The discussion of PSA-specific modeling results presented in Section 6.2.1 is separated into five different areas. For each PSA model area, the following results are discussed in detail:

- Groundwater flow model results;
- Simulated groundwater VOC sources;
- Particle tracking results – historical conditions;
- Transport model results – historical conditions; and

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<sup>2</sup> Two additional simulations (to those listed in Section 6.1.3) that varied several model parameters were run to observe model response.

- Transport model results – the closure configuration.

## 6.2.1 PSA-Specific Modeling Results

### 6.2.1.1 PSA 2N – 903 Pad /East Trenches Areas

The PSA 2N model area, located in the east-northeast part of the Site, is defined on Figure 6.1. The complexity of the area and results from initial transport modeling led to the development of a focused subset model (PSA 6 and 7) that concentrated on the East Trenches source area. The PSA 6 and 7 model outline is shown on Figure 6.4. The initial results for the larger PSA 2N model area are presented in this section for completeness and to explain the rationale for creating a subset model.

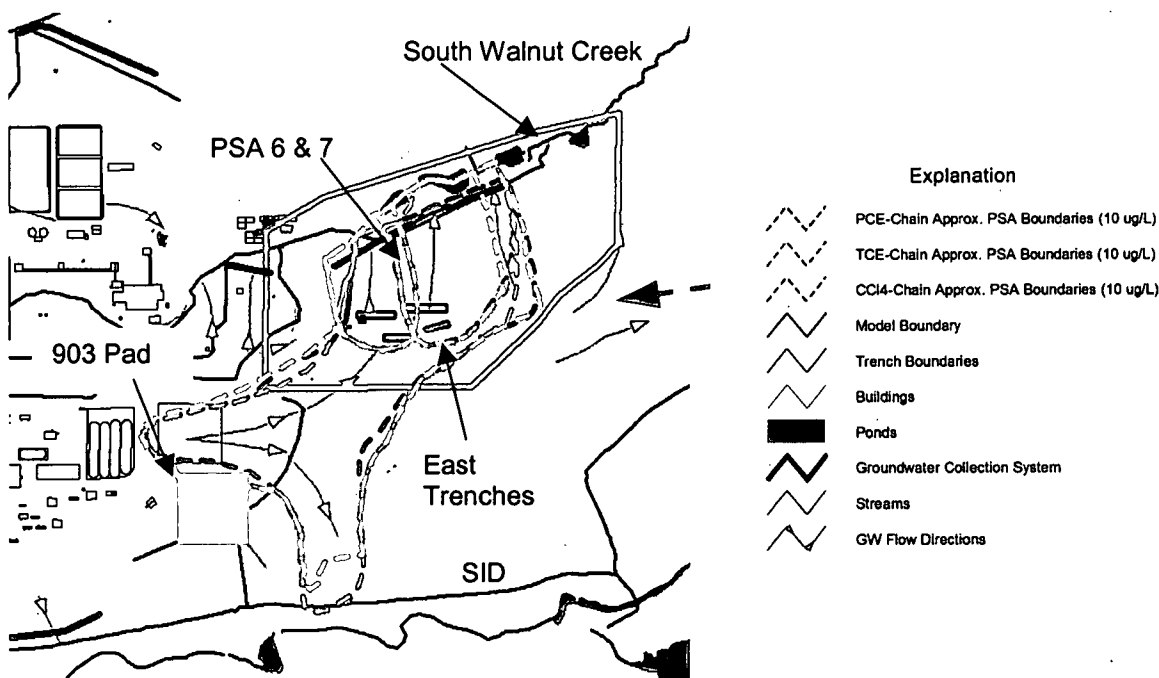


Figure 6.4. PSA 2N - Model area and PSA 2S, PSA 6 and PSA 7.

#### 6.2.1.1.1 Groundwater Flow Model Results

A ground-water divide extends from the 903 Pad to the east-northeast in PSA 2N. Groundwater flows along the divide and then diverges either north to South Walnut Creek (B-Pond Series) or south to the SID and Woman Creek. Because flow conditions in this area are complex, the PSA 2 area was divided into northern and southern models (PSA 2N and PSA 2S) to better simulate the northward and southward components of flow from the divide.

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The water balance calculated by the PSA 2N flow model indicated that water entered the model through constant head boundaries and recharge and left the model by constant head boundaries and ET. Simulated groundwater levels reproduced average annual groundwater levels within 1 to 2 meters (Figure 6.5).

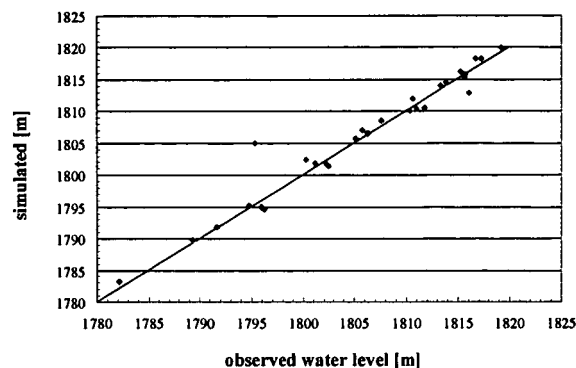


Figure 6.5. PSA 2N - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.1.2 Simulated Groundwater VOC Sources

HRR information suggested probable VOC sources for the PSA 2 (PSA 2N and PSA 2S) area were the 903 Pad area and East Trenches shown on Figure 6.4. Detectable VOC concentrations in down-gradient wells and the B-ponds were attributed primarily to the T-3 and T-4 trenches and possibly the 903 Pad area source (Kaiser-Hill, 1999). Sources were also considered in the T-1 and T-2 trenches.

The two primary inferred VOC source locations in this area (both Priority 1 releases) are the East Trenches (T-1 to T-4) and the 903 Pad. The East Trench sources are documented in the HHR database (Appendix A, reference numbers 277, 279, 280, and 287). The 903 Pad source is documented in the HHR database (Appendix A, reference numbers 34, 39, 178, and 179).

#### 6.2.1.1.3 Particle Tracking Results – Historical Conditions

Particles introduced in the 903 Pad and East Trenches (T-1 through T-4) areas were allowed to travel for 42 years based on the HRR information. Results of the particle tracking, summarized on Figure 6.6, show that simulated flow paths from inferred sources were consistent with the PSA 2N boundary. Particles introduced at the East Trenches (PSA 6 and 7) traveled north and intercepted South Walnut Creek and the B-ponds within the assumed release time. In the southern PSA 2N area, particles traveled from the 903 Pad source to the east and then north past the East Trenches source and commingled with the inferred VOC source particles introduced at the East Trenches. Particles from the 903

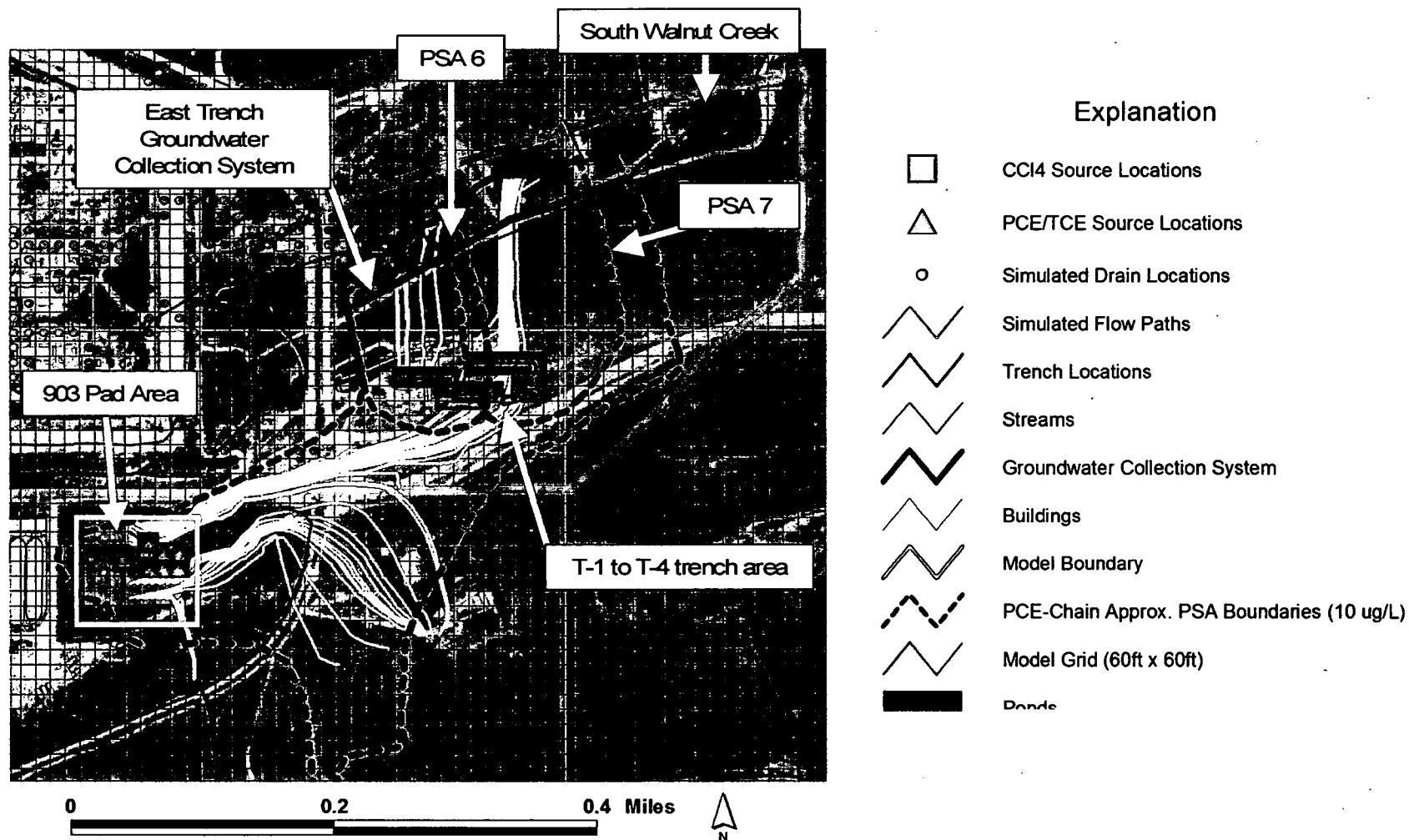


Figure 6.6. PSA 2N (and PSA 6 and PSA 7) - Simulated flow paths (after 42 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

Pad traveled primarily in the saturated higher conductivity unconsolidated material extending eastward along the center of the mesa.

Particles traveling in the lower conductivity weathered bedrock (no Arapahoe Sandstone) from the 903 Pad did not reach the East Trenches area within the time since VOCs were released.

#### **6.2.1.1.4 Transport Model Results – Historical Conditions**

Time-averaged concentrations at sample locations show PCE, TCE, and CCl<sub>4</sub> have been present above their draft surface water PRGs. TCE concentrations are the highest of the VOCs in the area and when coupled with a relatively low PRG, 0.19 mg/L, TCE is the contaminant most likely to reach a groundwater discharge area at or above its PRG.

The initial PSA 2N model reproduced overall average concentrations reasonably well, but failed to reproduce the high concentrations at down-gradient wells near the discharge area (23097, 23197, and 23397 along the eastern part of the East Trench groundwater collection system).

A subset model (PSA 6 and 7) was developed to better simulate down-gradient concentrations in the East Trenches source area. This model was based on particle flowpath results that indicated flow from the 903 Pad source travels to, and commingles with, the East Trenches source.

#### **6.2.1.1.5 Transport Model Results – Closure Configuration**

Sixteen closure configuration simulations were not run for the PSA 2N model because initial modeling showed that long-term concentrations from the 903 Pad area would not result in down-gradient groundwater discharge concentrations greater than draft PRGs. Instead, the closure configuration simulation for the area was modeled using the PSA 6 and 7 sub-scale model.

#### **6.2.1.2 PSA 6 and 7 (East Trenches Area – Sub-Scale Model)**

A sub-scale model within the PSA 2N area was developed to better simulate several high concentration wells down-gradient of the East Trench groundwater collection system (PSAs 6 and 7 defined on Figures B-1 through B-7, in Appendix B). Though the 903 Pad area is not included in the sub-scale model, transport from the 903 Pad source was included using specified concentrations along the southwestern model boundary. The hydraulic conductivity distribution imported from the integrated flow model was modified based on conversations with SMEs involved with the construction of the East Trenches Groundwater Collection System and in an East Trenches Plume report (Kaiser-Hill, 1999). A 100 foot-wide zone of increased permeability produced higher discharge rates during installation of the East Trenches Groundwater Collection System. For the sub-scale model, a preferential pathway was assumed to occur between the T-1

to T-4 trench area (Figure 6.6) and the set of observed high concentration wells (up to 6 mg/L) used to place the East Trench system. This zone was also assumed to extend at least to the B-2 Pond to account for observed VOC concentrations in this area. The hydraulic conductivity of this zone was increased only moderately above the average Arapahoe Sandstone unit value. This is reasonable given that the Arapahoe Sandstone hydraulic conductivity values range more than one order of magnitude over the Site (Kaiser-Hill, 2002). It should be recognized that the extents and depth of this higher permeability zone were assumed for this model only to the extent that they accounted for the higher concentration wells associated with Trench alignment. Effects of the assumed zone's northern and southern extent, connection to the B-ponds, vertical extent on transport, or flow in the area were not evaluated in this study.

#### 6.2.1.2.1 Groundwater Flow Model Results

Simulated groundwater levels compared well against observed well water levels (Figure 6.7), though some residuals range up to 5 meters. This is likely due to the increased hydraulic conductivity zone, and assumptions about how flow in this bedrock zone discharges to the East Trench system, the B-2 Pond, and unconsolidated materials in the area.

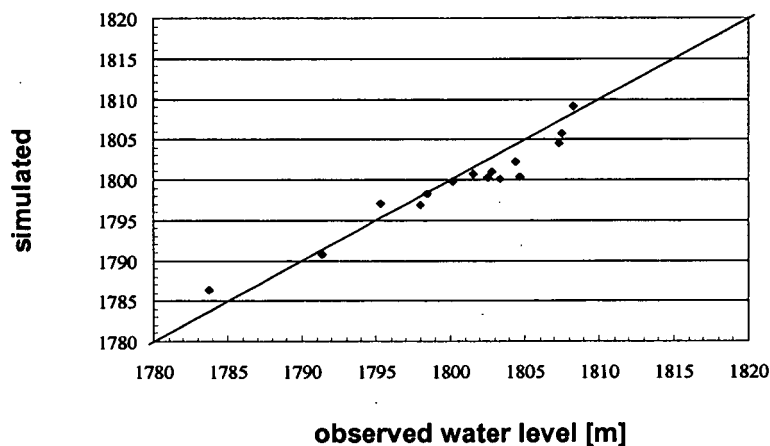


Figure 6.7. PSA 6 and 7 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.2.2 Particle Tracking Results – Historical Conditions

In contrast to the 903 Pad area, particles introduced in the PSA 6 and PSA 7 source areas (East Trenches, shown on Figure 6.6) preferentially traveled northward within the Arapahoe Sandstone layer, despite having been introduced into the lower unconsolidated material and upper weathered bedrock layers. This is partly due to the increased hydraulic conductivity zone that locally concentrates particles from the T-3/T-4 trenches towards the down-gradient high

concentration wells. It is also due to portions of the T-1 to T-4 trenches that occur in areas where Arapahoe Sandstone subcrops beneath the unconsolidated material. As a result, it was hypothesized that a constant groundwater source at any of these trenches did not collect at the weathered bedrock interface, but instead moved into the bottom of the Arapahoe Sandstone, where it collected at the sandstone/claystone interface and resulted in a constant dissolved-phase VOC source.

Simulated transport pathways from the T-1 through T-4 trench area were also strongly controlled by the extent, orientation, and degree of subcropping of the Arapahoe Sandstone in this area. The thin unconsolidated material along the northern mesa hillside remained unsaturated, in part due to greater lateral drainage of the higher conductivity underlying Arapahoe Sandstone in this area.

This flow model was used for transport simulations for the PSA 2N area because it reproduced the observed concentration distribution better than the larger-scale PSA 2N model described previously.

#### **6.2.1.2.3 Transport Model Results – Historical Conditions**

Time-averaged concentrations at sample locations show PCE and TCE are present above their draft surface water PRGs. PCE and TCE were detected above draft surface water PRG levels (4.7 mg/L and 221 mg/L, respectively) within the PSA 6 and 7 areas. CCl<sub>4</sub> was detected above its draft surface water PRG level within the PSA 6 and 7 areas with individual sample results ranging to 3.8 mg/L. The high TCE concentrations combined with a relatively low PRG (0.19 mg/L for TCE versus 1.46 mg/L for PCE and 0.58 mg/L for CCl<sub>4</sub>) makes it the contaminant most likely to reach a groundwater discharge area at or above its PRG.

The East Trenches Groundwater Collection System influences transport modeling in this area. The system was installed between February, and August, 1999 to intercept contamination migrating from the East Trenches to the discharge area. The modeling used time-averaged concentrations from wells prior to installation of the groundwater collection system for comparison to maximize the number of available data points. Several wells in the area with high concentrations (23097, 23197, 23397, and 23597) were installed to locate the system and were sampled only once in 1998. The wells located beyond the groundwater collection system nearer the discharge area showed post-remediation concentrations at the low end of the range of seasonal variability. As of 2002, the TCE concentration of influent to the system was in the 1 to 2 mg/L range. A higher resolution model would be necessary to evaluate groundwater collection system performance.

Simulations including the high conductivity zone reproduced the observed average concentrations well throughout the model area, including lower concentrations to the west of the zone. Sensitivity runs using more conservative



and less conservative parameter values bracketed the time-averaged concentration distribution. The TCE results for high concentration and low concentration simulations are shown on Figure 6.8 and Figure 6.9.

The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain; CCL<sub>3</sub> and CCL<sub>2</sub> for the CCl<sub>4</sub> degradation chain) was bracketed by the various sensitivity runs with the high source concentration (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain are shown on Figure D-1 and Figure D-2 of Appendix D. There were only three locations with low concentrations of VC, so only cis-1,2-DCE is shown for the PCE chain.

#### **6.2.1.2.4 Transport Model Results – Closure Configuration**

Under current conditions in the PSA 6 and 7 model area, groundwater flows in the lower weathered bedrock (Arapahoe Sandstone), north towards South Walnut Creek. Because the area did not change significantly under the proposed closure configuration flow conditions in the PSA 6 and 7 areas were largely unaffected.

Fourteen transport model sensitivity runs were simulated for the PCE/TCE and CCl<sub>4</sub> VOC chains. Due to a low water table at source cells in the upper weathered bedrock, the upper layer source scenario was not modeled. Due to numerical problems (high mass balance errors), the low dispersion case was also excluded. All runs were simulated for at least 100 years to establish steady long-term concentrations with time at the PSA extent along south Walnut Creek to the north.

The average simulated PCE and TCE groundwater concentration for all model runs at discharge areas are shown on Figure 6.10 and Figure 6.11. There were no locations where simulated PCE, or CCl<sub>4</sub> were above draft PRG values at groundwater discharge locations within the PSA 6 or 7 areas. Average simulated groundwater TCE concentrations, however, for all 14 runs at several groundwater discharge locations were above draft surface water PRG levels (0.19 mg/L) immediately down-gradient of the modeled high conductivity zone (Figure 6.11). This was consistent with the time-averaged concentrations in pre-trench wells near the high conductivity zone. Groundwater TCE concentrations were simulated above draft PRG values at the B-1 and B-2 Ponds.

The bar charts on Figure 6.12 show results for discharge locations impacted by CCl<sub>4</sub>. Concentrations at these locations ranged from non-detect in some sensitivity runs, to near the draft surface water PRG (0.58 mg/L) for the high concentration case. This was consistent with time-averaged concentrations and was expected from the conceptual model of the area. Unlike PCE/TCE, the highest concentration source for CCl<sub>4</sub> was in PSA 6, west of the high conductivity zone that influences the higher PCE/TCE concentrations in PSA 7 (Appendix B).

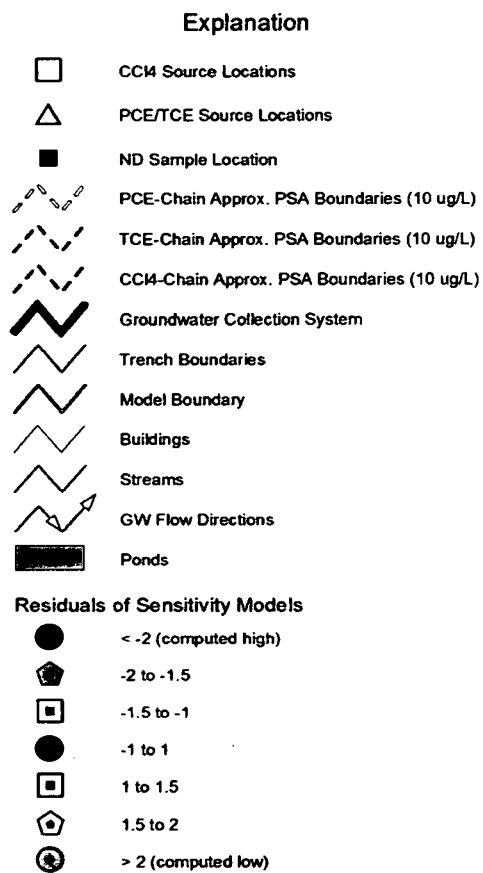


Figure 6.8. PSA 6 and 7 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (high hydraulic conductivity).

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Explanation

- CCM Source Locations
- △ PCE/TCE Source Locations
- ND Sample Location
- - - PCE-Chain Approx. PSA Boundaries (10 ug/L)
- - - TCE-Chain Approx. PSA Boundaries (10 ug/L)
- - - CCM-Chain Approx. PSA Boundaries (10 ug/L)
- Groundwater Collection System
- Trench Boundaries
- Model Boundary
- Buildings
- Streams
- GW Flow Directions
- Ponds
- Residuals of Sensitivity Models
  - < -2 (computed high)
  - ◐ -2 to -1.5
  - ◑ -1.5 to -1
  - -1 to 1
  - ◐ 1 to 1.5
  - ◑ 1.5 to 2
  - > 2 (computed low)

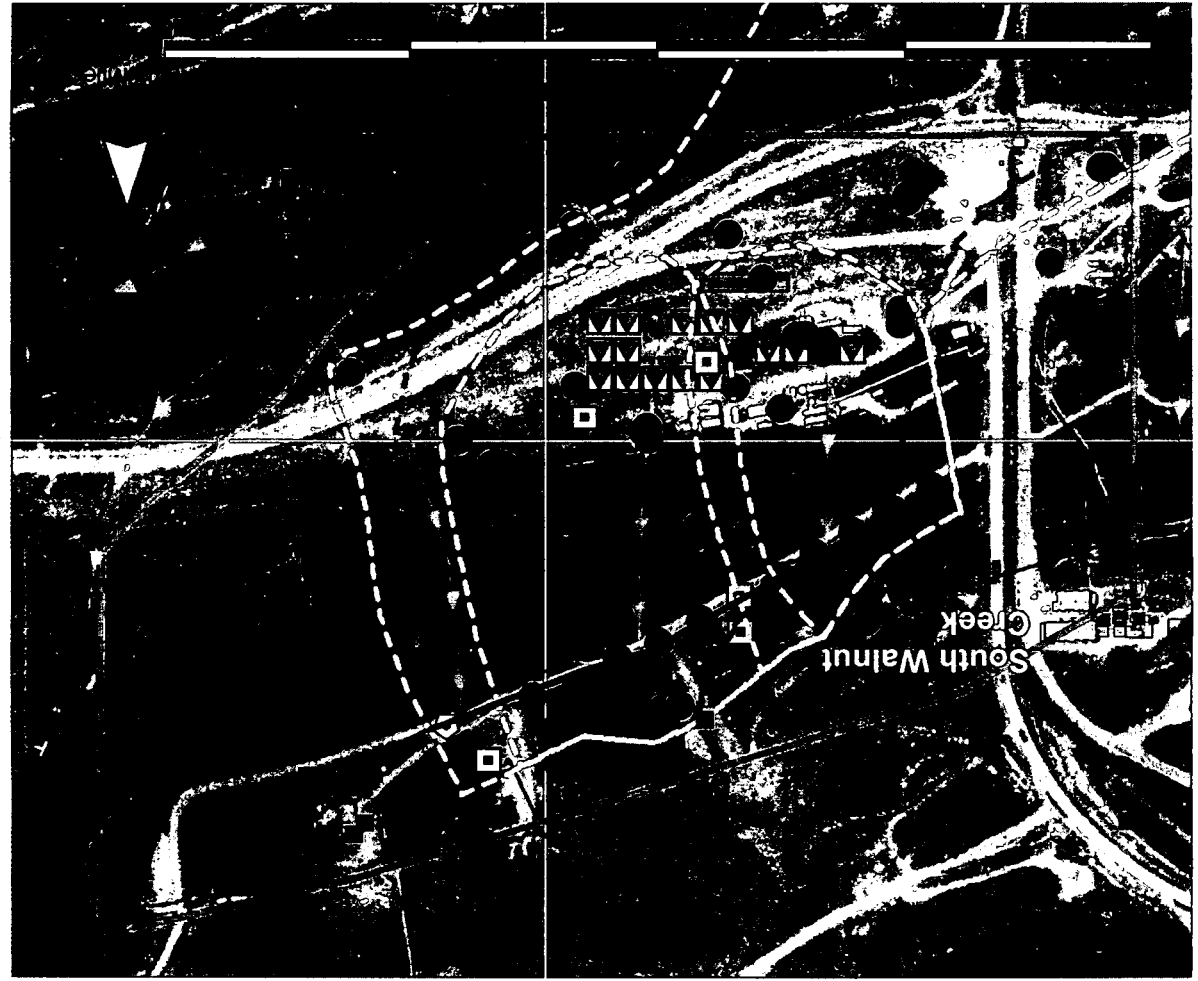


Figure 6.9. PSA 6 and 7 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (low source concentration).

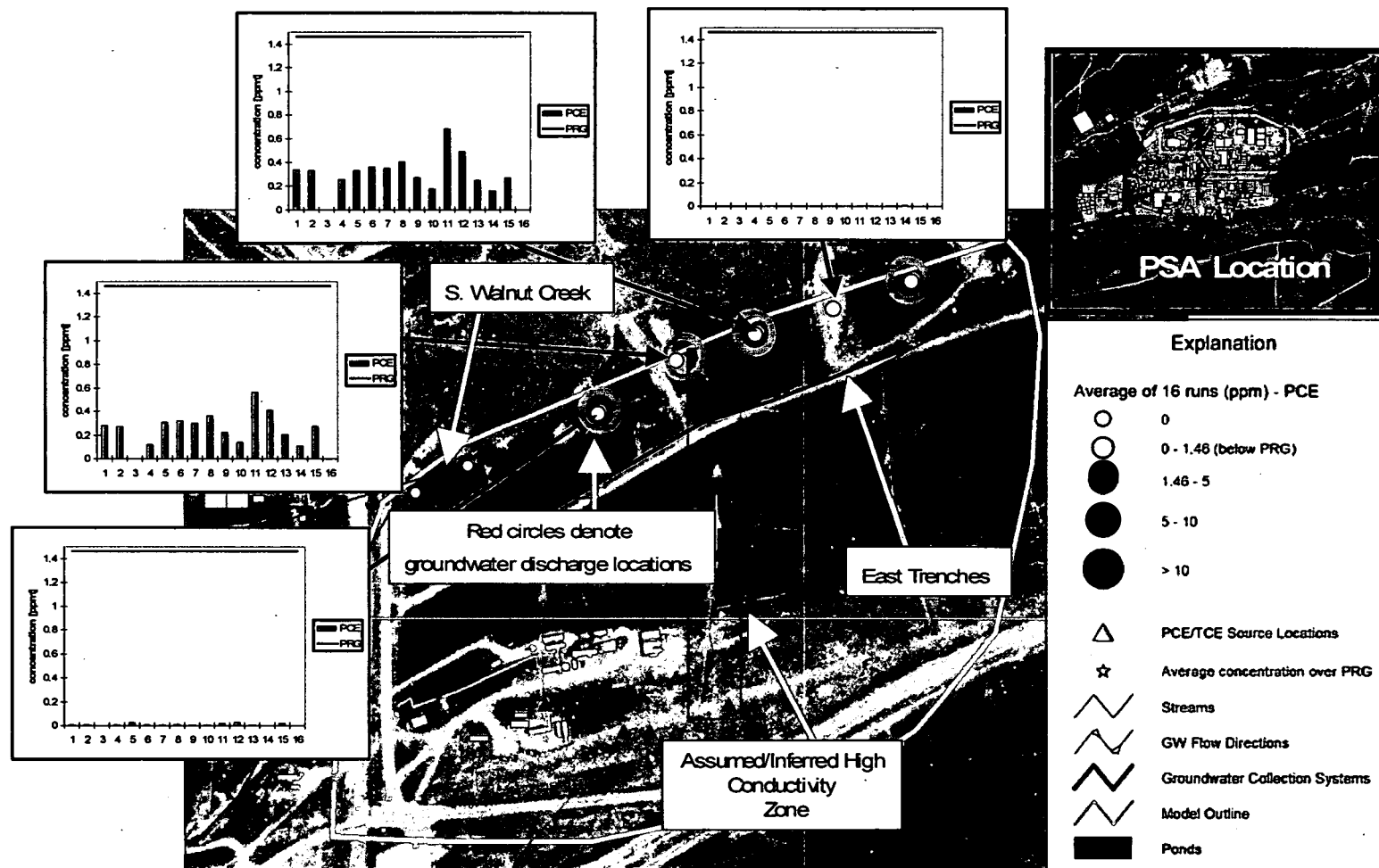


Figure 6.10. PSA 6 and 7 - Simulated PCE groundwater concentrations at discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

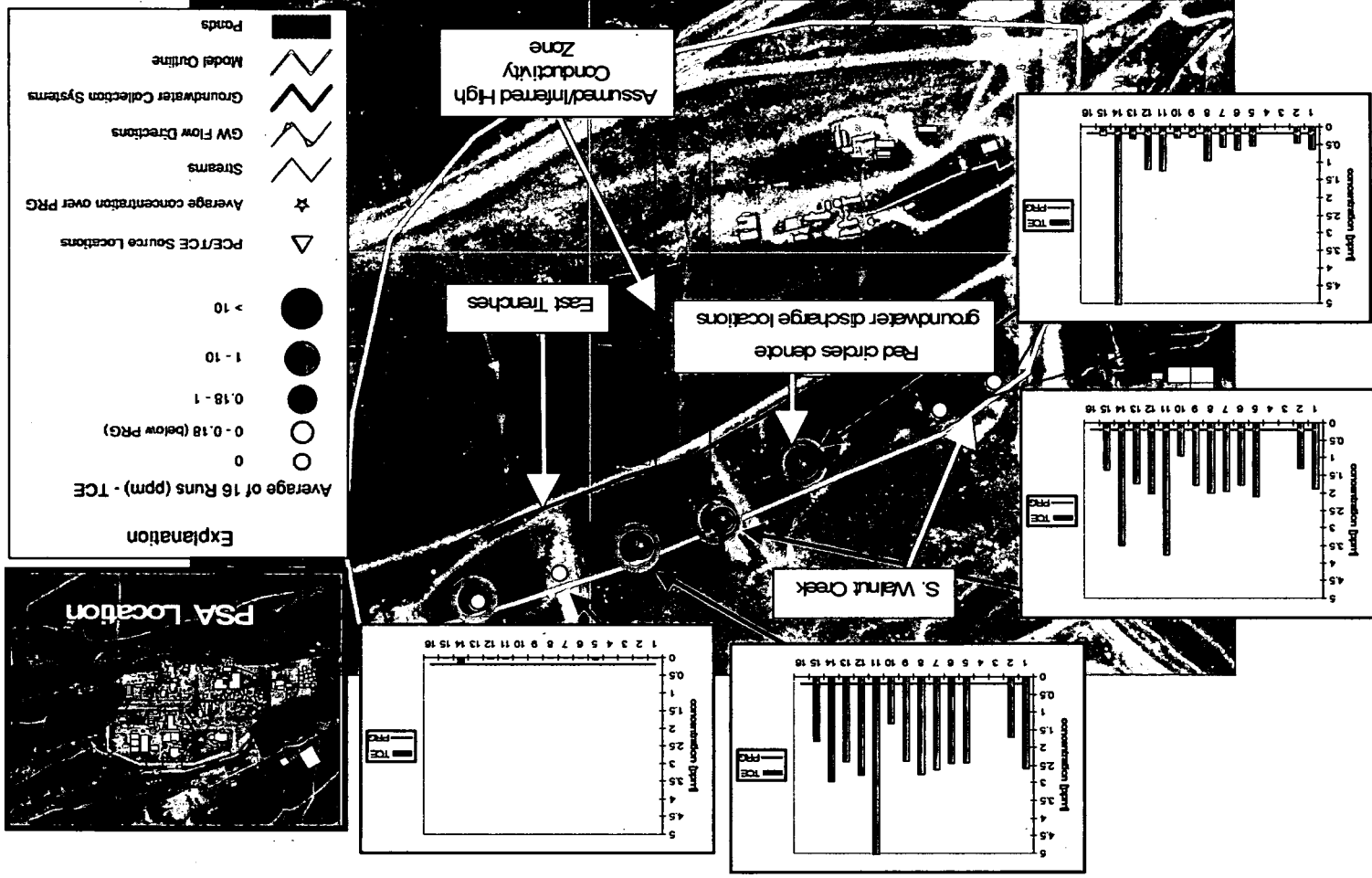


Figure 6.11. PSA 6 and 7 - Simulated TCE groundwater concentrations at discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

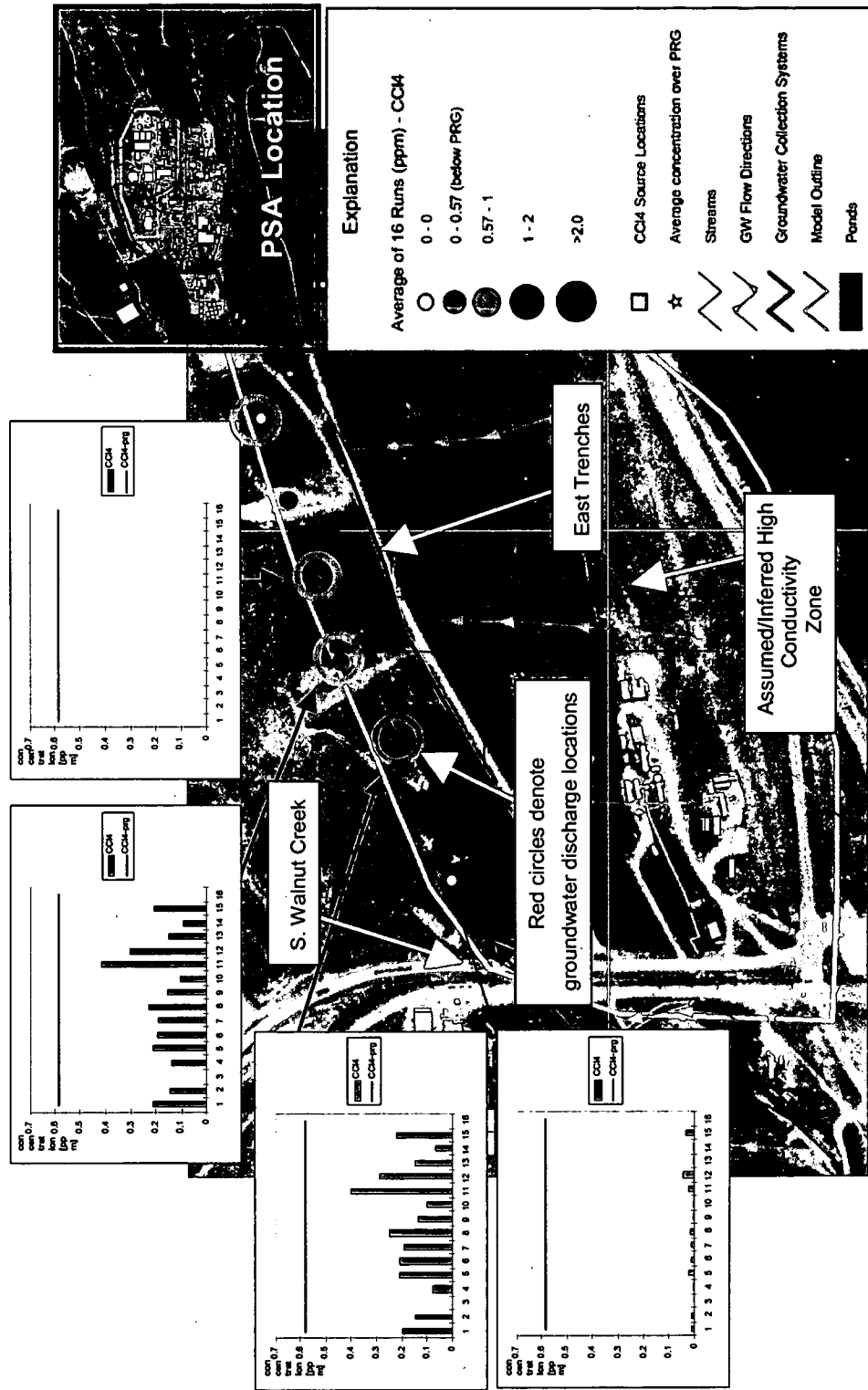


Figure 6.12. PSA 6 and 7 - Simulated CCl<sub>4</sub> groundwater concentrations at discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

### **6.2.1.3 PSA 2S (Ryan's Pit/Southern 903 Pad Area)**

The PSA 2S model area is located in the southeastern part of the VOC study area and is defined on Figure 6.1 and Figure 6.13.

#### **6.2.1.3.1 Groundwater Flow Model Results**

Constant hydraulic heads were specified along PSA 2S model boundary cells for all four saturated zone layers (Figure 6.13). Constant hydraulic heads along the Woman Creek boundary were assigned values near ground surface. This was valid because they represent the lowest heads in the model area and did not affect up-gradient flow calculations.

Average annual groundwater levels in the model area were reproduced reasonably well (Figure 6.14). Residuals ranged from 1 to 2 meters.

Simulated model water balance results showed that water entered the model primarily as direct recharge and to a lesser extent by lateral inflow (i.e., constant head cells). Discharge occurred mostly by ET near Woman Creek. Integrated flow model results indicated only localized and intermittent (high groundwater levels) discharge occurred to the SID in this model area.

#### **6.2.1.3.2 Simulated Groundwater VOC Sources**

The two VOC source locations inferred for the PSA 2S area were associated with the 903 Pad and Ryan's Pit areas. They were documented as Priority 1 releases in the HRR database (Appendix A, reference numbers 34, 39, 178, 179, and 30, 31, 32, and 33). Solvents, paint thinners, diesel fuel, and other construction related chemicals/materials were disposed of at trench T-2 (Ryan's Pit) until 1970. The pit was backfilled in 1971. In the 903 Pad area, drums were stored (IHSS 112 and 155) from 1958 to 1967. Leaking drums were noted in 1964. They were removed and the asphalt pad was completed in 1969. Despite soil remediation in the area, it was hypothesized that VOCs released in the area likely migrated downward into the groundwater as DNAPL and collected above the weathered bedrock (claystone in this area). It is also possible that DNAPL entered the weathered bedrock and acts as a constant dissolved-phased VOC source in groundwater given the steady time-averaged concentrations in down-gradient wells.

#### **6.2.1.3.3 Particle Tracking Results – Historical Conditions**

Inferred VOC sources were simulated in both the lower unconsolidated deposits and in the upper weathered bedrock at both the 903 Pad and at Ryan's Pit at locations shown on Figure 6.15. The long extent of PSA 2S also supported introducing a source into the unconsolidated material so that simulated VOCs would reach the extent of the PSA. Multiple inferred VOC source locations were evaluated in the 903 Pad area.

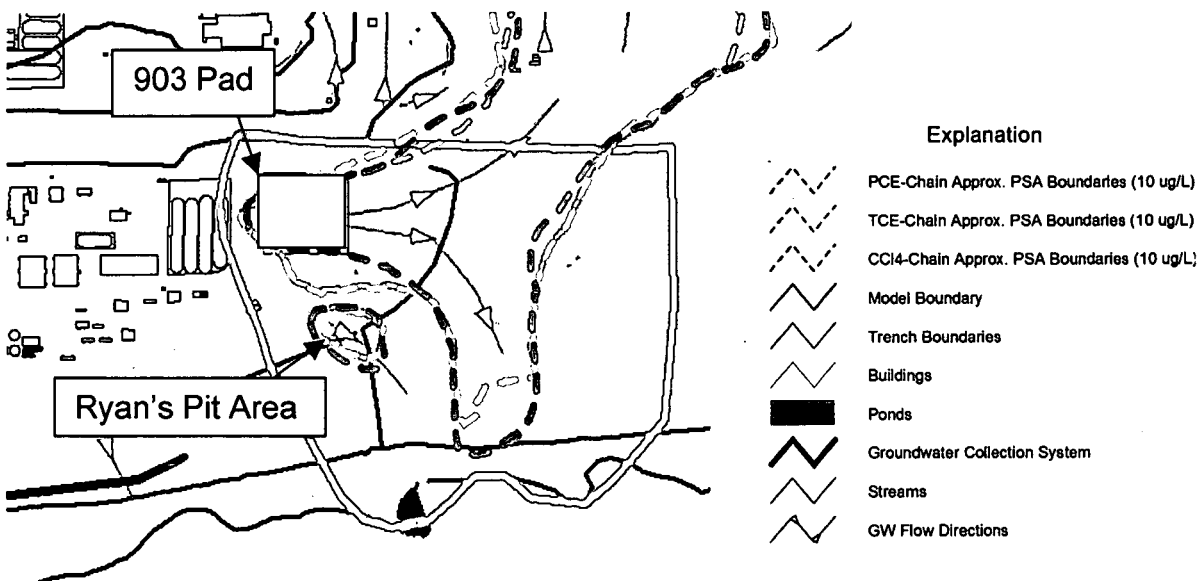


Figure 6.13. PSA 2S – Model area and PSAs

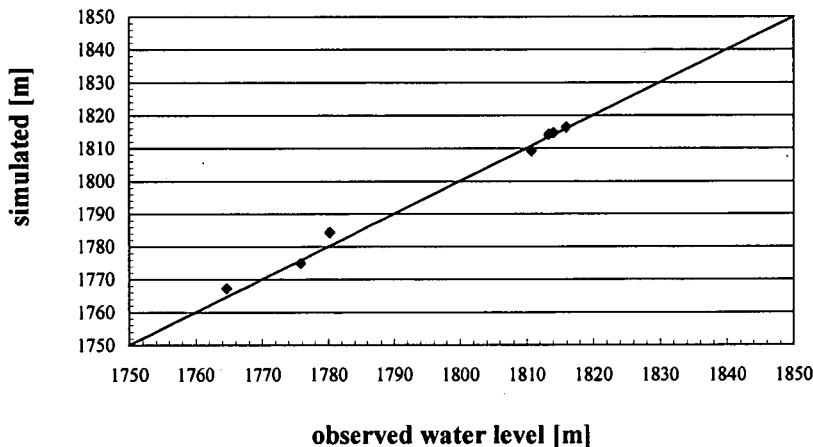


Figure 6.14. PSA 2S - Simulated versus average observed annual groundwater levels (meters above msl).

Calculated flow paths and travel distances agreed well with the inferred VOC source locations and release times for PSA 2S and PSA 3 (Ryan's Pit) (Figure 6.15). Particles introduced in the lower unconsolidated material traveled to the PSA 2S and PSA 3 extents within the assumed 42-year travel time. Particles introduced into the upper weathered bedrock did not reach the assumed PSA extent just above the SID because the hydraulic conductivity of the weathered bedrock in this area is low compared to the unconsolidated material. Results also showed that some of the particles introduced into the lower unconsolidated material migrated into the lower weathered bedrock layer east of the 903 Pad area due to downward hydraulic gradients near the assumed sources.

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Particle paths were affected by two main factors, unconsolidated thickness and the degree to which the unconsolidated material is unsaturated. The unconsolidated material thickness increases east of the 903 Pad. In the flow model, this caused groundwater flows to be directed eastward first, then to the southeast towards Woman Creek. As particles traveled within the hillslope towards Woman Creek flow paths became irregular (Figure 6.15). This was because the unconsolidated material is unsaturated in this area due to shallower depths to bedrock. Results showed that particle pathways are consistent with the assumed PSA 2S extents.

#### **6.2.1.3.4 Transport Model Results – Historical Conditions**

Time-averaged concentrations at sample locations show PCE, TCE, and CCl<sub>4</sub> are present above their draft surface water PRGs. Time-averaged TCE concentrations are highest in PSA 2S, especially in the Ryan's Pit area. As a result, TCE is the contaminant most likely to reach groundwater discharge areas at or above its PRG.

Sensitivity simulations indicated that time-averaged TCE concentrations within PSA 2S could be reproduced (as shown on Figures 6.16 and 6.17). Results indicated that the high hydraulic conductivity value sensitivity run produced conservatively high concentrations at sample locations. The increased groundwater velocities, due to higher conductivity values, produced higher concentrations at down-gradient locations, and prevented more significant degradation losses. Specifying low hydraulic conductivity values under-predicted time-averaged concentrations within the PSA.

The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain; chloroform and methylene chloride for the CCl<sub>4</sub> degradation chain) was bracketed by the various sensitivity runs. These sensitivity runs included high conductivity and high degradation (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain. The results are shown on Figure D-3 and Figure D-4 of Appendix D.

#### **6.2.1.3.5 Transport Model Results – Closure Configuration**

Flow conditions did not change significantly under the closure design (except for the 903 Pad area).

Sixteen transport simulations were simulated for the PCE/TCE degradation chain. All runs were simulated for 450 years to establish steady long-term concentrations with time at the PSA extent along Woman Creek to the south. Closure configuration simulations indicated steady TCE concentrations, at groundwater discharge locations, were attained after hundreds of years. One of the reasons it takes so long for this PSA to reach steady conditions is the relatively slow travel velocity in the bedrock and longer distances from the source

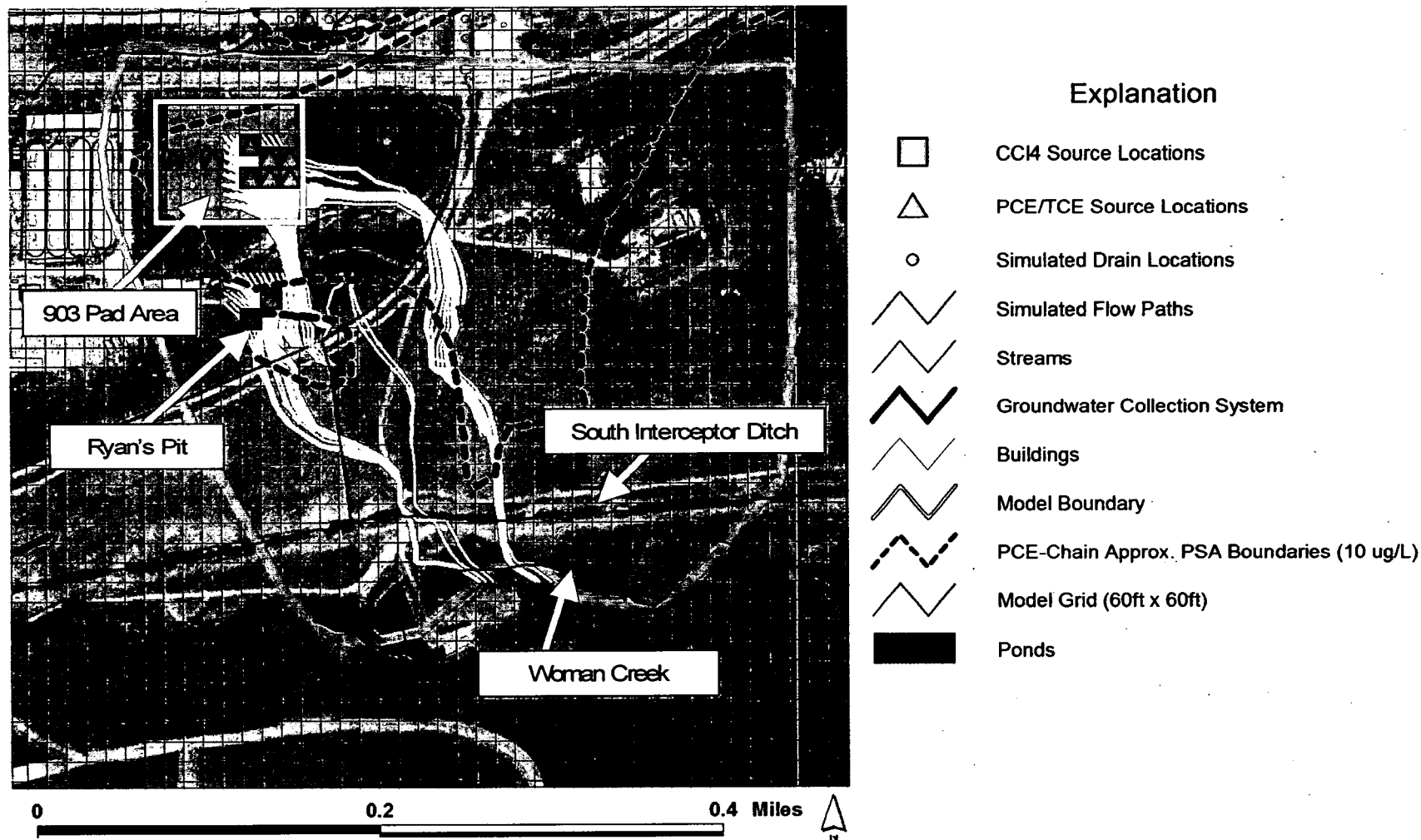


Figure 6.15. PSA 2S - Simulated flow paths (after 42 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

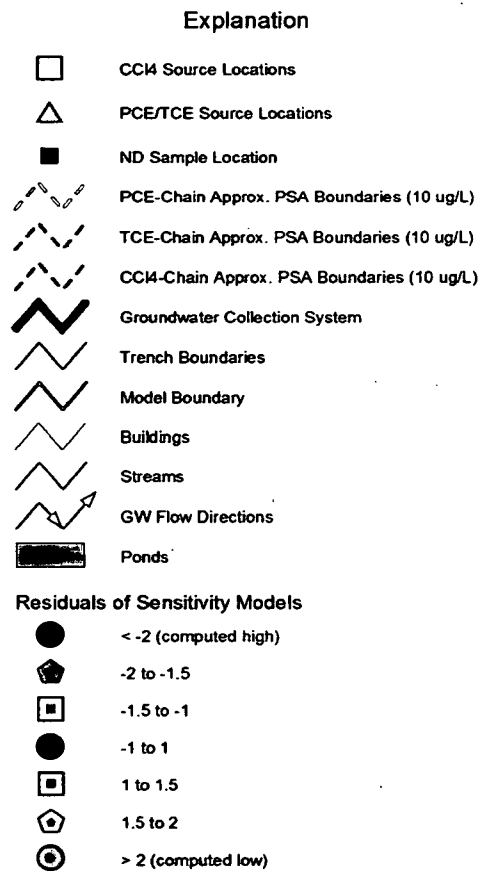


Figure 6.16. PSA 2S – Log residual concentrations. High predicted concentration (TCE) sensitivity run (high hydraulic conductivity).

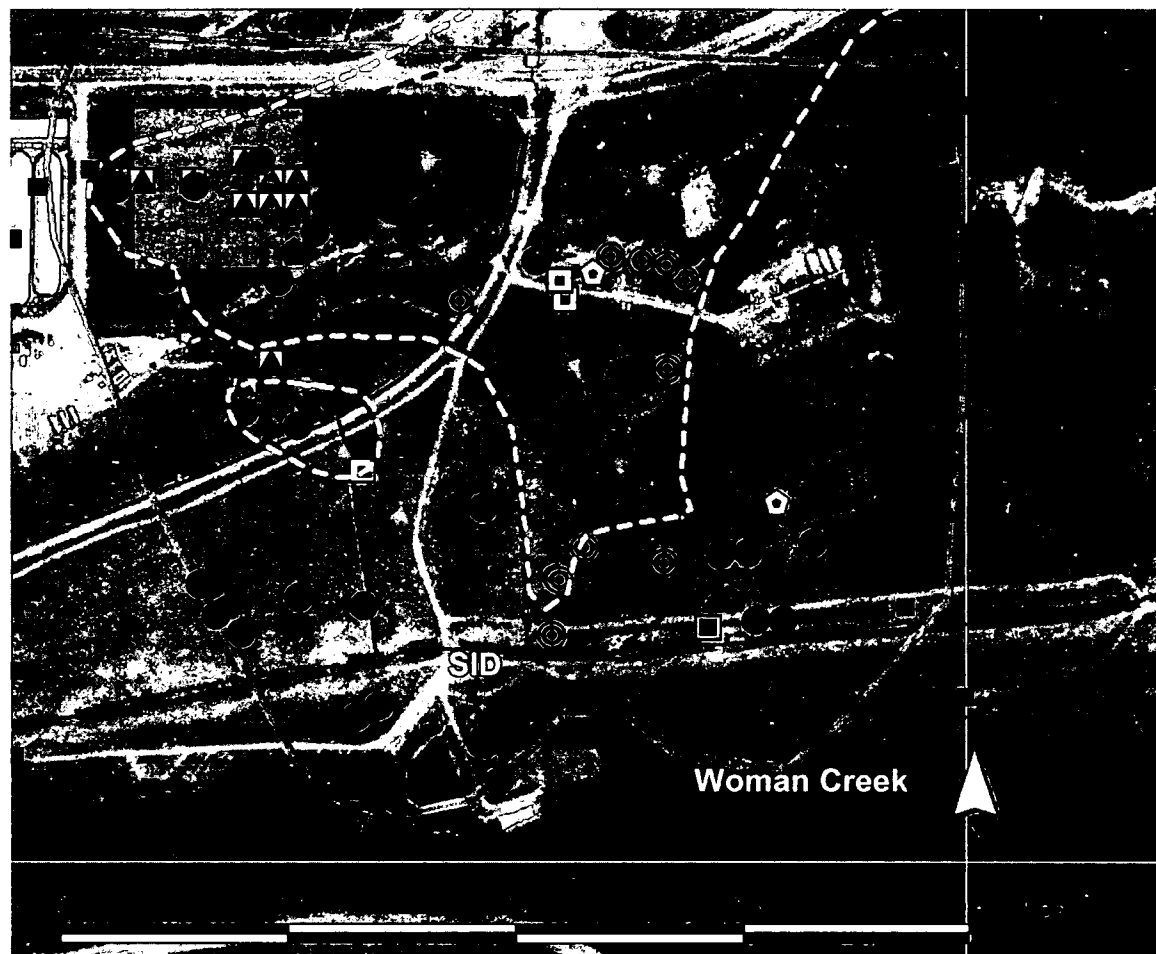
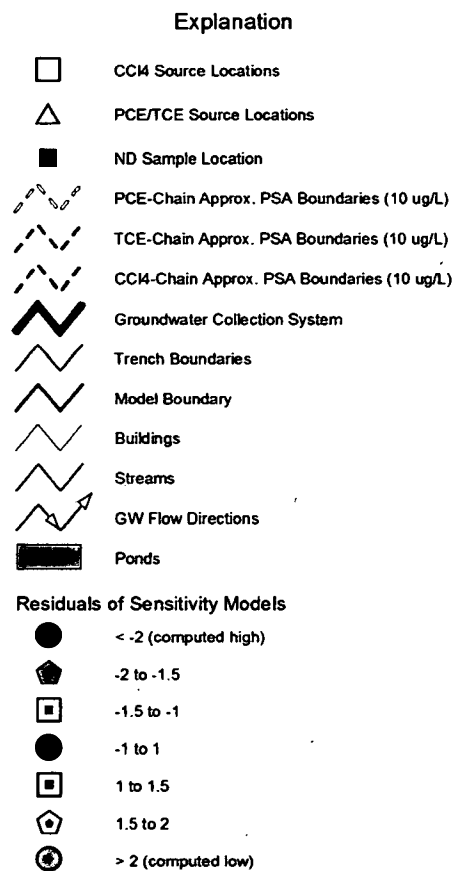


Figure 6.17. PSA 2S – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (low hydraulic conductivity).

areas to stream locations. The average simulated groundwater concentration for each of the model runs at groundwater discharge areas for TCE is shown on Figure 6.18.

Sixteen transport simulations were also simulated for the  $\text{CCl}_4$  degradation chain. All runs were simulated for at least 450 years to establish steady long-term concentrations with time at the PSA extent along Woman Creek to the south. The average simulated groundwater concentration for each of the model runs at groundwater discharge areas for  $\text{CCl}_4$  is shown on Figure 6.19.

The bar charts on Figure 6.18 and Figure 6.19 show long-term simulated groundwater discharge concentrations for the 16 closure configuration runs for TCE in the PSA 2S area. Simulated TCE concentrations (averaged from all 16 runs), were greater than draft surface water PRG (0.19 mg/L) at a number of locations along the dirt road-side drainage ditch that runs northeast to southwest, down-gradient from the 903 Pad. Integrated flow modeling results, however, indicated that discharge only occurred along this drainage near its confluence with the SID. The average of all the simulated closure-condition TCE and  $\text{CCl}_4$  groundwater concentrations at discharge locations along Woman Creek were below draft surface water PRG.

Mass flux from the transport model indicated that the simulated dominant loss mechanisms for VOCs were ET and biodegradation (Figure 6.20 and 6.21).

#### **6.2.1.4 PSA 5 (Mound Groundwater Collection System Area)**

The PSA 5 model area is located in the northeast part of the Site and is defined on Figure 6.1 and Figure 6.22.

##### **6.2.1.4.1 Groundwater Flow Model Results**

Constant head boundaries for PSA 5 were placed at the western and southern up-gradient boundaries where flow enters the model, as well as the northern down-gradient boundary where flow exits the model. Results of the simulated water balance indicated most inflow occurred via recharge, and less occurred through up-gradient constant head cells. Groundwater discharged as baseflow, ET, and as groundwater to the East Mound collection system. Simulated heads compared well with observed groundwater water levels (as shown on Figure 6.23). Residuals ranged from one to two meters.

125

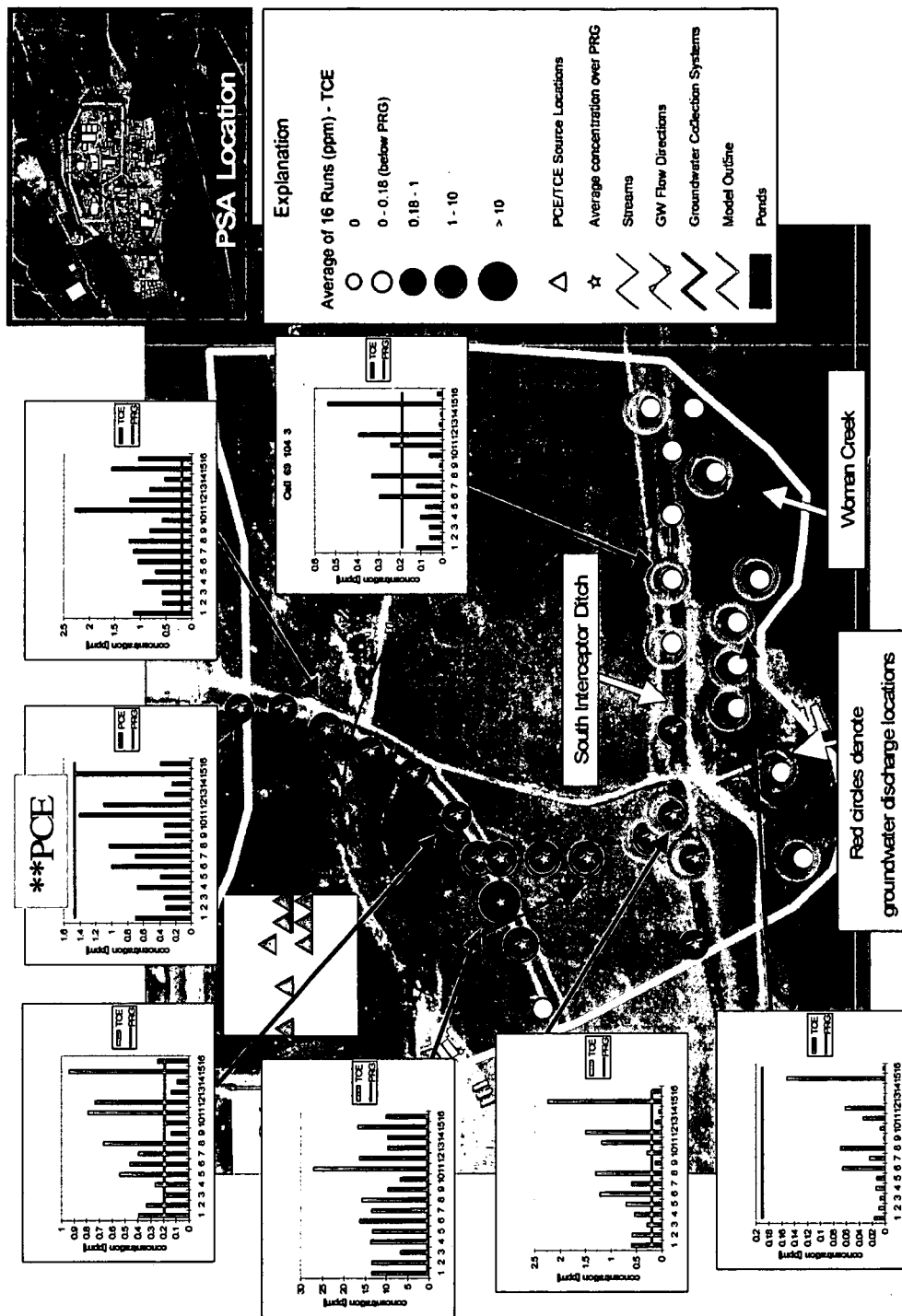


Figure 6.18. PSA 2S - Simulated TCE groundwater concentrations at discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

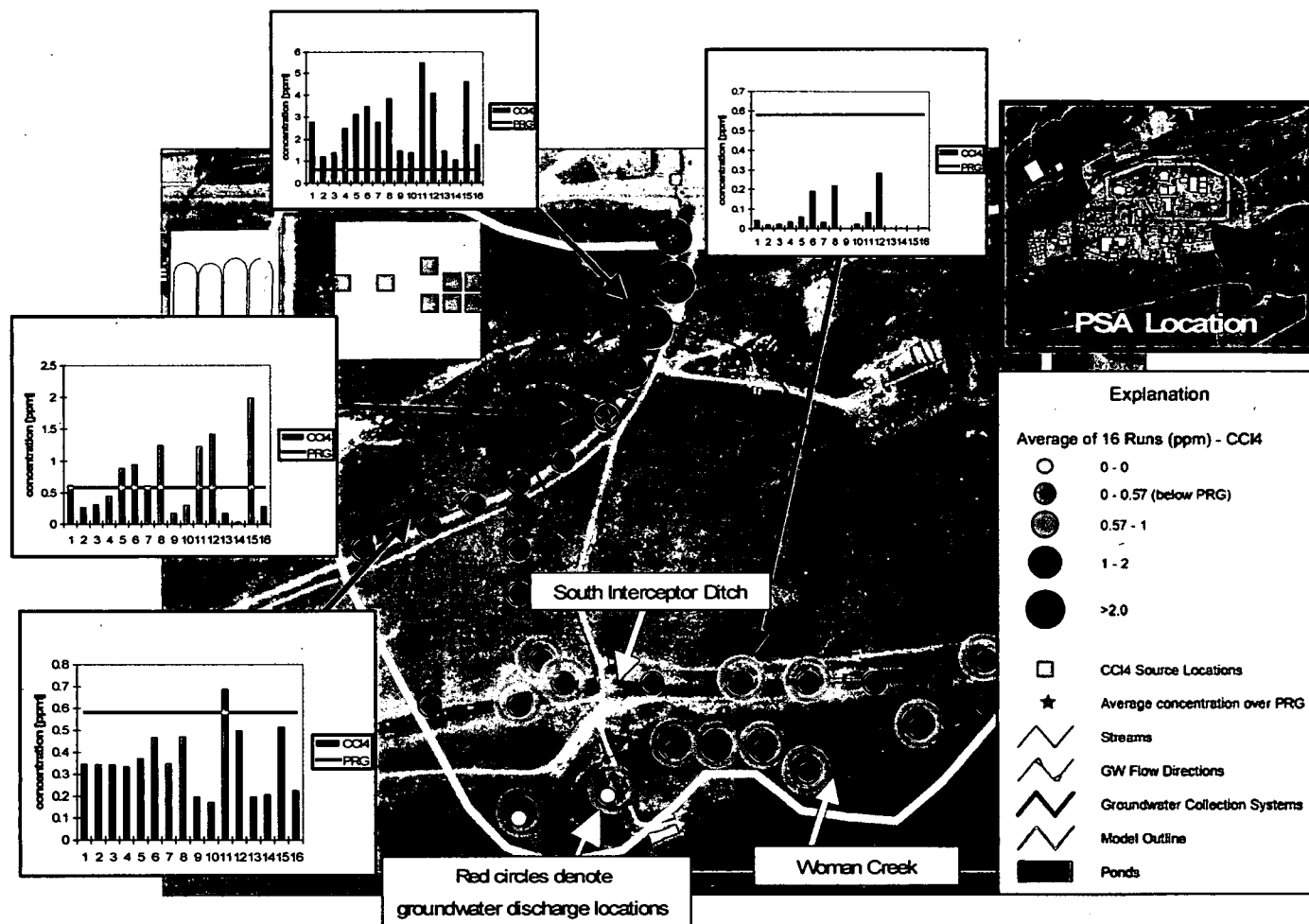


Figure 6.19. PSA 2S - Simulated  $\text{CCl}_4$  groundwater concentrations at discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

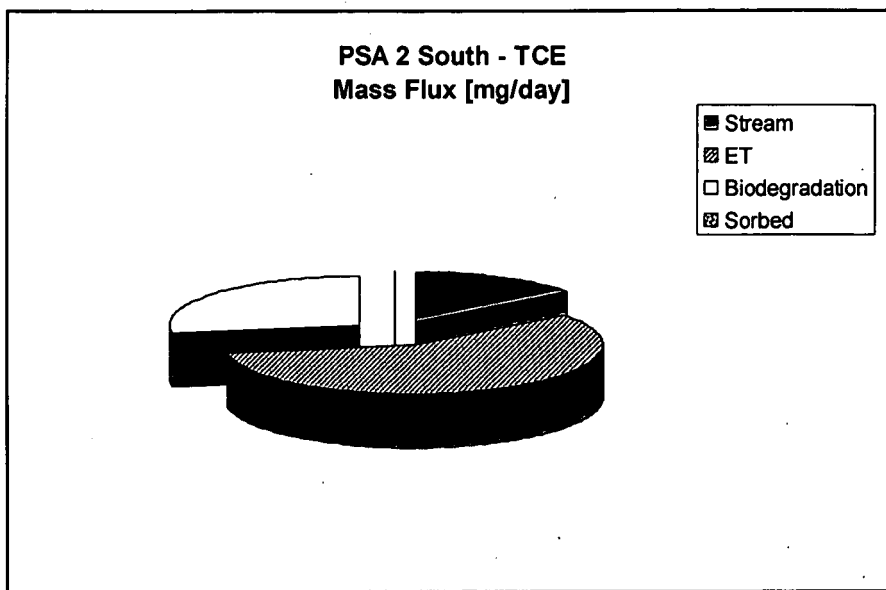


Figure 6.20. PSA 2S - Steady-state mass flux for TCE.

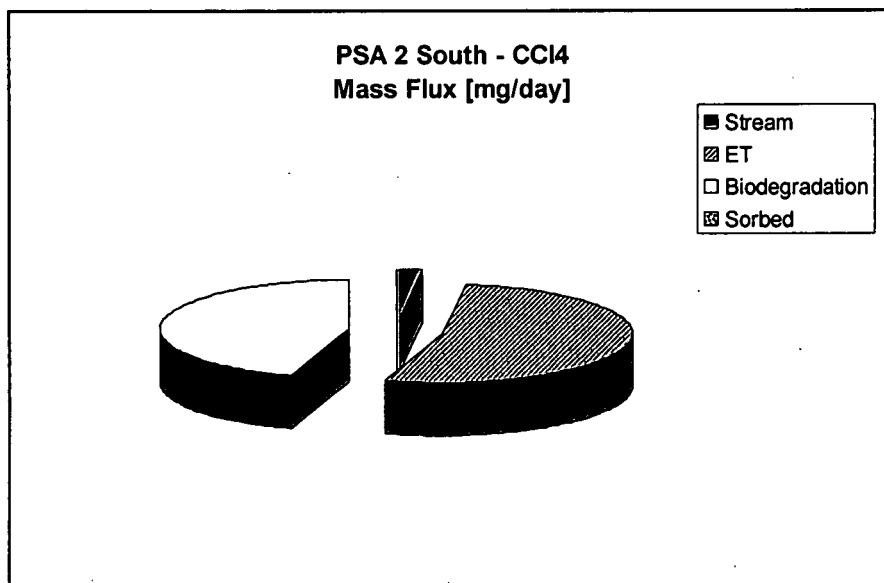


Figure 6.21. PSA 2S - Steady-state mass flux for CCl<sub>4</sub>.



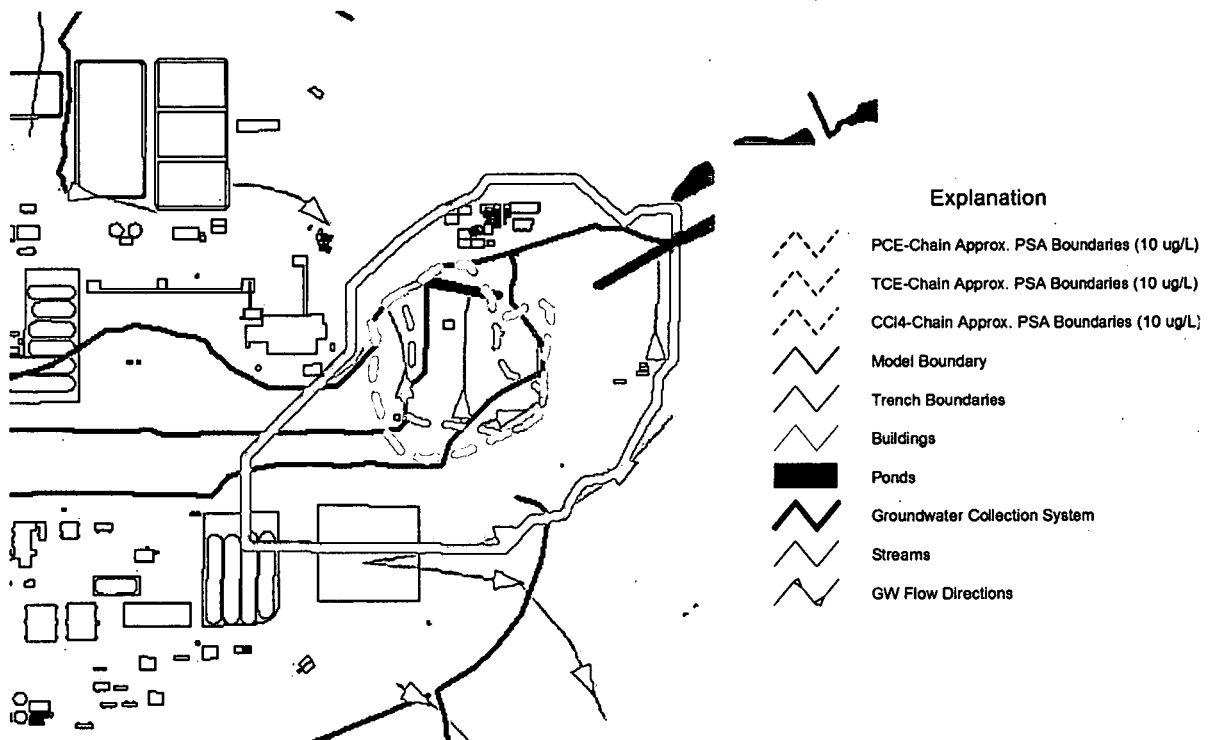


Figure 6.22. PSA 5 - Model area and PSA.

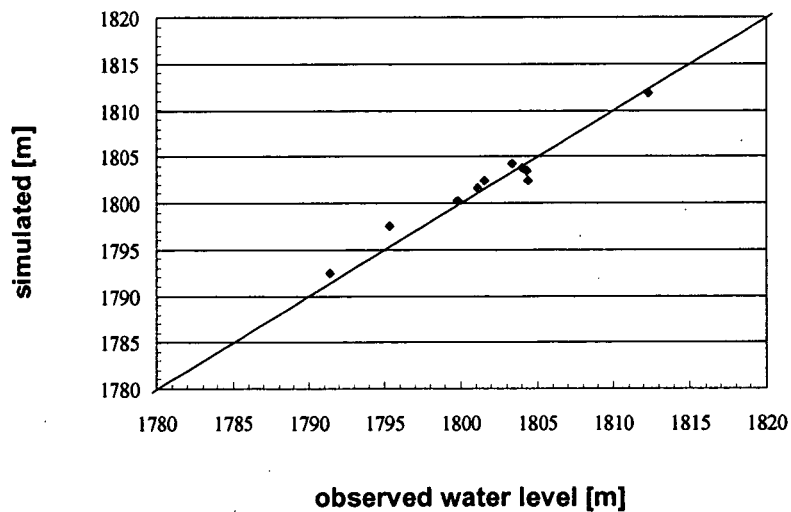


Figure 6.23. PSA 5 - Simulated versus average observed annual groundwater levels (meters above msl).

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#### **6.2.1.4.2 Simulated Groundwater VOC Sources**

The primary VOC source for PSA 5 model area was the Mound Site (drum storage area) as described in a final report for the Mound Site Plume Project by IT Corporation (1998). A secondary source was also introduced in a former Oil Burn Pit area (IHSS 153) (described in the HRR database in Appendix A).

PSA 5 contains five Priority 1 Releases, three in the up-gradient Mound Trenches area and two down-gradient near the Mound groundwater collection system (Appendix A, References 35, 36, 41 of the HRR VOC Summary Table). Drums were placed at the Mound Site from 1954 to 1958. Some of the drums contained lathe coolant (a mixture of hydraulic oil and  $\text{CCl}_4$ ) and others contained PCE (Kaiser-Hill, 1998a). The drums were removed in 1970 but up to 10% of the drums (of the 1405 total) were suspected to have leaked. Removal of VOC contaminated soil was completed in 1997 (Kaiser-Hill, 1998a). Even though soil was excavated and remediated from the source area, the VOCs of concern are denser than water (DNAPL) and migrate downward through permeable materials until reaching a low permeability layer. It is likely that contaminants have migrated through the unconsolidated material to the upper weathered bedrock. In areas where the soil was not completely removed and remediated, residual NAPL is likely present, acting as a persistent long-term source. This is supported by time-series plots of concentration data from wells near the source areas that do not show a decreasing trend with time.

The three up-gradient Priority 1 releases (Mound Site) are closely located. They were simulated with constant PCE sources in two adjacent model cells (Figure 6.24). In this model area, the unconsolidated deposits remain unsaturated in many areas. As a result, inferred sources were specified in the weathered bedrock. There was no need to introduce TCE as an additional parent at the Mound source location based on the observed concentration distribution.

Two wells were recently installed (in 2003) to investigate a suspected source (the Oil Burn Pit, IHSS 153) west of the Mound Site. Groundwater samples from well 91103, located in the source area, showed a PCE concentration of 18 mg/L and a TCE concentration of 11 mg/L. Well 91203, located just north of the source area, showed a PCE concentration of 0.004 mg/L. A PCE/TCE source was inferred at the higher concentration well location.

#### **6.2.1.4.3 Particle Tracking Results – Historical Conditions**

Particles were introduced in the upper weathered bedrock at the model sources and allowed to travel for 42 years (Figure 6.24). Particle tracking results indicated that simulated flow directions agreed well with assumed locations, depths, and release timing of the PSA sources. Particles introduced at the drum storage area traveled north in the weathered bedrock where they were intercepted by the Mound Groundwater Collection System. Particles from the former Oil Burn Pit traveled northwest and discharged to South Walnut Creek, west of the Mound Groundwater Collection System. Thin unconsolidated

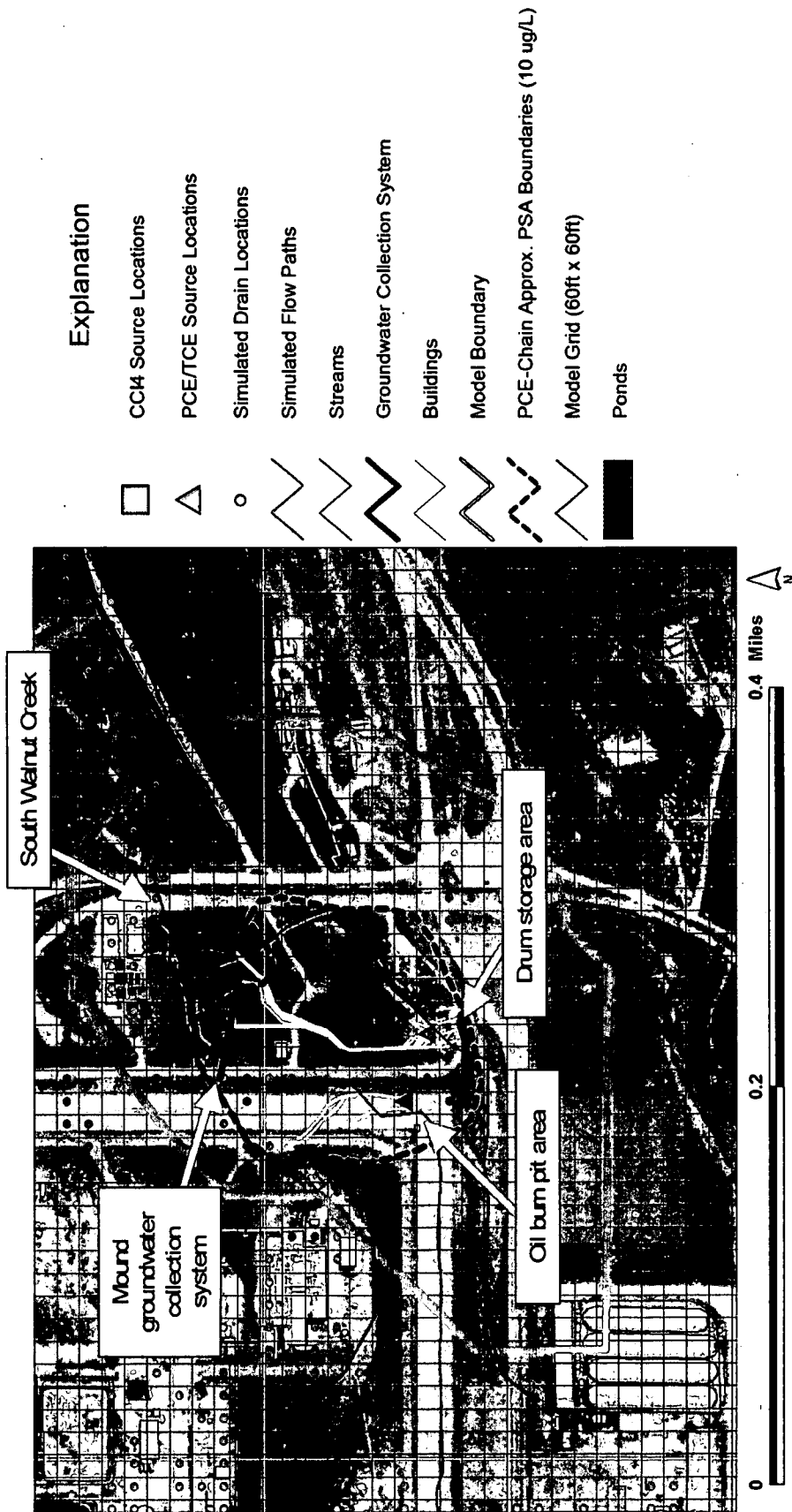


Figure 6.24. PSA 5 - Flow paths (after 42 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

material (<2 meters) on the hillslope north of the assumed sources was unsaturated. This forced groundwater to flow mainly within the bedrock in this area. The northeast trending Arapahoe Sandstone caused particles to flow eastward when intercepted.

#### **6.2.1.4.4 Transport Model Results – Historical Conditions**

Time-averaged concentrations at sample locations show PCE and TCE are present above their draft surface water PRGs. Time-averaged PCE and TCE concentrations (65.2 mg/L and 3.8 mg/L) were greater than the draft surface water PRG levels (1.46 mg/L and 0.19 mg/L, respectively) within the PSA 5 area.

The highest time-averaged CCl<sub>4</sub> concentration is below its PRG. As a result CCl<sub>4</sub> was not modeled. Additionally, simulated sources were not inferred for the two down-gradient Priority 1 releases. One was associated with well 3586, which has an average VC concentration above its PRG value but the concentrations in the well exhibited a declining trend that is now below the PRG. The other is to the northeast in an area of very low concentrations.

A total of 14 sensitivity runs using more conservative and less conservative parameter values bracketed the time-averaged concentrations. The modeled contamination distribution for most simulation runs extends laterally beyond the 10 ug/L PSA outline. Additional data from the new wells to the west and the reports of VOC odors from soil digging to the east support these results. The TCE results for high hydraulic conductivity and low hydraulic conductivity simulations are shown on Figures 6.25 and 6.26. Sensitivity runs bracket the observed daughter product concentrations in the model area except for the high VC area north of the treatment system (well 3586).

The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain) was bracketed by the various sensitivity runs with the high hydraulic conductivity (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain. The results are shown on Figure D-5 and Figure D-6 of Appendix D.

#### **6.2.1.4.5 Transport Model Results – Closure Configuration**

Under current conditions groundwater flows to the north from the Mound Site and northwest from the Oil Burn Pit towards South Walnut Creek in the PSA 5 model area. Flow was primarily in the weathered bedrock, which occurs at shallow depths along the hillslope. Under the proposed closure configuration, South Walnut Creek along the northwest model boundary area will be re-graded. Simulations indicated that the closure configuration modifications redirect the current northward flow from the Mound Site to be more northwest.

A total of 16 transport models were simulated. All runs were simulated for at least 100 years to establish steady long-term concentrations with time at the PSA

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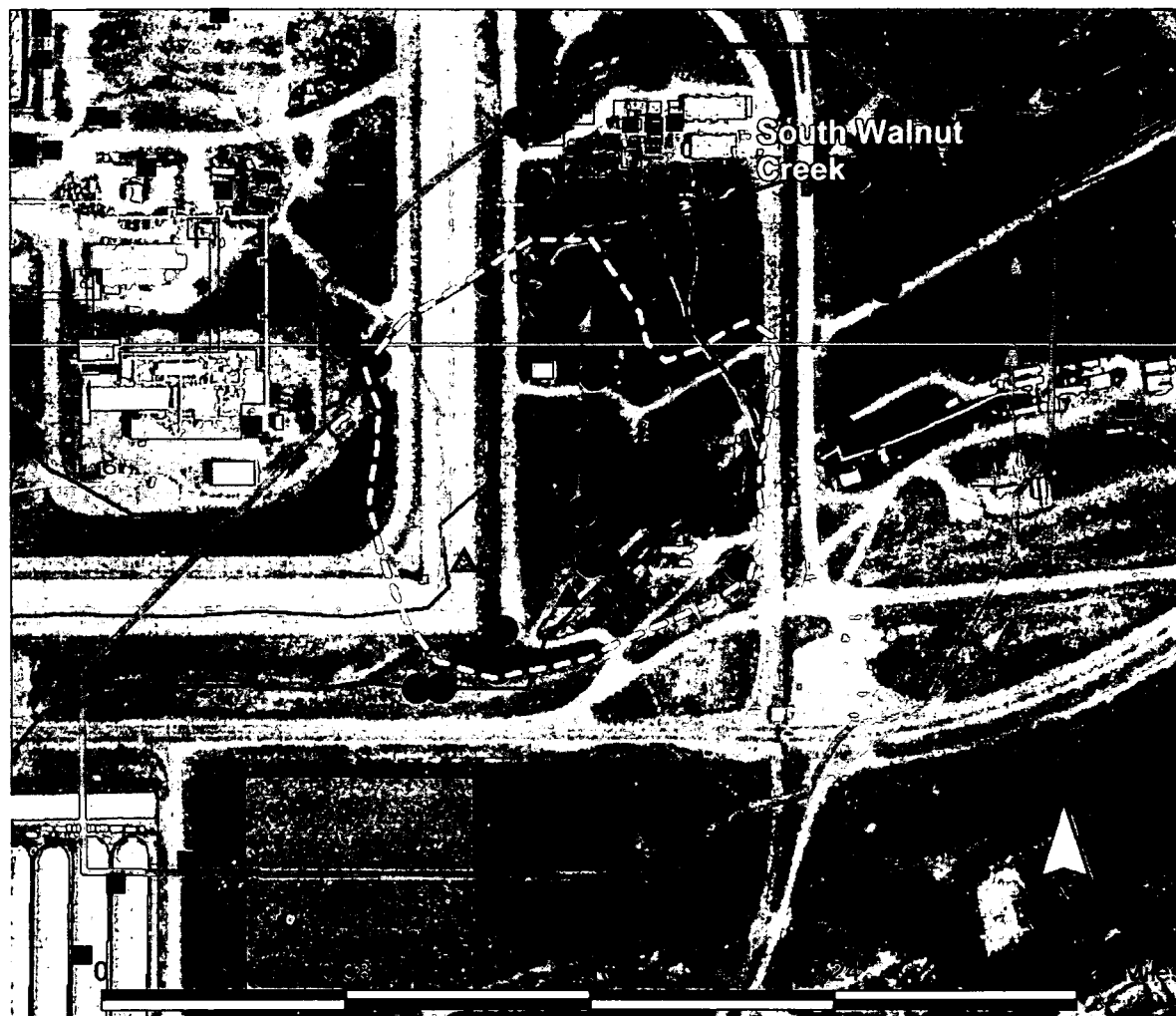
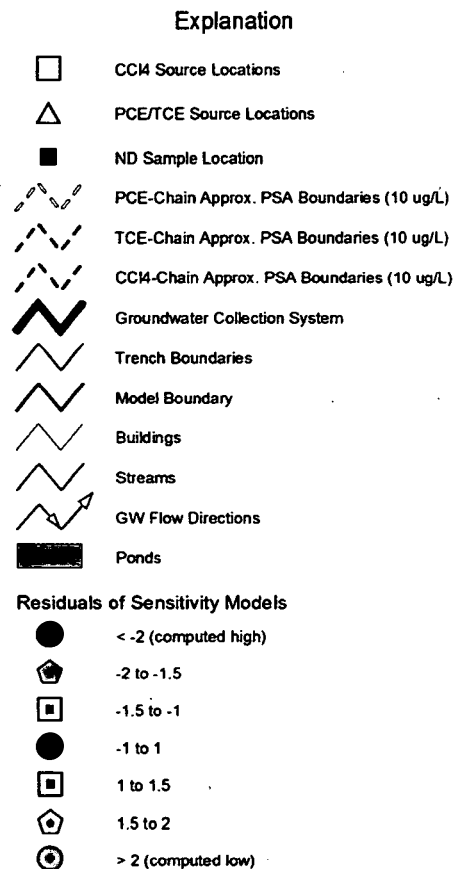


Figure 6.25. PSA 5 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (high hydraulic conductivity).

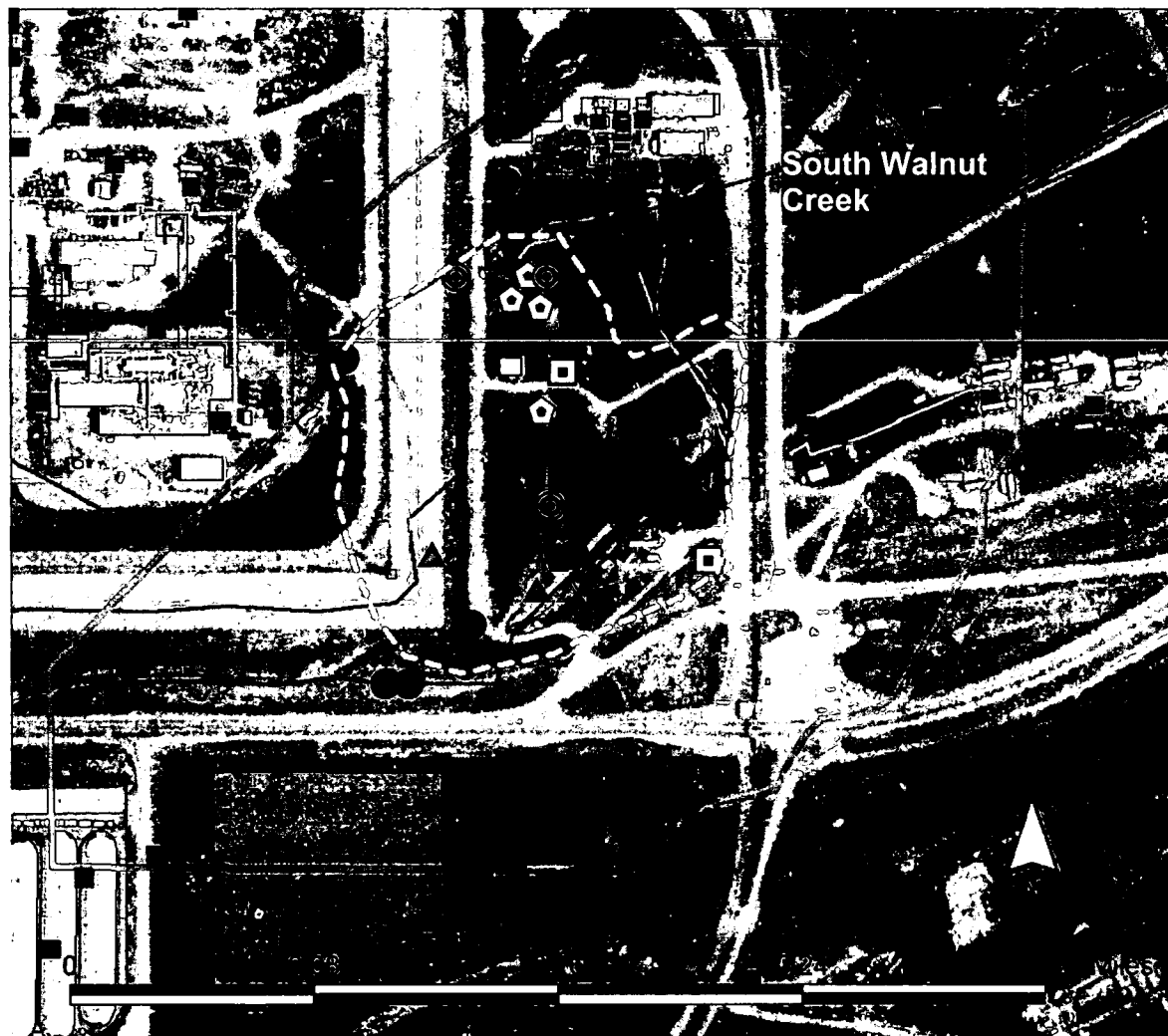
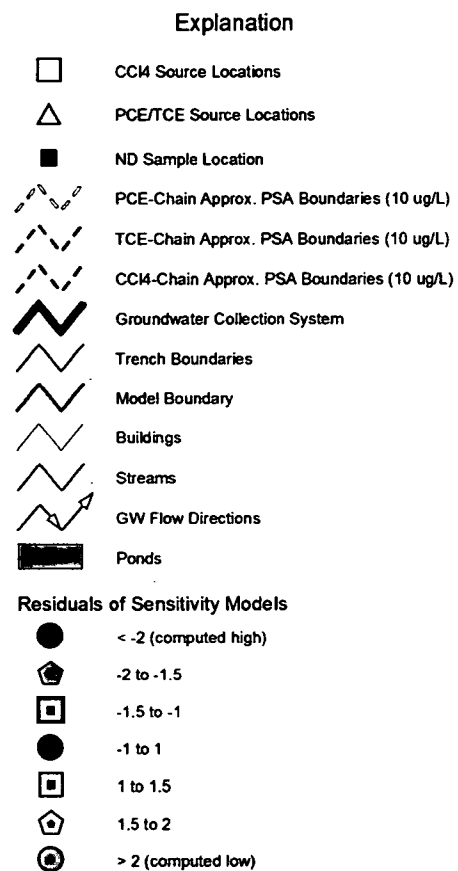


Figure 6.26. PSA 5 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (low hydraulic conductivity).

extent along South Walnut Creek to the north and northwest. Steady-state conditions took at least 60 years to develop at all stream locations for this PSA. The average simulated groundwater concentration for each of the model runs at integrated flow groundwater discharge areas for PCE and TCE are shown on Figures 6.27 and 6.28. No time-averaged  $\text{CCl}_4$  concentrations occur above draft surface water PRG value (0.58 mg/L) within the PSA 5 area. As such,  $\text{CCl}_4$  was not simulated for the closure configuration.

The bar charts on Figure 6.27 and Figure 6.28 show simulated closure-condition groundwater discharge concentrations for PCE and TCE. Simulated PCE concentrations were below draft surface water PRGs for the average of all sensitivity runs at discharge locations. However, simulated groundwater discharge concentrations for the high source concentration sensitivity simulation, were above draft surface water PRG at several locations. Simulated TCE concentrations, averaged from all runs, were greater than the draft surface water PRG (0.19 mg/L) along a section of the southern tributary of South Walnut Creek (Figure 6.28). Two main factors contributed to the high-simulated discharge concentrations. The inferred VOC source (an oil burn pit), located west-northwest of the Mound trench source, causes the higher discharge concentrations. The Arapahoe Sandstone subcrops immediately west of the Mound trench source and the modeled oil burn pit source and may preferentially direct VOCs to south Walnut Creek.

Mass flux from the transport model indicated the simulated dominant loss mechanisms for VOCs were ET and biodegradation (Figure 6.29 and 6.30). Biodegradation was the more important mass flux loss mechanism for the parent, PCE, which is introduced as a constant concentration at the source. The TCE daughter, produced through PCE degradation, occurred down-gradient from the source nearer discharge areas where ET and loss to streams are more significant loss mechanisms.

135

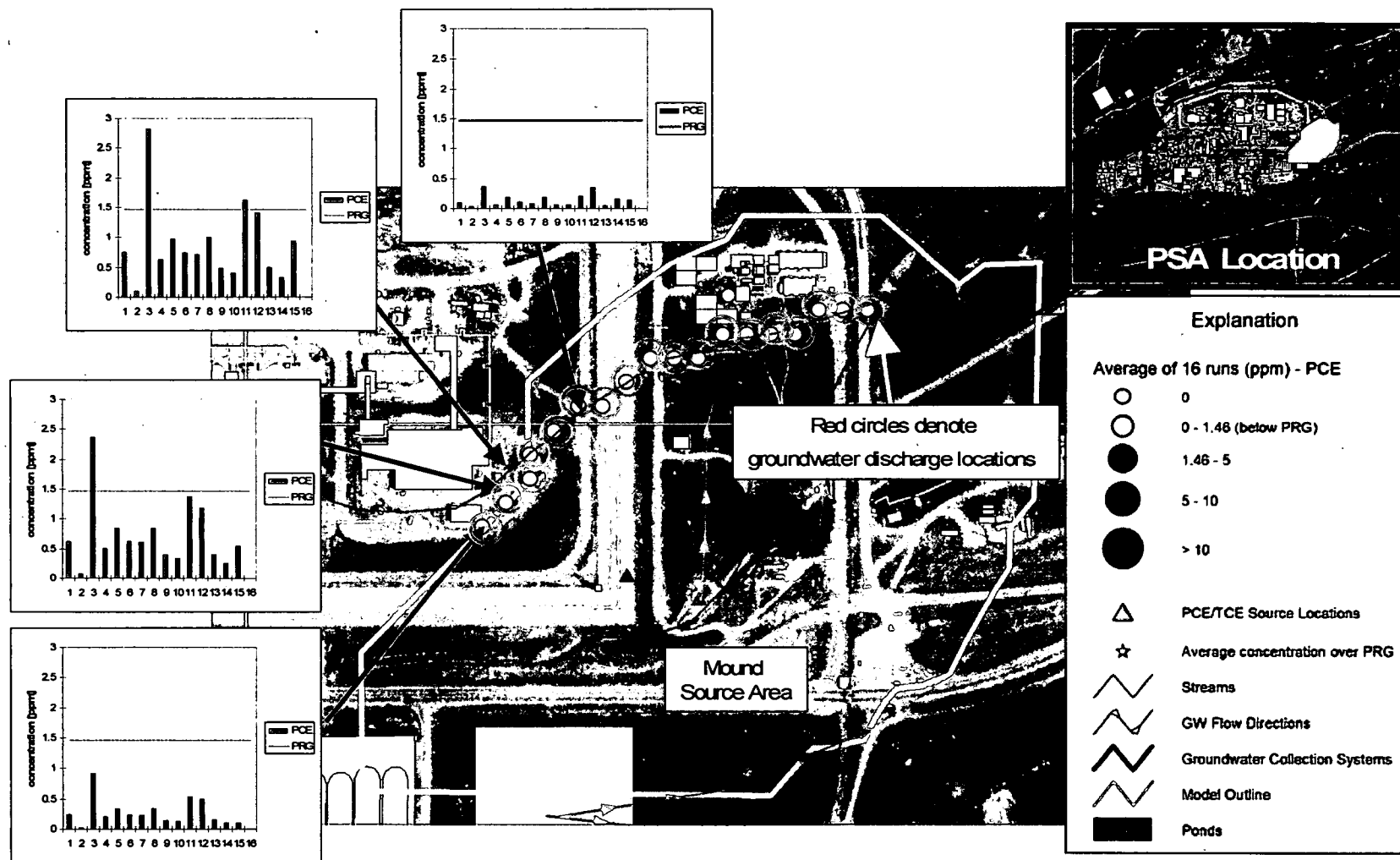


Figure 6.27. PSA 5 - Simulated PCE groundwater concentrations at discharge cells. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.



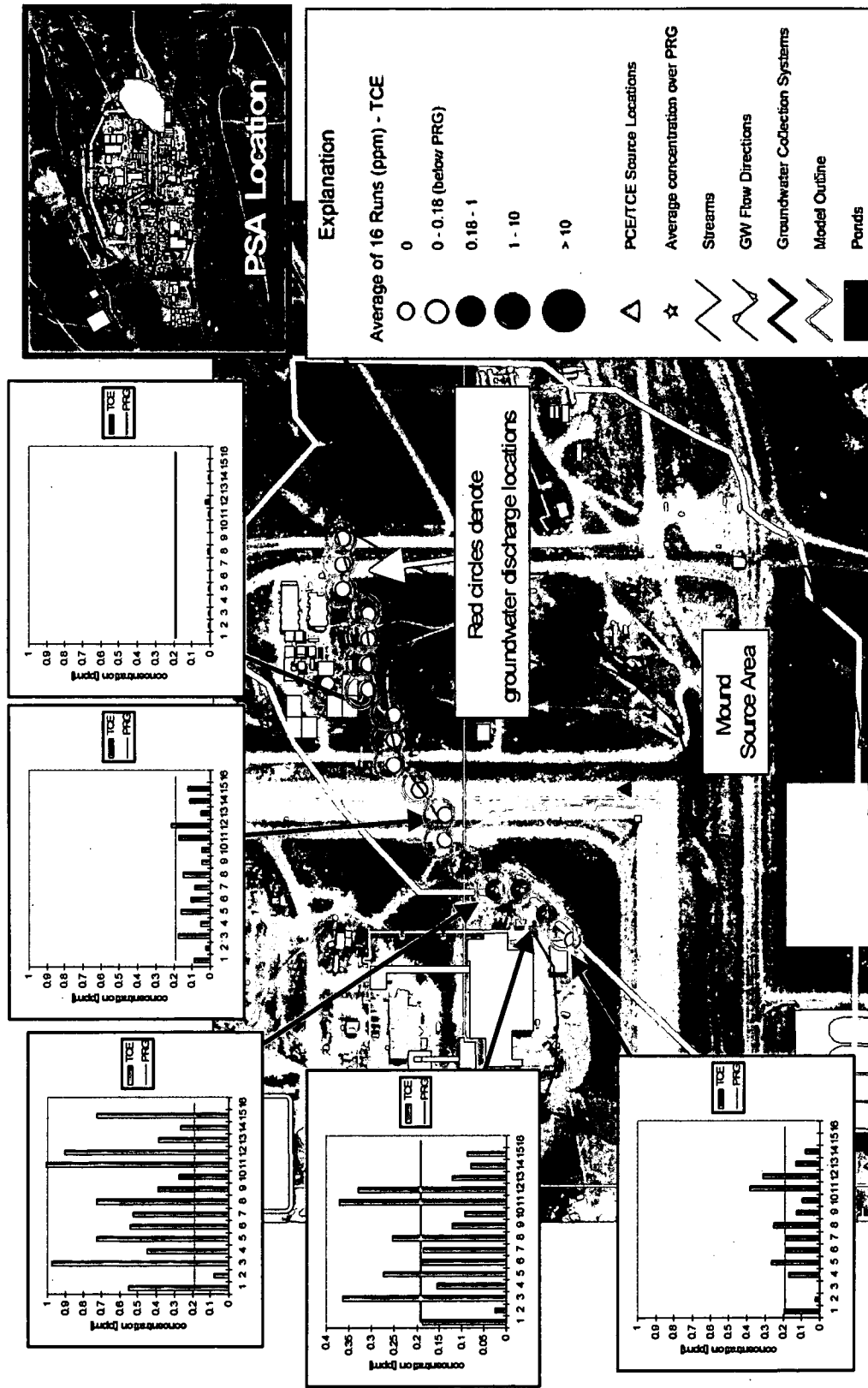


Figure 6.28. PSA 5 - Simulated TCE groundwater concentrations at discharge cells. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

**PSA 5 - PCE**  
**Mass Flux [mg/day]**

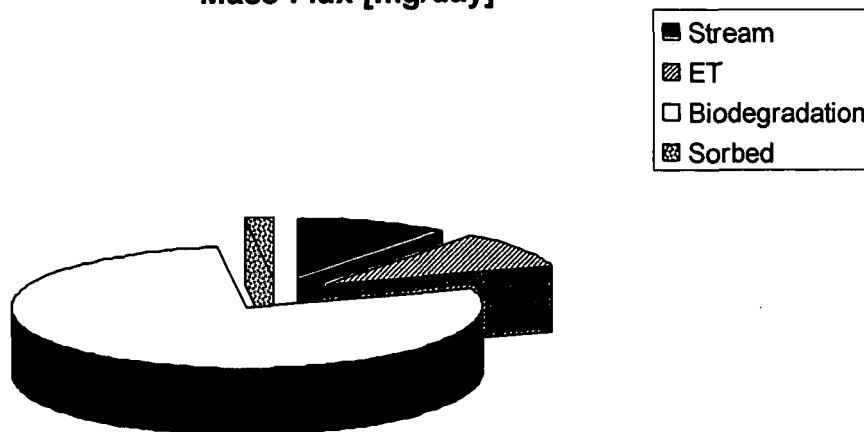


Figure 6.29. PSA 5 - Steady-state mass flux for PCE.

**PSA 5 - TCE**  
**Mass Flux [mg/day]**

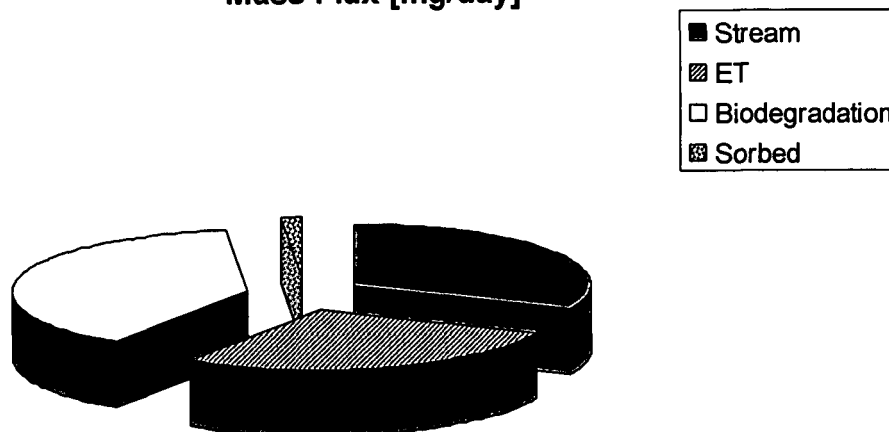


Figure 6.30. PSA 5 - Steady-state mass flux for TCE.

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#### 6.2.1.5 PSA 9 (881 Hillside Area)

The PSA 9 model area is located on the southern slope of the IA, southeast of the Building 881 area (Figure 6.1). PSA 9 occurs immediately north of the 881 Hillside Groundwater Collection System shown on Figure 6.31.

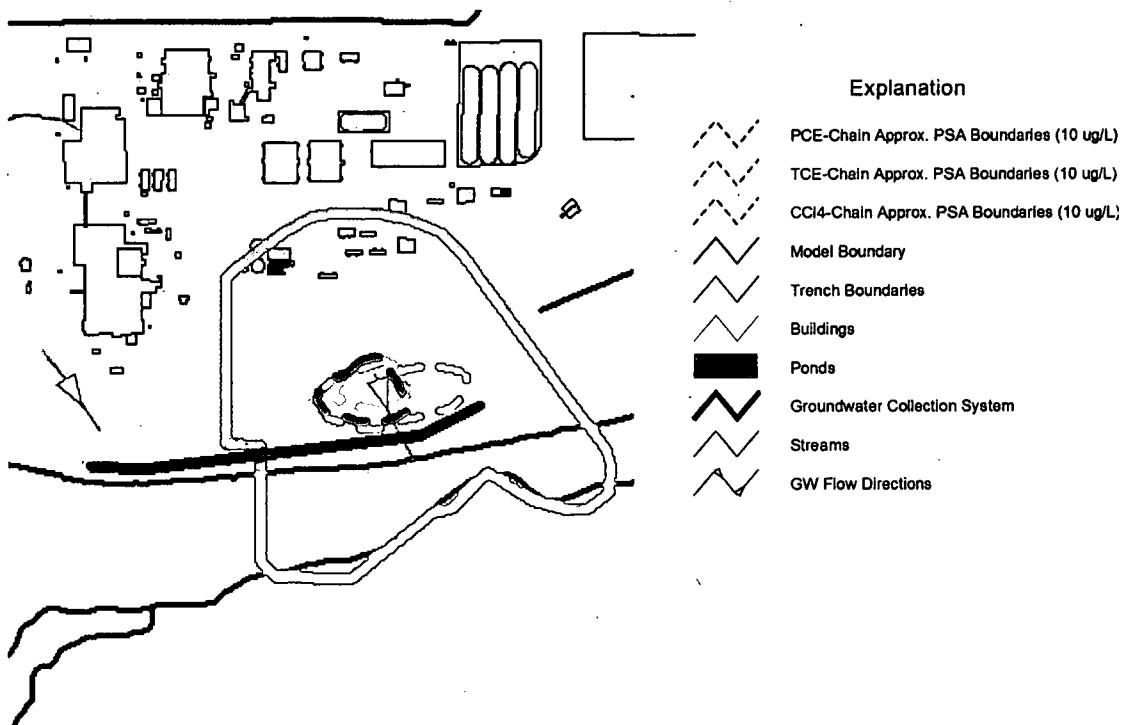


Figure 6.31. PSA 9 - Model area and PSA.

##### 6.2.1.5.1 Groundwater Flow Model Results

Constant head boundaries for PSA 9 were placed along the northern and western up-gradient boundaries to simulate lateral inflow of groundwater. No-flow boundaries were specified along the hillslopes, parallel to groundwater flow. Constant heads were also assigned along Woman Creek at the southern boundary to simulate groundwater discharge to the stream area.

Simulated water balance results indicated that groundwater discharged through ET and to Woman Creek. Only a small amount discharged to the 881 Hillside Groundwater Collection System, which is consistent with available discharge information. Simulated hydraulic heads compare well with observed groundwater water levels in the area (Figure 6.32).

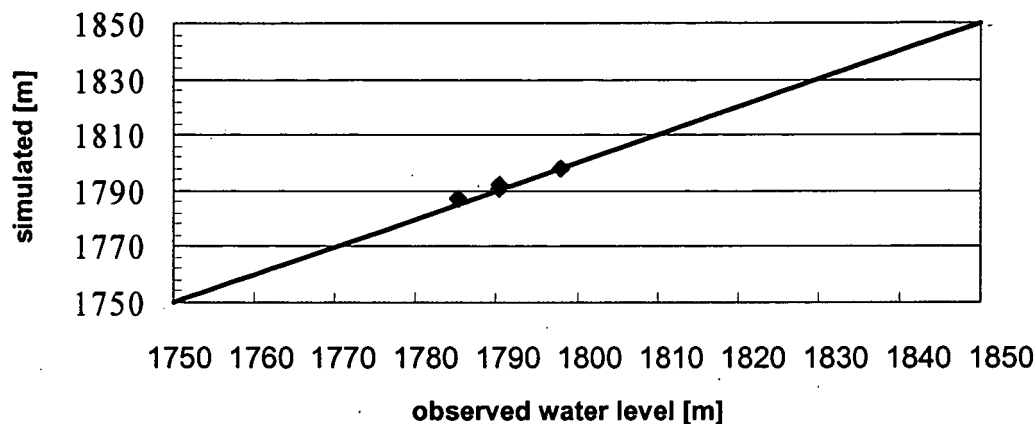


Figure 6.32. PSA 9 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.5.2 Simulated Groundwater VOC Sources

One Priority 2 release was documented within the PSA 9 area. HRR information indicates this was the "West Scrap Metal Storage Area" (HRR Reference No. 180 through 185, Appendix A). The release, discovered in 1968, included PCE, TCE, and  $\text{CCl}_4$ -chain contaminants; as well as unknown volumes of solvents, oil-grease, and nitrates.

#### 6.2.1.5.3 Particle Tracking Results – Historical Conditions

Particles were introduced into the model at the inferred VOC source area and allowed to travel for 30 years (Figure 6.33). Particles were entered into the upper weathered bedrock because the thin overlying unconsolidated material in this PSA remains unsaturated most of the year.

Particle tracking results suggested that assumed source locations, depths, and release timing were reasonable. Results also indicated that calculated flow directions agreed well with the assumed PSA shape and extent (Figure 6.33). Particles traveled only short distances in this PSA because groundwater flows is restricted to the low conductivity weathered bedrock (claystone/siltstone). Arapahoe Sandstone is not present in this area. This is further supported by the low VOC concentrations collected at the 881 Hillside Groundwater Collection System, immediately down-gradient of PSA 9. VOC concentrations were consistently below RFCA Tier II Groundwater action levels which resulted in it being decommissioned in 2000 (Kaiser-Hill, 2003a).

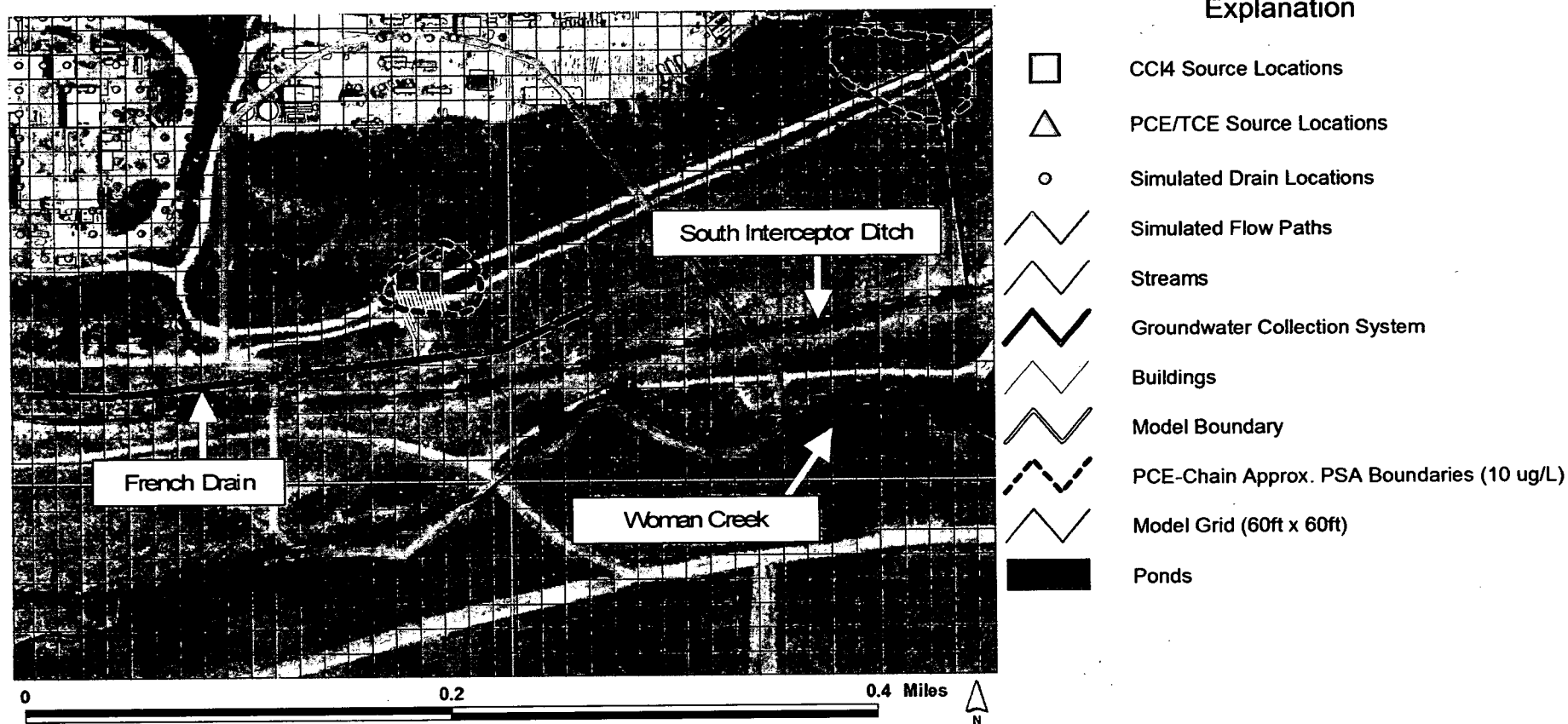


Figure 6.33. PSA 9 - Simulated flow paths (after 30 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

#### **6.2.1.5.4 Transport Model Results – Historical Conditions**

Within PSA 9, PCE, TCE, and  $\text{CCl}_4$  were detected above the draft surface water PRG levels (1.4 mg/L, 0.19 mg/L, and 0.58 mg/L respectively). Estimated PSA boundaries for each VOC chain are shown on Figures B-1 through B-7 (Appendix B). Effective source concentrations were specified for PCE, TCE, and  $\text{CCl}_4$  at the inferred VOC source location for PSA 9.

Results of sensitivity simulations conducted with the PSA 9 model showed that concentration distributions of parent VOCs, PCE, TCE, and  $\text{CCl}_4$  were reproduced over the range of input parameter values considered. PSA 9 model residual concentration plots for TCE (i.e., log of time-averaged minus simulated concentrations), shown on Figures 6.34 and 6.35, indicated that, in areas down-gradient from the inferred VOC source, the highest concentrations (Figure 6.34) were produced with low degradation values, while the lowest concentrations (Figure 6.35) were produced when source concentrations were specified in the lower weathered bedrock. Sensitivity simulations of PCE and  $\text{CCl}_4$  showed results similar to those obtained for TCE.

The PSA 9 model area was comparatively unique because simulated concentrations remained largely insensitive to changes in parameter values, as represented on Figure 6.34 (highest simulated concentrations) and Figure 6.35 (lowest simulated concentrations). In these figures, only the three northeastern observation points were affected by parameter changes. These zero observed concentration points produced higher model concentrations in the low degradation case, and zero concentrations in the case where the sources were introduced into the lowest layer of weathered bedrock. The insensitivity of the model to changes in parameter values can be mainly attributed to the low hydraulic conductivity of the weathered bedrock/siltstone/claystone in the area, which results in longer residence times and increased potential for degradation.

The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain; chloroform and methylene chloride for the  $\text{CCl}_4$  degradation chain) was bracketed by the various sensitivity runs, with high concentrations (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain. The results are shown on Figure D-7 and Figure D-8 of Appendix D.

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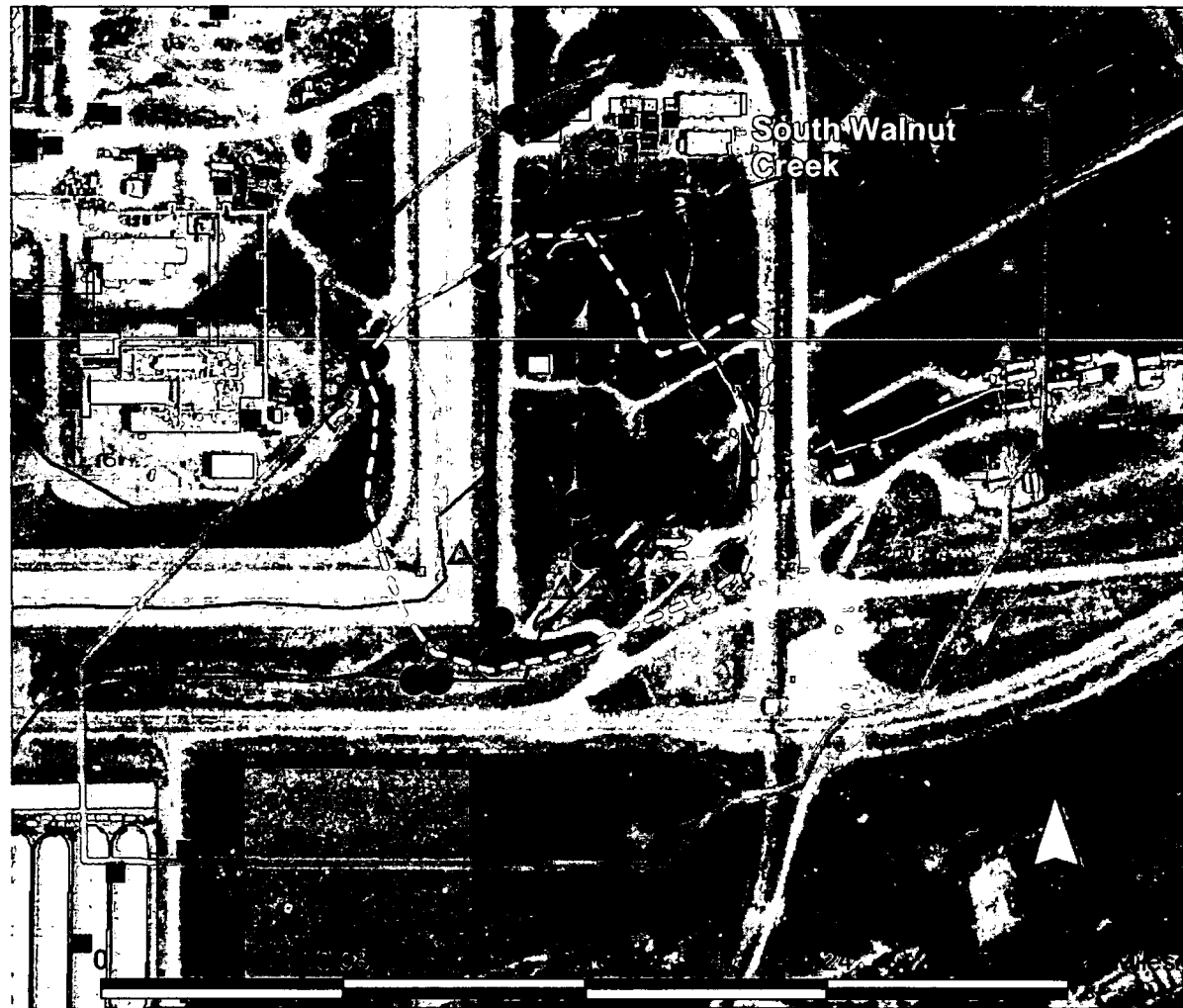
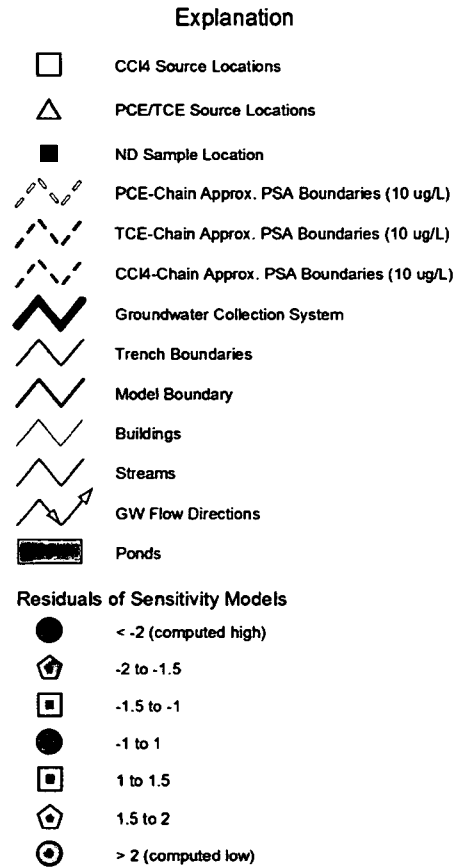


Figure 6.25. PSA 5 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (high hydraulic conductivity).

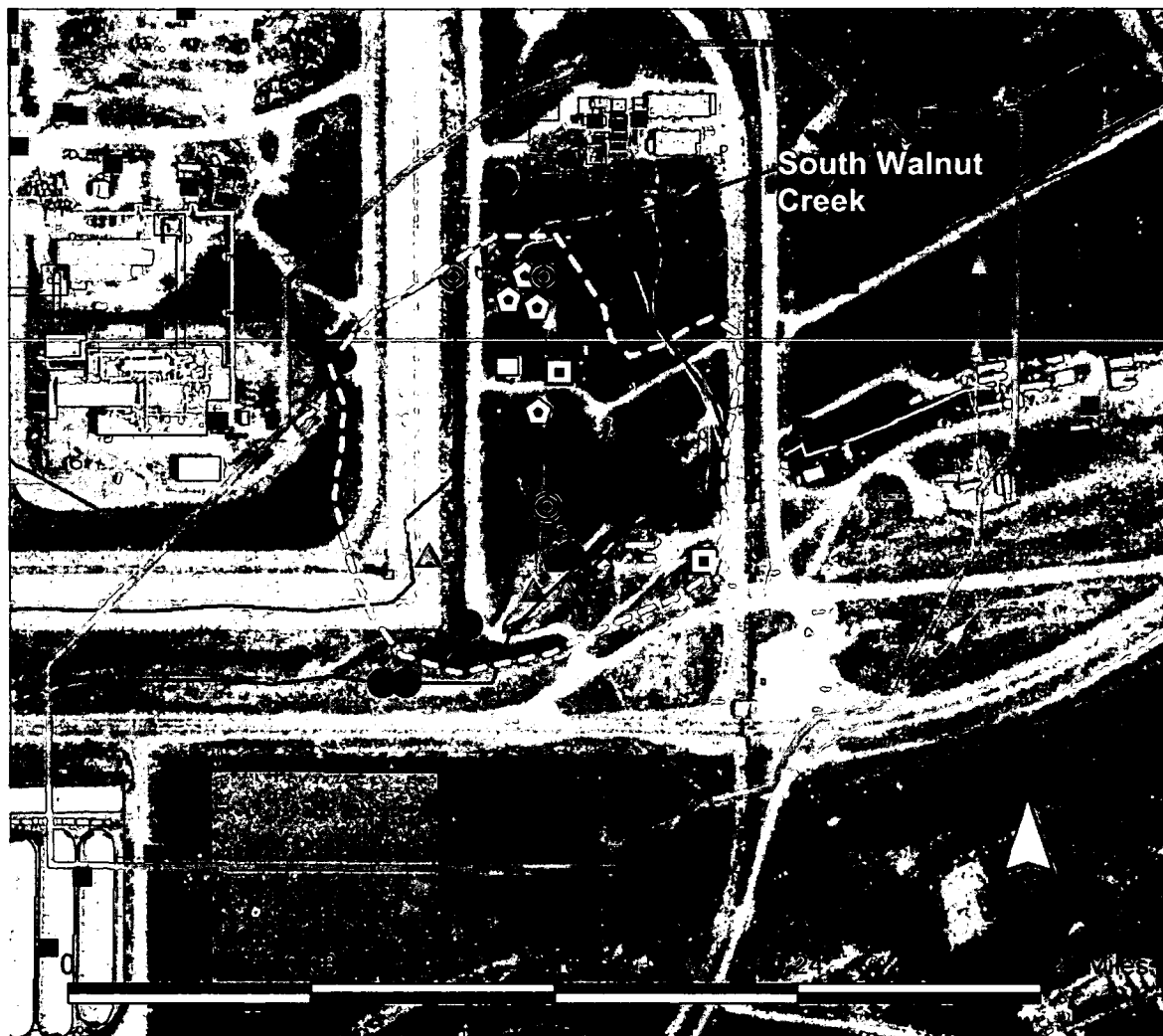
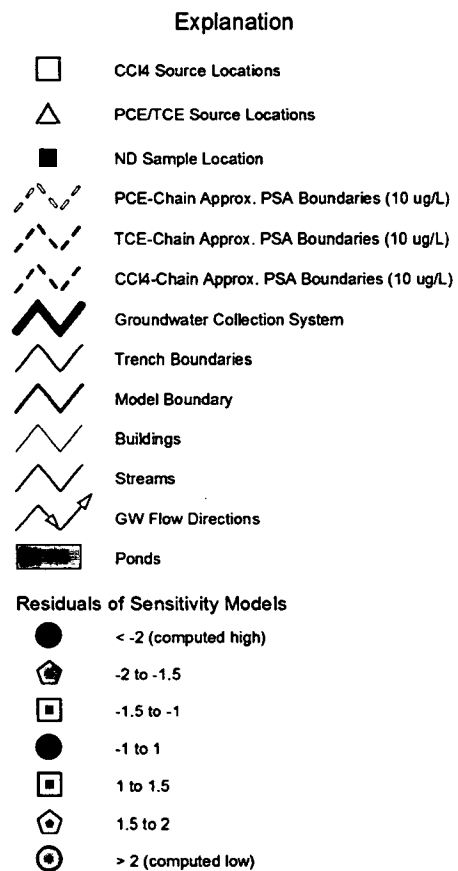


Figure 6.26. PSA 5 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (low hydraulic conductivity).



extent along South Walnut Creek to the north and northwest. Steady-state conditions took at least 60 years to develop at all stream locations for this PSA. The average simulated groundwater concentration for each of the model runs at integrated flow groundwater discharge areas for PCE and TCE are shown on Figures 6.27 and 6.28. No time-averaged  $\text{CCl}_4$  concentrations occur above draft surface water PRG value (0.58 mg/L) within the PSA 5 area. As such,  $\text{CCl}_4$  was not simulated for the closure configuration.

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Mass flux from the transport model indicated the simulated dominant loss mechanisms for VOCs were ET and biodegradation (Figure 6.29 and 6.30). Biodegradation was the more important mass flux loss mechanism for the parent, PCE, which is introduced as a constant concentration at the source. The TCE daughter, produced through PCE degradation, occurred down-gradient from the source nearer discharge areas where ET and loss to streams are more significant loss mechanisms.

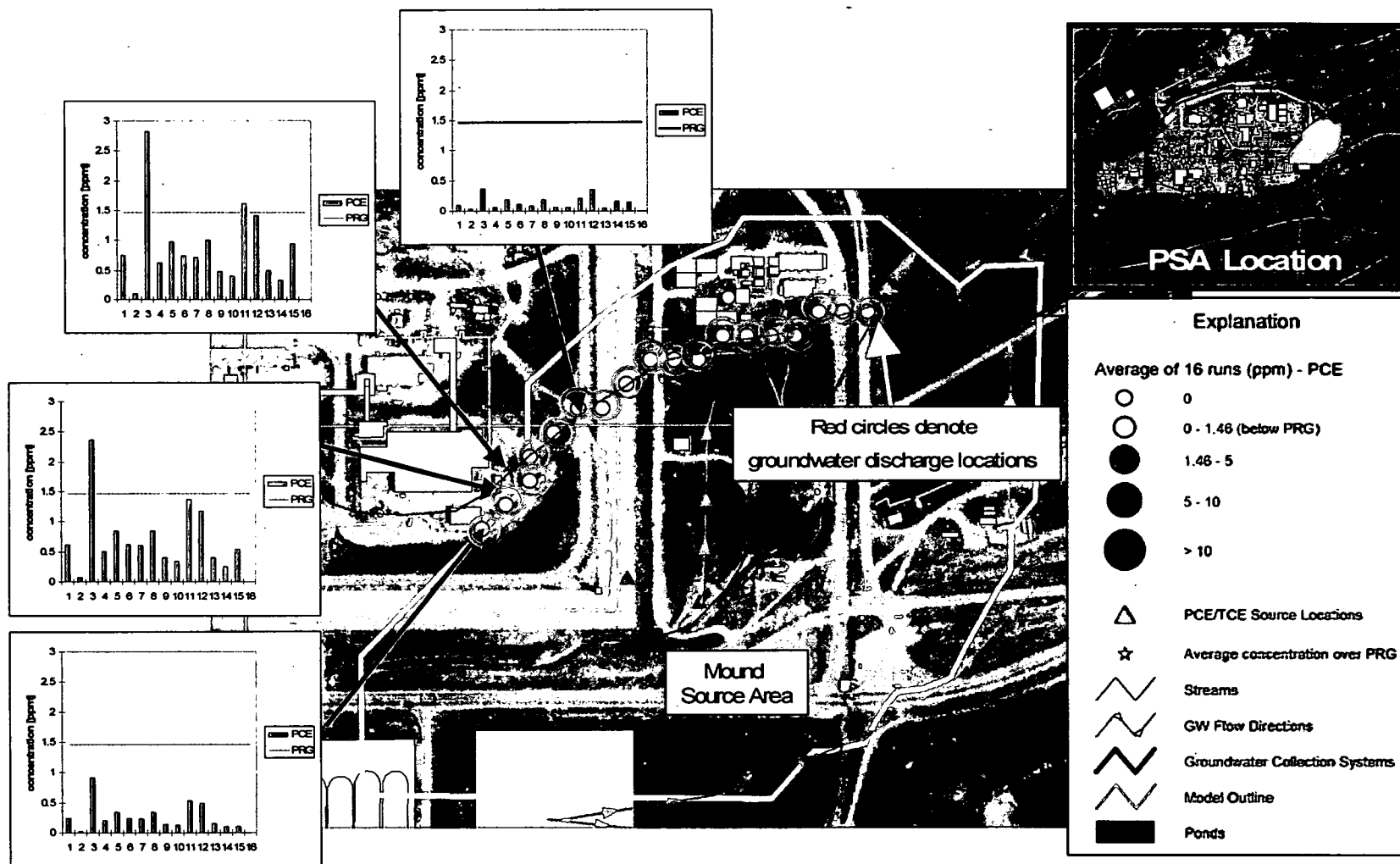


Figure 6.27. PSA 5 - Simulated PCE groundwater concentrations at discharge cells. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

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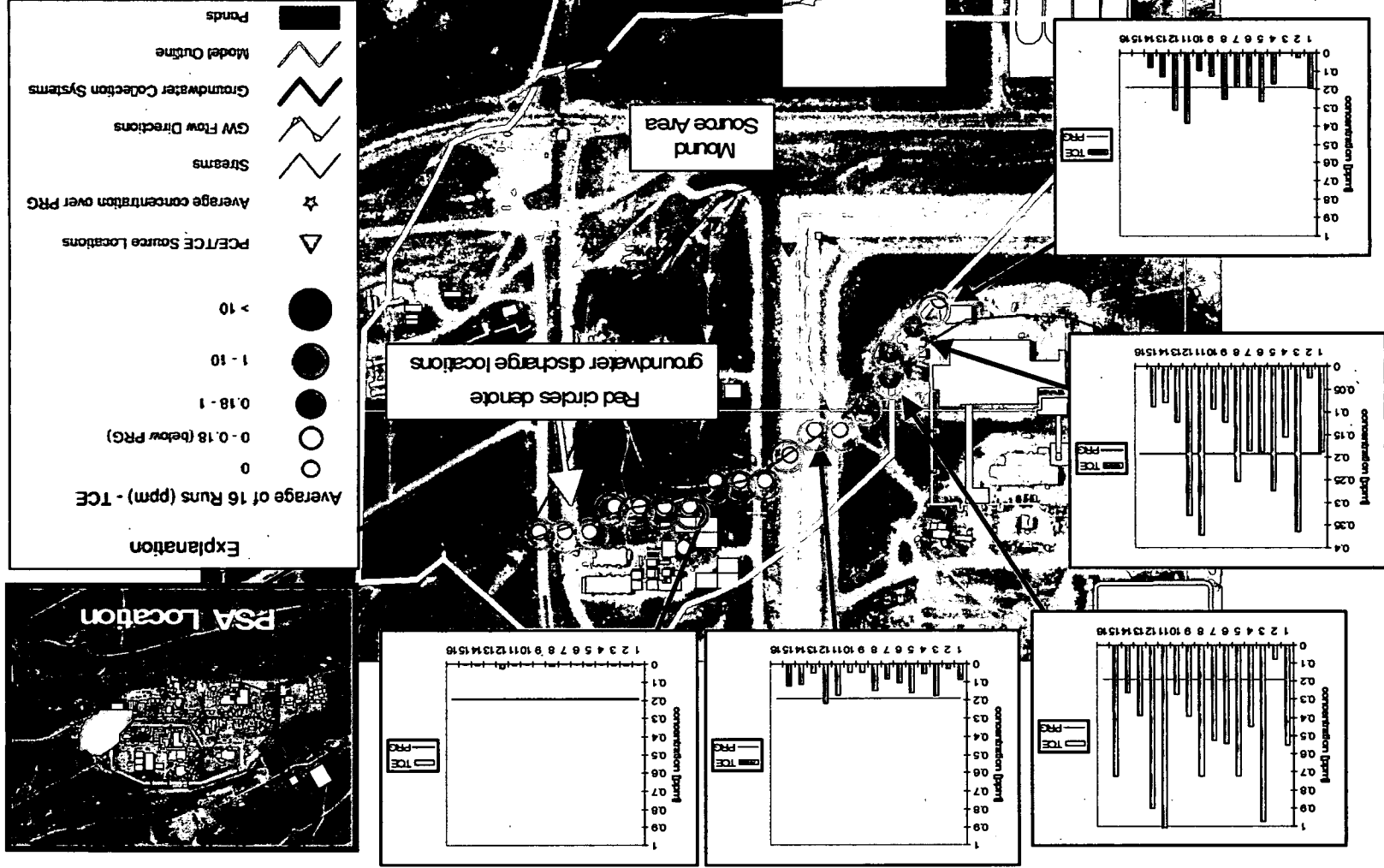


Figure 6.28. PSA 5 - Simulated TCE groundwater concentrations at discharge cells. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

**PSA 5 - PCE  
Mass Flux [mg/day]**

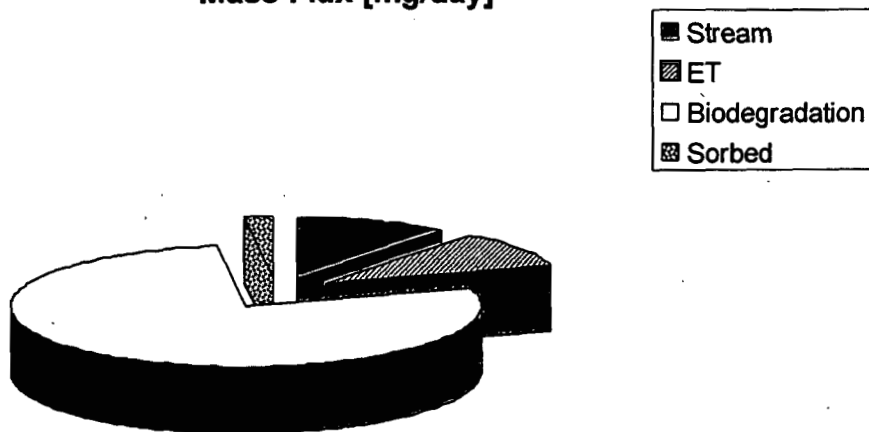


Figure 6.29. PSA 5 - Steady-state mass flux for PCE.

**PSA 5 - TCE  
Mass Flux [mg/day]**

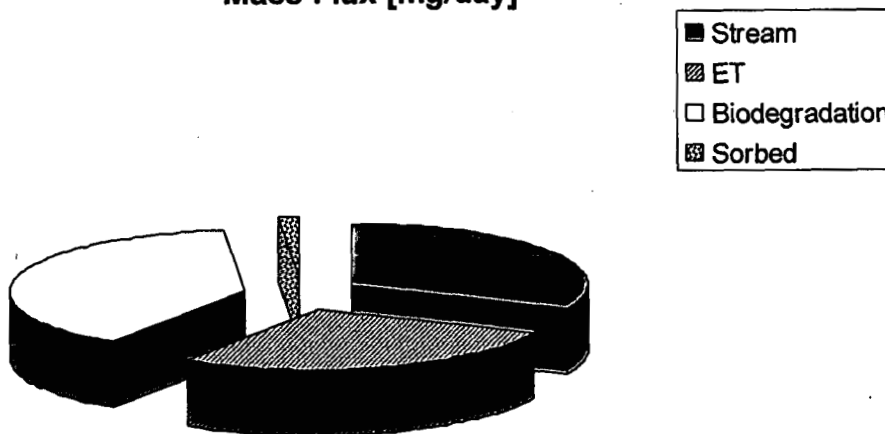


Figure 6.30. PSA 5 - Steady-state mass flux for TCE.

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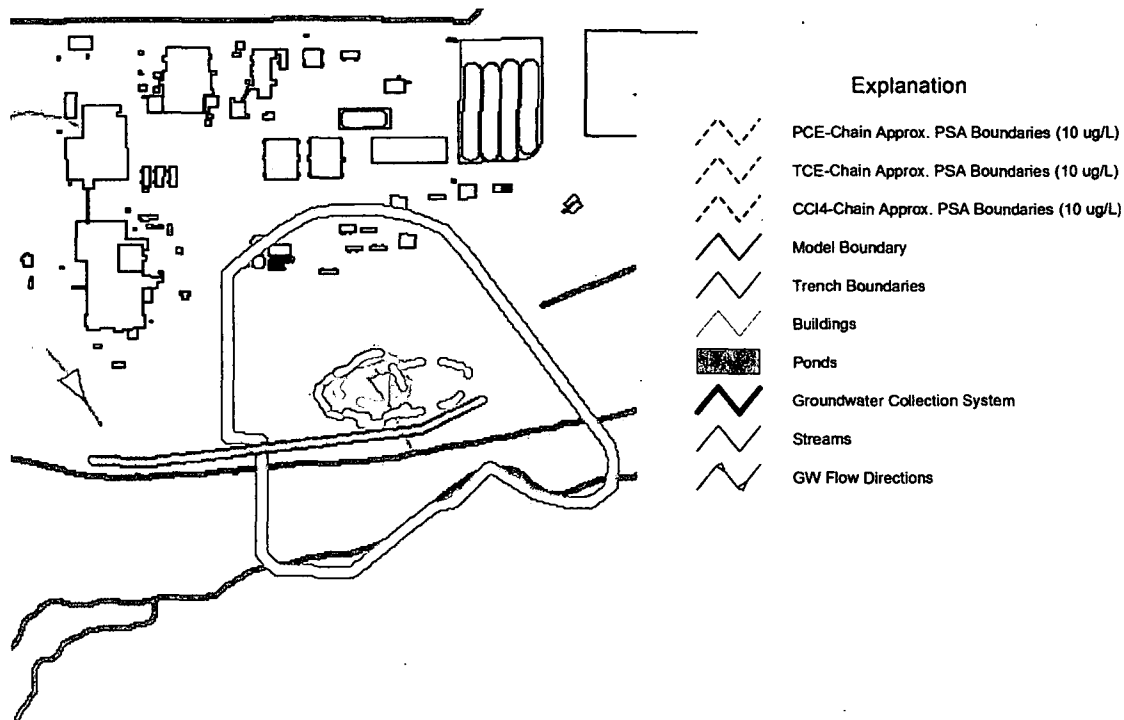


Figure 6.31. PSA 9 - Model area and PSA.

#### 6.2.1.5.1 Groundwater Flow Model Results

Constant head boundaries for PSA 9 were placed along the northern and western up-gradient boundaries to simulate lateral inflow of groundwater. No-flow boundaries were specified along the hillslopes, parallel to groundwater flow. Constant heads were also assigned along Woman Creek at the southern boundary to simulate groundwater discharge to the stream area.

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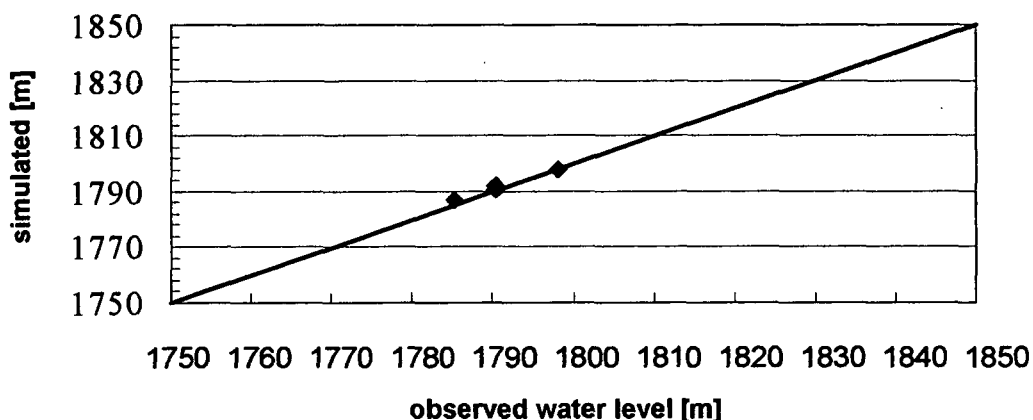


Figure 6.32. PSA 9 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.5.2 Simulated Groundwater VOC Sources

One Priority 2 release was documented within the PSA 9 area. HRR information indicates this was the "West Scrap Metal Storage Area" (HRR Reference No. 180 through 185, Appendix A). The release, discovered in 1968, included PCE, TCE, and CCl<sub>4</sub>-chain contaminants; as well as unknown volumes of solvents, oil-grease, and nitrates.

#### 6.2.1.5.3 Particle Tracking Results – Historical Conditions

Particles were introduced into the model at the inferred VOC source area and allowed to travel for 30 years (Figure 6.33). Particles were entered into the upper weathered bedrock because the thin overlying unconsolidated material in this PSA remains unsaturated most of the year.

Particle tracking results suggested that assumed source locations, depths, and release timing were reasonable. Results also indicated that calculated flow directions agreed well with the assumed PSA shape and extent (Figure 6.33). Particles traveled only short distances in this PSA because groundwater flows is restricted to the low conductivity weathered bedrock (claystone/siltstone). Arapahoe Sandstone is not present in this area. This is further supported by the low VOC concentrations collected at the 881 Hillside Groundwater Collection System, immediately down-gradient of PSA 9. VOC concentrations were consistently below RFCA Tier II Groundwater action levels which resulted in it being decommissioned in 2000 (Kaiser-Hill, 2003a).

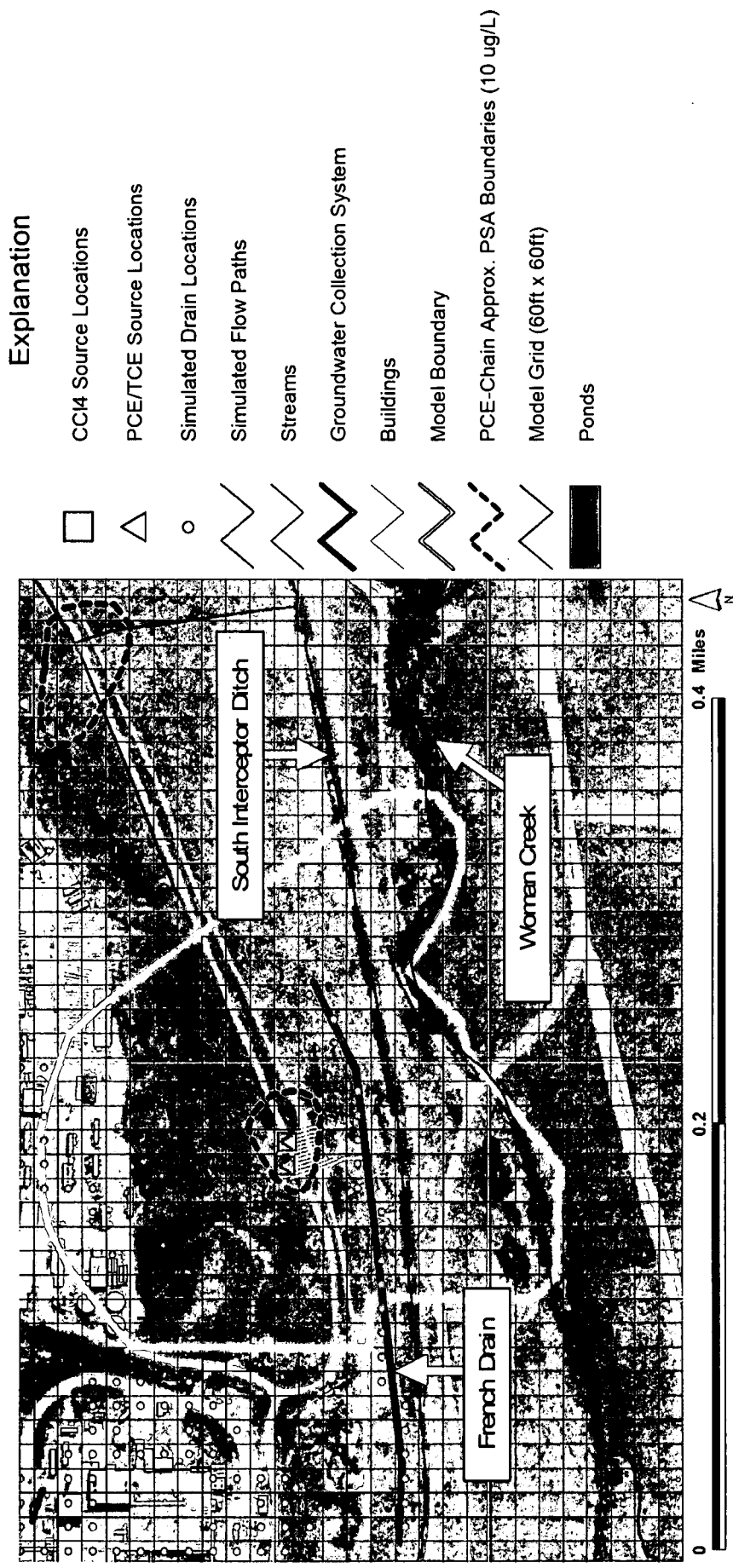


Figure 6.33. PSA 9 - Simulated flow paths (after 30 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

#### **6.2.1.5.4 Transport Model Results – Historical Conditions**

Within PSA 9, PCE, TCE, and  $\text{CCl}_4$  were detected above the draft surface water PRG levels (1.4 mg/L, 0.19 mg/L, and 0.58 mg/L respectively). Estimated PSA boundaries for each VOC chain are shown on Figures B-1 through B-7 (Appendix B). Effective source concentrations were specified for PCE, TCE, and  $\text{CCl}_4$  at the inferred VOC source location for PSA 9.

Results of sensitivity simulations conducted with the PSA 9 model showed that concentration distributions of parent VOCs, PCE, TCE, and  $\text{CCl}_4$  were reproduced over the range of input parameter values considered. PSA 9 model residual concentration plots for TCE (i.e., log of time-averaged minus simulated concentrations), shown on Figures 6.34 and 6.35, indicated that, in areas down-gradient from the inferred VOC source, the highest concentrations (Figure 6.34) were produced with low degradation values, while the lowest concentrations (Figure 6.35) were produced when source concentrations were specified in the lower weathered bedrock. Sensitivity simulations of PCE and  $\text{CCl}_4$  showed results similar to those obtained for TCE.

The PSA 9 model area was comparatively unique because simulated concentrations remained largely insensitive to changes in parameter values, as represented on Figure 6.34 (highest simulated concentrations) and Figure 6.35 (lowest simulated concentrations). In these figures, only the three northeastern observation points were affected by parameter changes. These zero observed concentration points produced higher model concentrations in the low degradation case, and zero concentrations in the case where the sources were introduced into the lowest layer of weathered bedrock. The insensitivity of the model to changes in parameter values can be mainly attributed to the low hydraulic conductivity of the weathered bedrock/siltstone/claystone in the area, which results in longer residence times and increased potential for degradation.

The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain; chloroform and methylene chloride for the  $\text{CCl}_4$  degradation chain) was bracketed by the various sensitivity runs, with high concentrations (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain. The results are shown on Figure D-7 and Figure D-8 of Appendix D.



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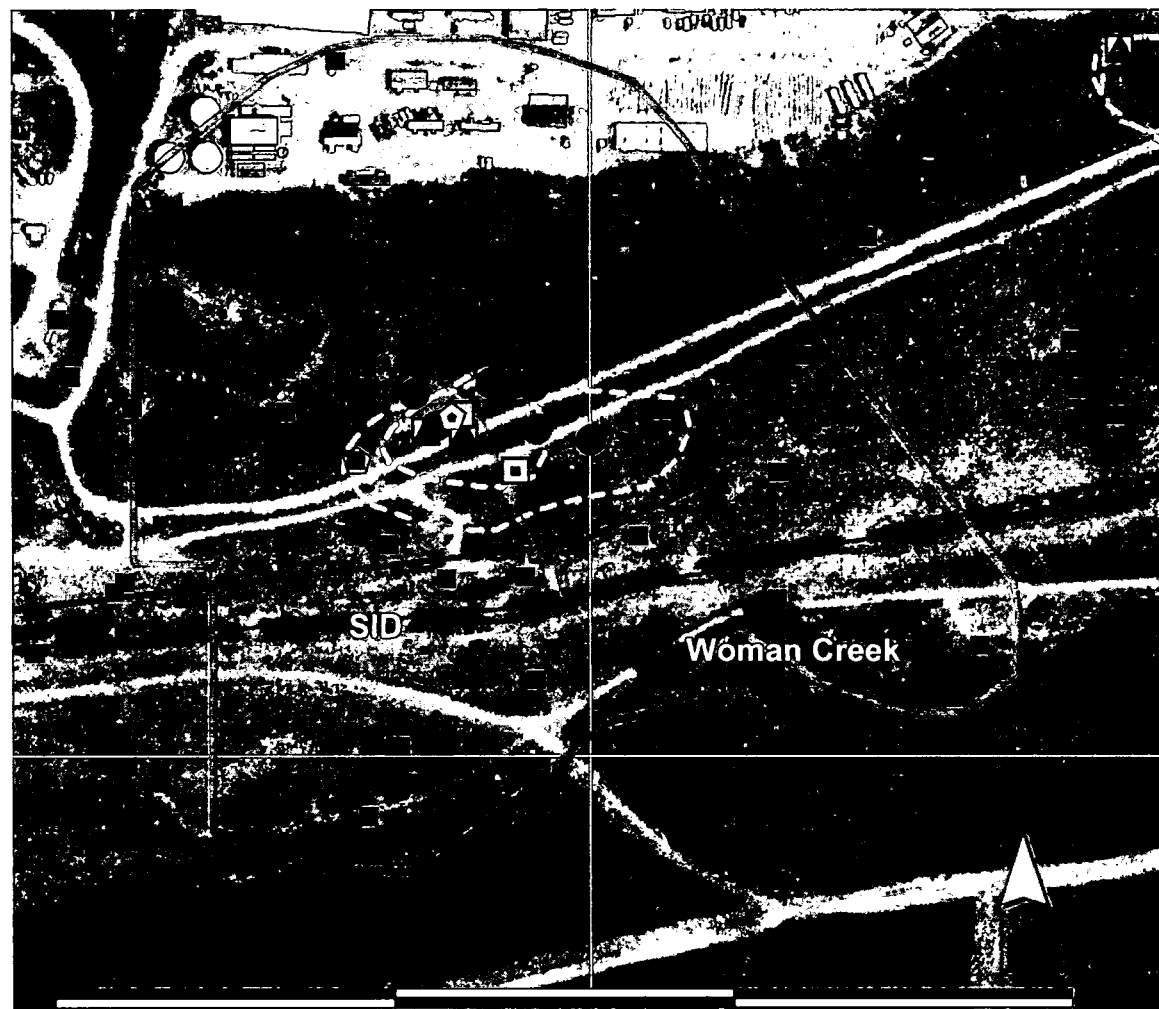
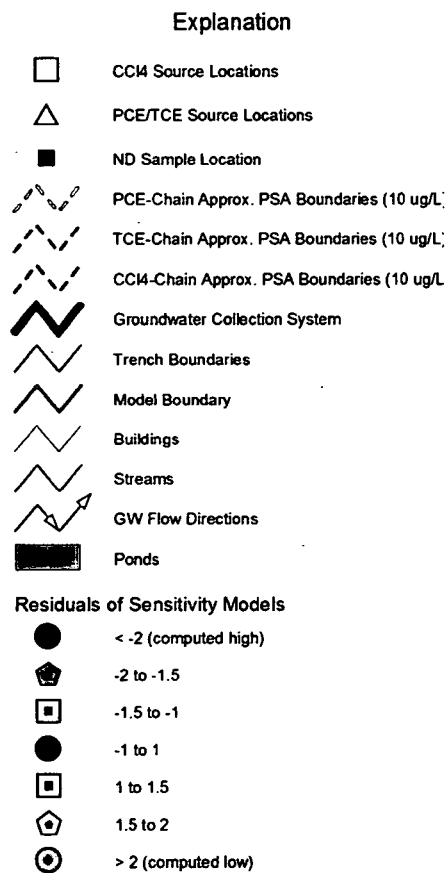


Figure 6.34. PSA 9 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (low degradation).

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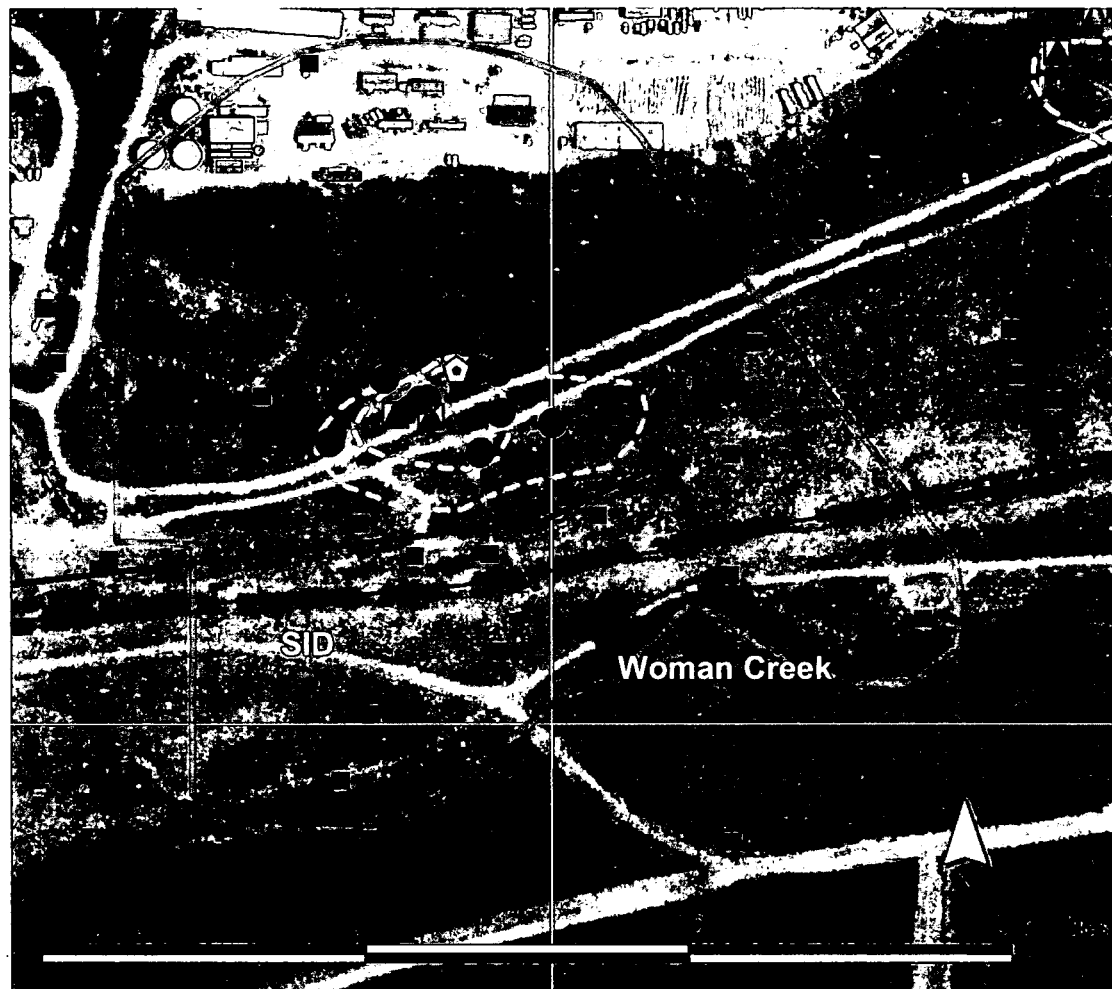
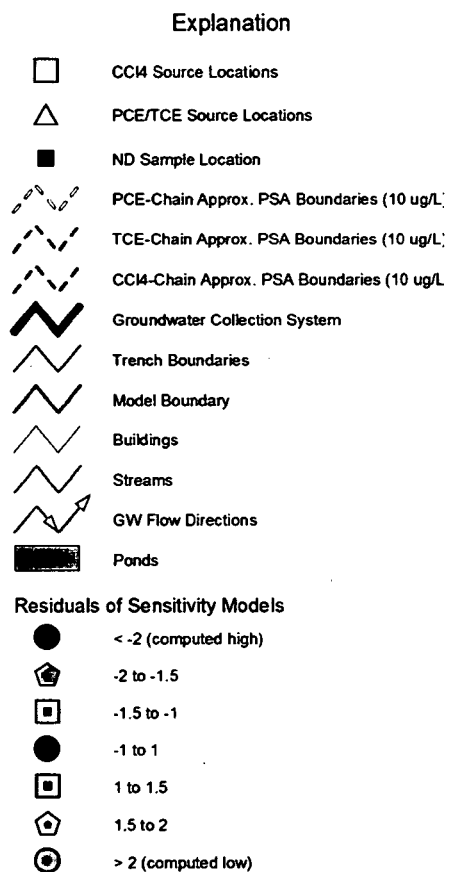


Figure 6.35. PSA 9 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (contaminants introduced into lowest weathered bedrock layer).

#### **6.2.1.5.5 Transport Model Results – Closure Configuration**

In PSA 9, groundwater flows primarily within the weathered bedrock to the south towards the SID and Woman Creek for current conditions. Integrated flow modeling of the closure configuration showed that groundwater flow paths and velocities change little in the PSA 9 model area.

A total of 16 transport simulations were run using the PSA 9 model. All runs were simulated for 100 years to establish steady long-term concentrations with time at the southern PSA extent, along Woman Creek. Simulated groundwater concentrations for the 16 sensitivity simulations at selected groundwater discharge locations along Woman Creek are shown on Figure 6.36 and Figure 6.37.

The bar charts on Figure 6.36 show discharge locations impacted by TCE. Simulated concentrations at these locations ranged from non-detect (zero) to a approximately 0.01 mg/L down-gradient of the inferred VOC source. The PCE/TCE chain was not predicted to impact groundwater above draft surface water PRG levels at any potential discharge location.

The bar charts on Figure 6.37 show discharge locations that have the most potential to be impacted by CCl<sub>4</sub>. Simulated concentrations at these locations were zero down-gradient of the inferred VOC source. The CCl<sub>4</sub> chain was not predicted to impact groundwater above draft surface water PRG levels at any potential discharge location.

Results from the 16 sensitivity models indicated that PCE/TCE and CCl<sub>4</sub> degradation chains have a low sensitivity to changes in parameter values such as; source release location, dispersivity, sorption, porosity, source concentration, degradation rates, and hydraulic conductivity. Mass flux from the transport model indicated that the simulated dominant loss mechanism for VOCs was biodegradation (as shown on Figures 6.38 and 6.39).

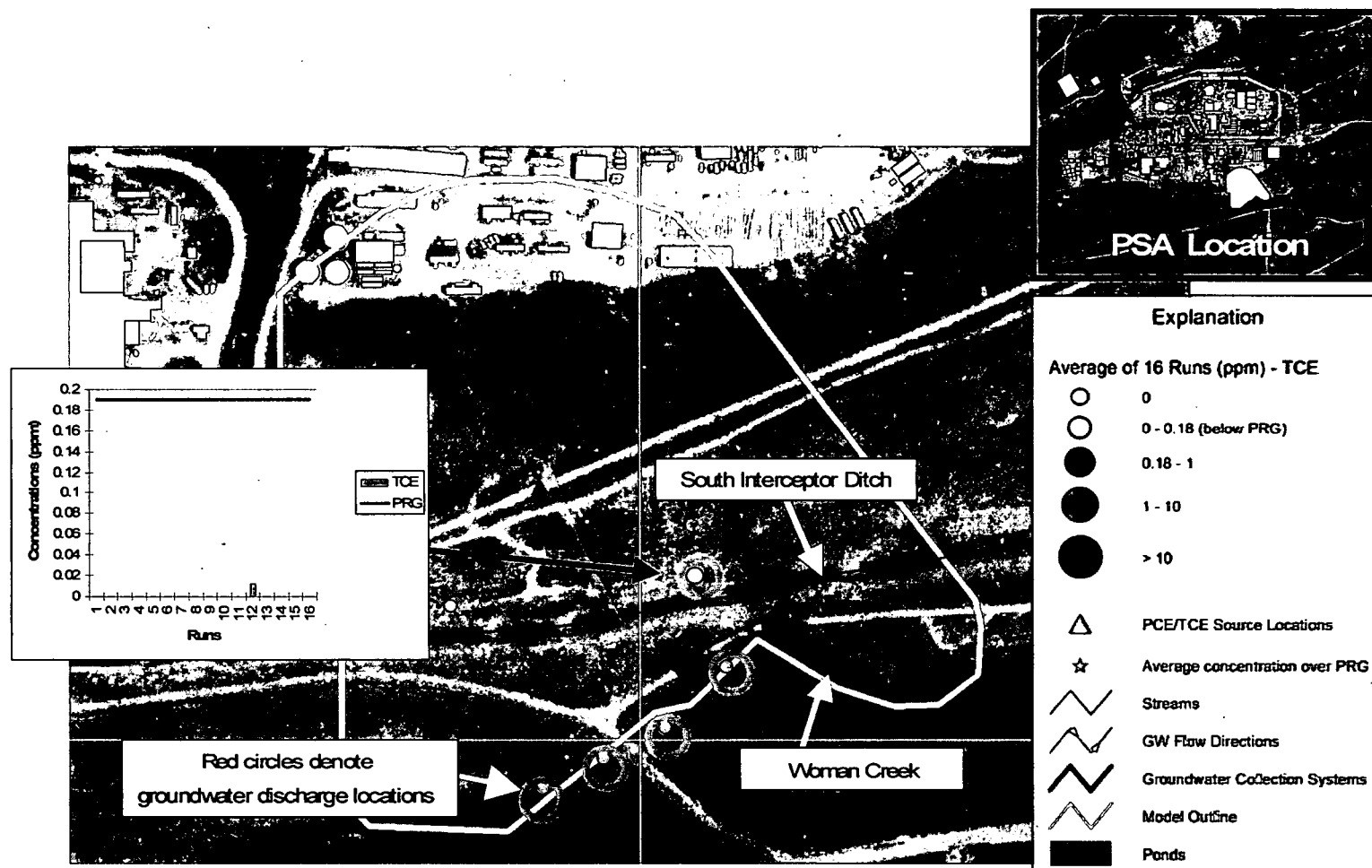


Figure 6.36. PSA 9 - Simulated TCE groundwater concentrations at potential discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

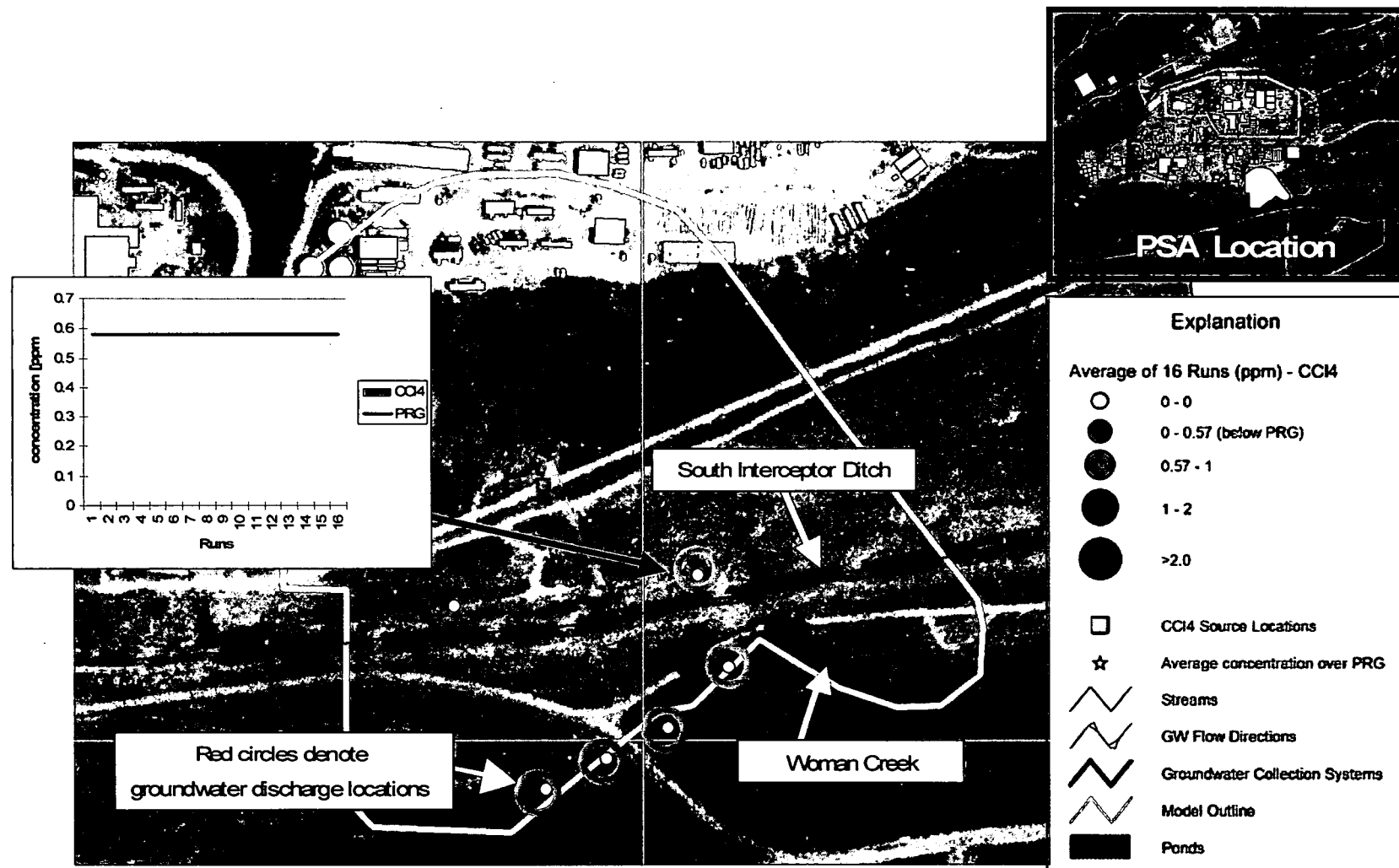


Figure 6.37. PSA 9 - Simulated CCl<sub>4</sub> groundwater concentrations at potential discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

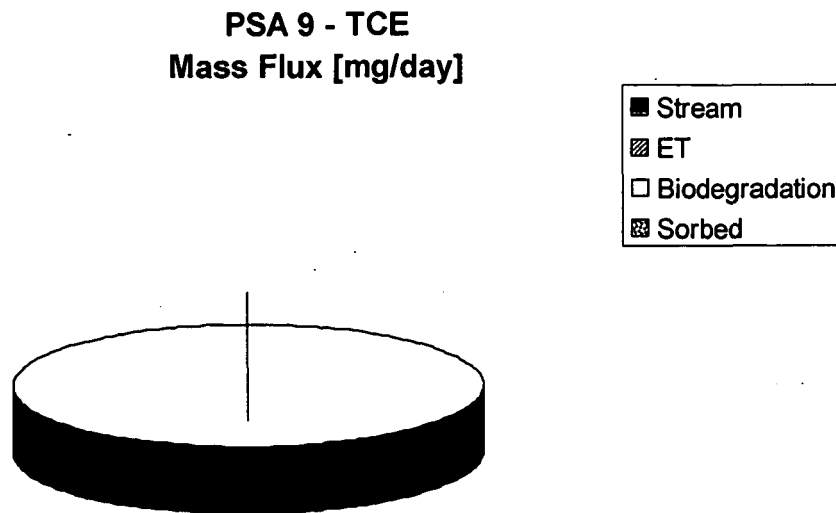


Figure 6.38. PSA 9 - Simulated steady-state mass flux for TCE.

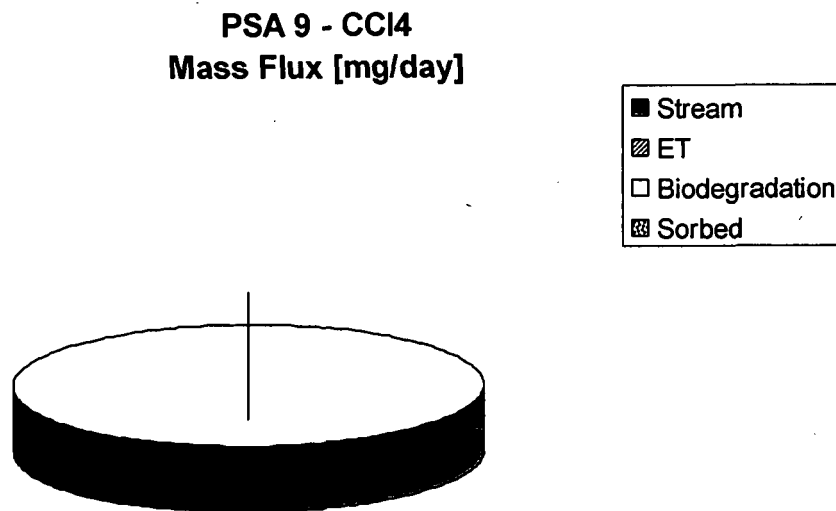


Figure 6.39. PSA 9 - Simulated steady-state mass flux for CCl<sub>4</sub>.

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### 6.2.1.6 PSA 10 (Building 444 Area)

The PSA 10 model area is defined on Figure 6.1 and Figure 6.40. Only the PCE VOC-chain was simulated in this area.

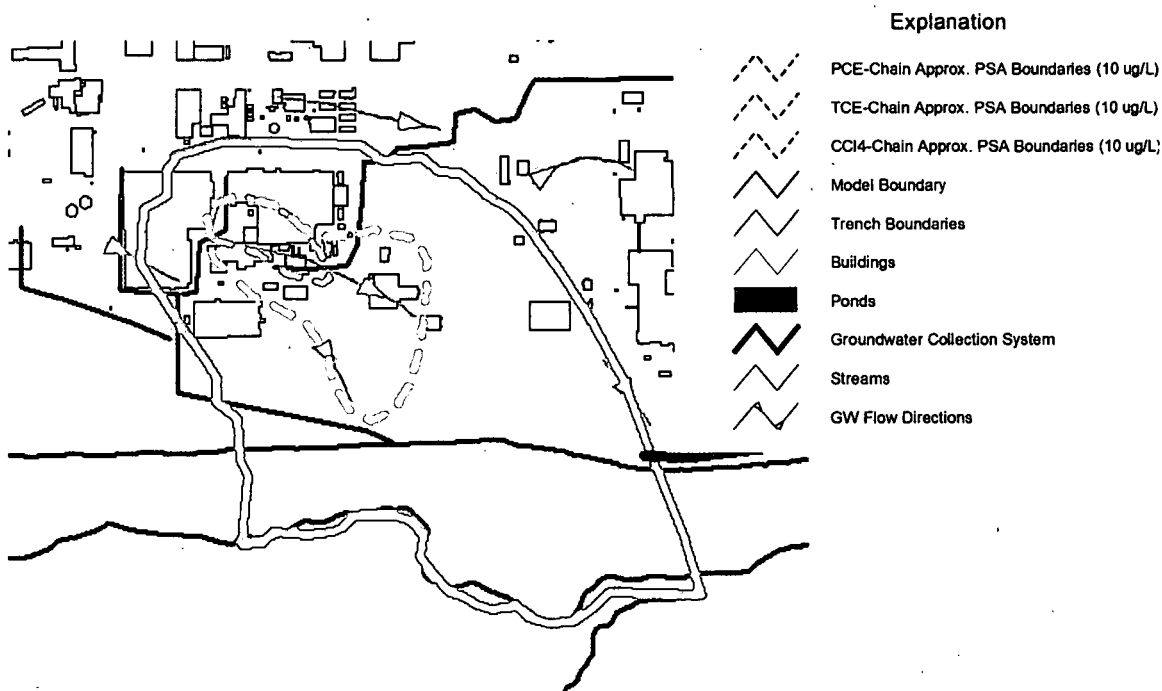


Figure 6.40. PSA 10 - Model area and PSA.

#### 6.2.1.6.1 Groundwater Flow Model Results

Constant head boundaries for PSA 10 were placed along the northern and western up-gradient boundaries, where groundwater flows into the model. They were also specified along the eastern and southern down-gradient boundary (along Woman Creek) where groundwater flow discharges. Simulated water balance results indicated that discharge occurred primarily as ET and to a lesser extent, baseflow and drain discharge. Simulated heads compared well with observed groundwater levels in the model area, with residuals ranging from 1 to 2 meters (Figure 6.41).

#### 6.2.1.6.2 Simulated Groundwater VOC Sources

Three Priority 1 releases and three Priority 2 releases were identified in PSA 10 for PCE and TCE. The Priority 1 releases were described as "B44/B453 Drum Storage Area", "B444", and "B447" (HRR Reference No. 10, Appendix A). HRR information indicated these releases were discovered between 1951 and 1957, and PCE and TCA-chain contaminants; as well as unknown volumes of solvents,

oil-grease, and nitrates were found. The Priority 2 releases were described as "South Loading Dock", "Radioactive Site", and "Uranium Chip Roaster" (HRR Reference No. 88, and 90-93, Appendix A). The HRR also indicated that these releases were discovered between 1953 and 1956 (approximately) and included TCE and CCl<sub>4</sub>- chain contaminants and unknown volumes of solvents and oil-grease.

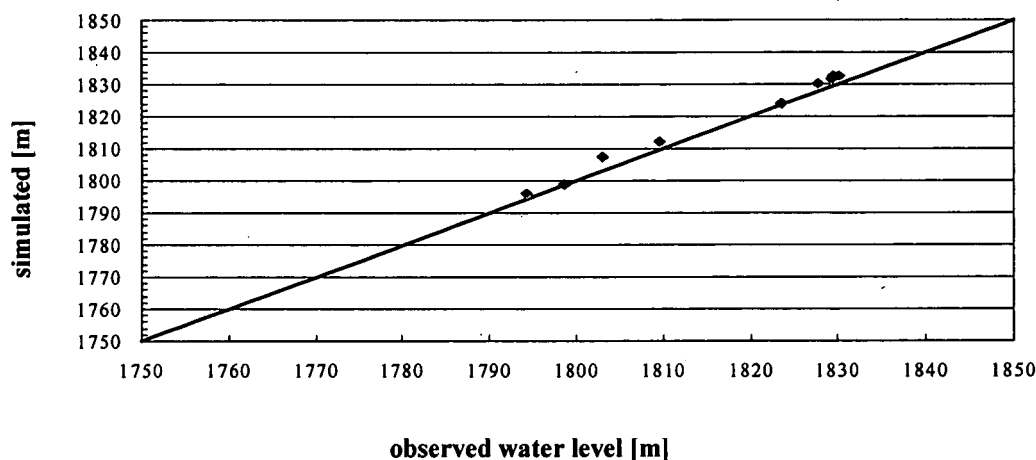


Figure 6.41. PSA 10 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.6.3 Particle Tracking Results – Historical Conditions

Particles introduced into the model at these locations were allowed to travel for 50 years. Source particles were entered into the lower unconsolidated material because it is relatively thick and remained saturated in the steady-state flow model.

Particle tracking results (shown on Figure 6.42) agreed well with the assumed PSA 10 extent, and pathways did not intersect either the SID or Woman Creek. Results showed that flow directions were generally south to southeast. Subsurface drains (shown in green) affected the pathways only slightly. Pathways were affected more by unsaturated unconsolidated material areas along the hillslope, which caused changes in the velocity, and by the pathway at the southern extent. Time-averaged well VOC concentrations in the southern PSA area are probably due to dispersion, not advective transport.

#### 6.2.1.6.4 Transport Model Results – Historical Conditions

TCE was detected above the draft surface water PRG level (0.19 mg/L) within the PSA 10 area. PCE was detected at a maximum average historical concentration of 0.25 mg/L, which was below the draft surface water PRG level

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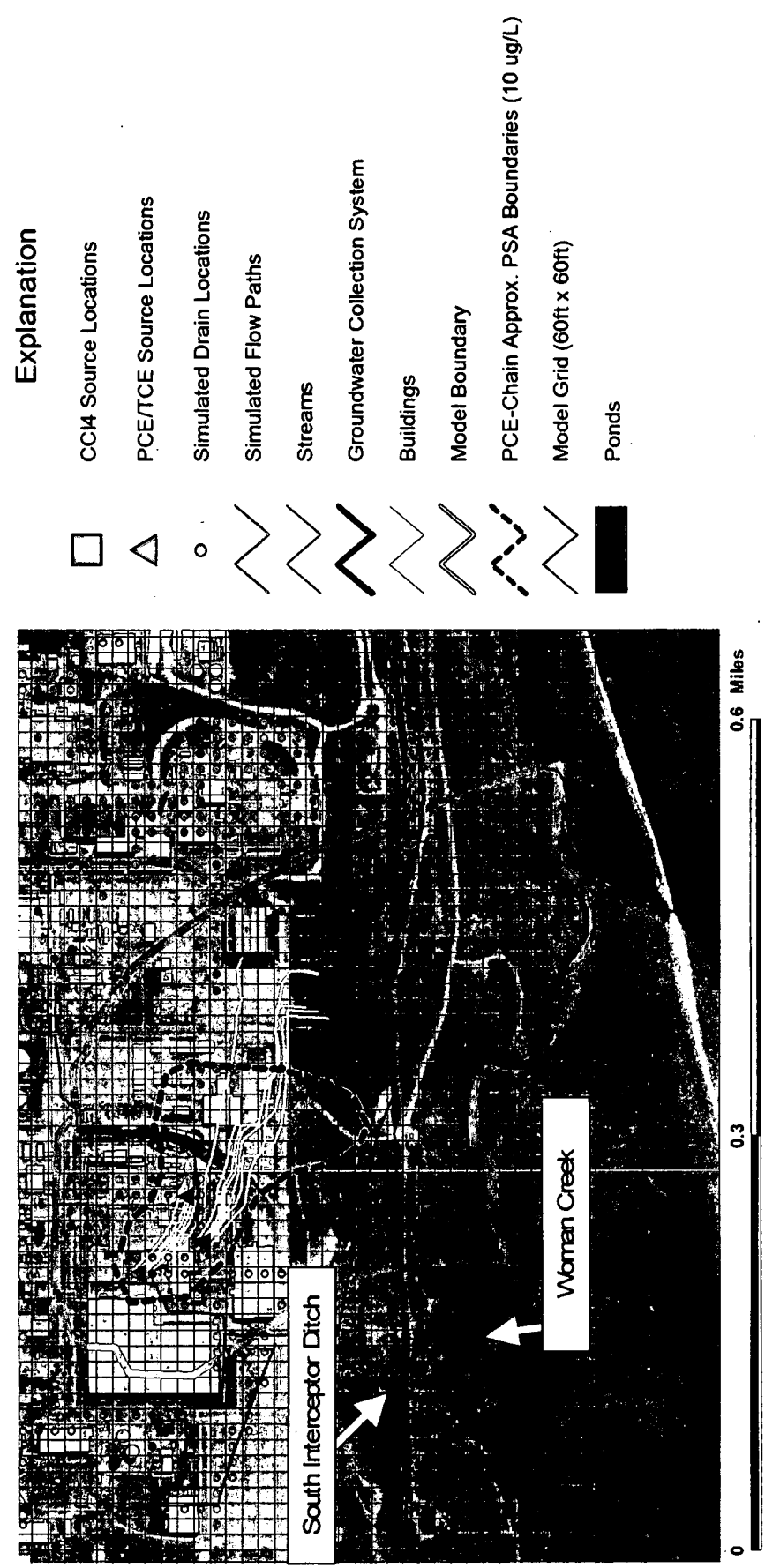


Figure 6.42. PSA 10 - Simulated flow paths (after 50 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

of 1.46 mg/L.  $\text{CCl}_4$  was not detected above 10 ug/L within the PSA 10 area. Estimated PSA boundaries for the PCE chain are shown on Figures B-1 through B-4 (in Appendix B).

Plots showing the log residual concentrations for TCE (i.e., log of observed minus simulated concentrations) are shown on Figures 6.43 and 6.44. Results of sensitivity simulations indicated that the highest down-gradient concentrations (Figure 6.43) were produced with low degradation values and the lowest down-gradient concentrations (Figure 6.44) were produced with high degradation values. The two southern-most observation points are located across the SID and are insensitive to the high and low concentration model runs. Emphasis was placed on reproducing down-gradient wells, as near-source well concentrations may not reflect well-mixed groundwater concentrations.

Down-gradient concentrations (close to Woman Creek and the SID) were only slightly sensitive to parameter values changes in the model, while sample locations south of Woman Creek had no response (Figure 6.43 and Figure 6.44). A long groundwater flow path from the up-gradient source area allowed for longer residence times and increased potential for degradation. Additionally, as groundwater nears the streams, VOC loss to ET increases. In general, model simulations indicated that groundwater travels within the upper, unconsolidated material in the PSA 10 area.

The PSA 10 model performed well. Sensitivity simulations bracketed the observed average historical concentrations of parent and daughter products and performed as expected for each parameter change (based on descriptions in Table 6.3). The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain) was bracketed by the various sensitivity runs with the high source concentration (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain are shown on Figure D-9 and Figure D-10 of Appendix D.

#### **6.2.1.6.5 Transport Model Results – Closure Configuration**

Results of the integrated flow modeling of the closure configuration showed that groundwater flow directions up-gradient of the SID in PSA 10 changed slightly. Under current conditions, flows from inferred VOC sources near Building 444 were directed southeastward, then southward along the hillslope towards the SID and Woman Creek. For the closure configuration, the integrated flow model simulated flow directions near the inferred source area in a more eastward direction than current conditions, but then is redirected southward toward the SID and Woman Creek. This change is due to notable modifications (i.e., removal of subsurface drains, buildings, and pavement) in the inferred source areas. Groundwater in the PSA 10 area flows in both the upper unconsolidated material and weathered bedrock layers.

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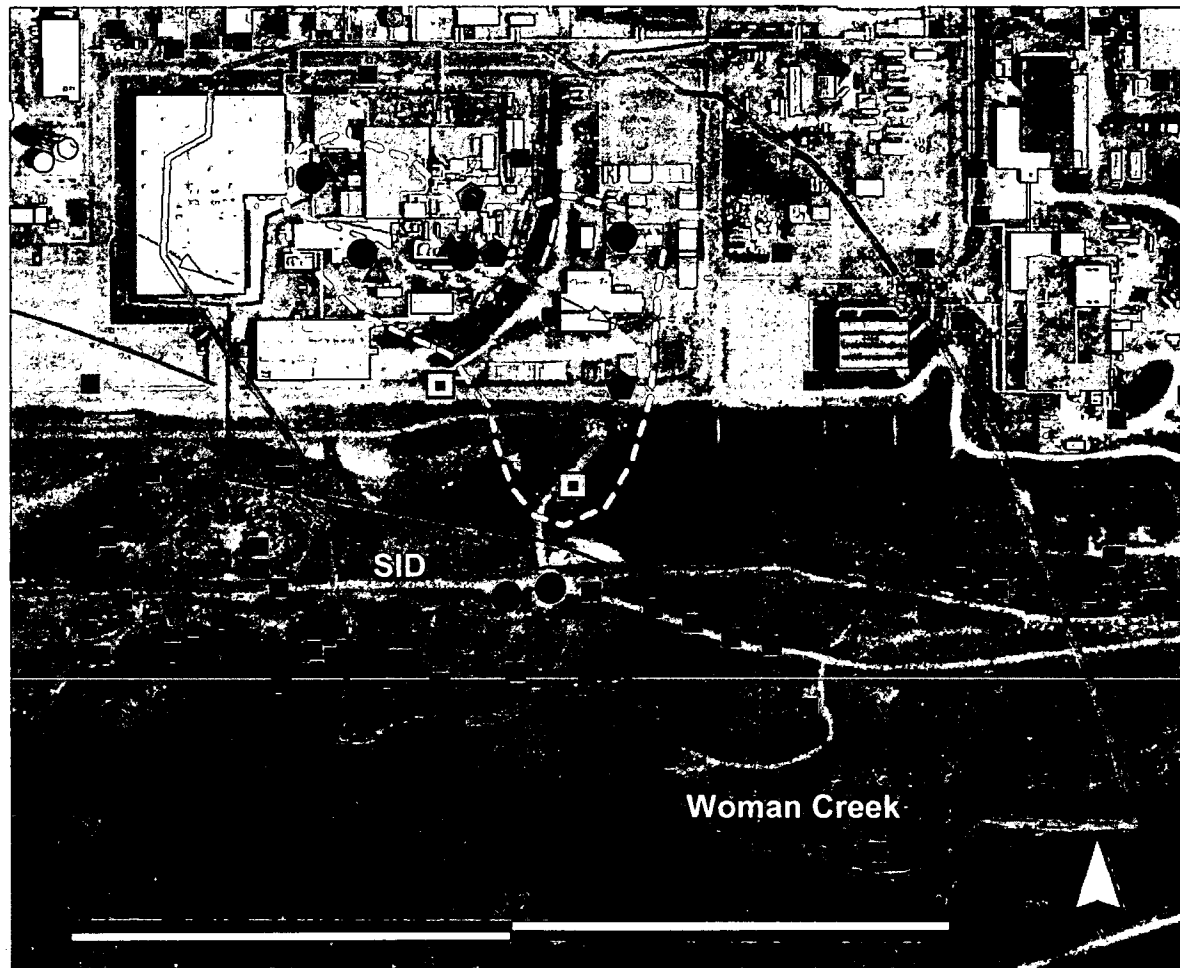
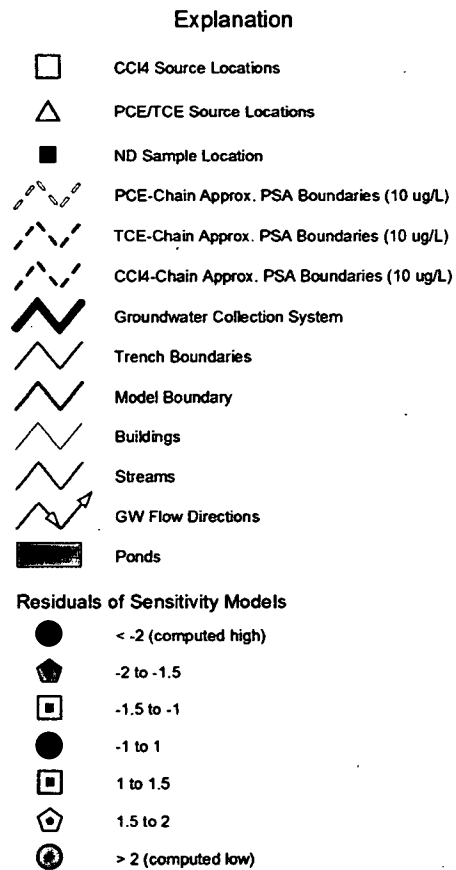
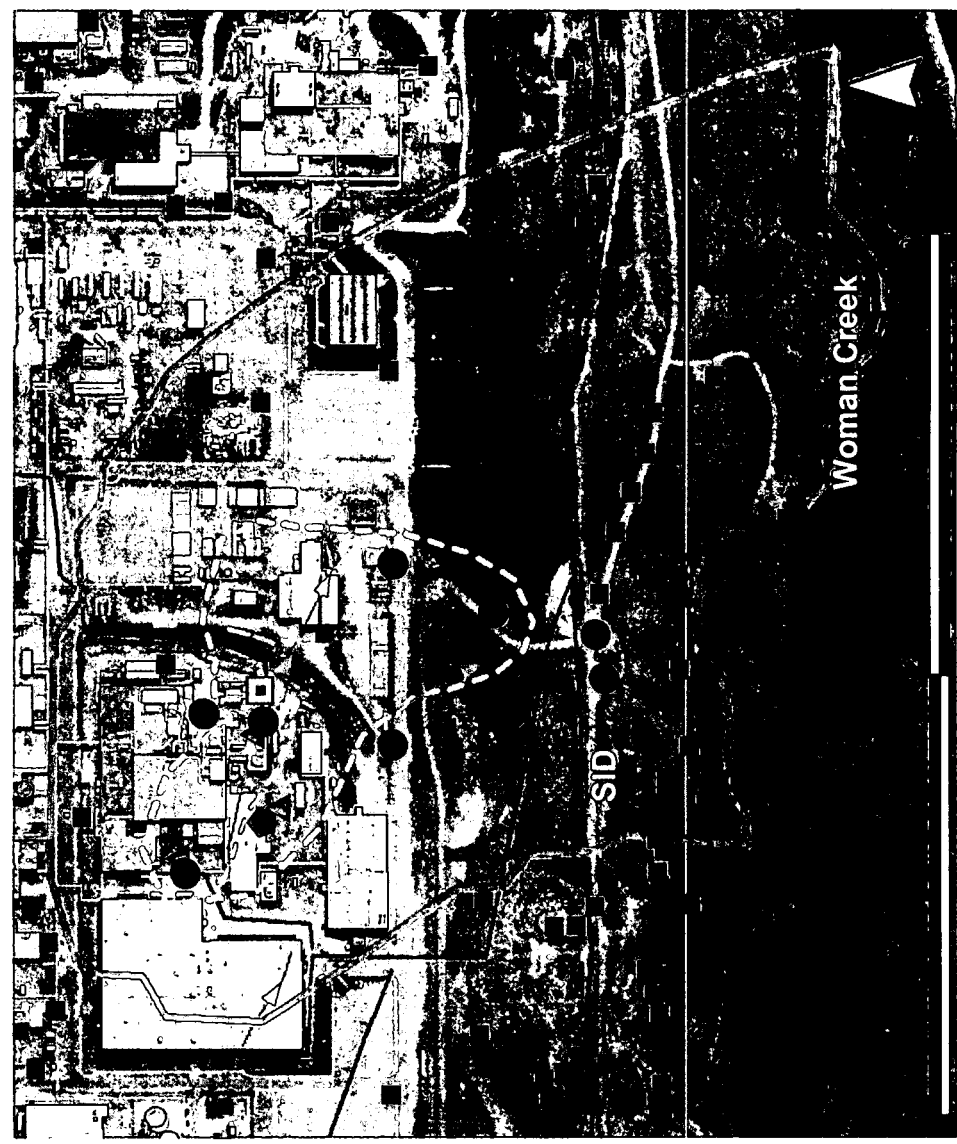


Figure 6.43. PSA 10 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (low degradation).

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- Explanation**
- CCM Source Locations
  - △ PCE/TCE Source Locations
  - ND Sample Location
  - PCE-Chain Approx. PSA Boundaries (10 ug/L)
  - TCE-Chain Approx. PSA Boundaries (10 ug/L)
  - CCM-Chain Approx. PSA Boundaries (10 ug/L)
  - Groundwater Collection System
  - Trench Boundaries
  - Model Boundary
  - Buildings
  - Streams
  - GW Flow Directions
  - Ponds
- Residuals of Sensitivity Models**
- < -2 (computed high)
  - ◐ -2 to -1.5
  - ◑ -1.5 to -1
  - ◒ -1 to 1
  - ◓ 1 to 1.5
  - ◔ 1.5 to 2
  - ⊕ > 2 (computed low)

Figure 6.44. PSA 10 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (high degradation).

A total of 16 transport simulations were run using the PSA 10 model. All runs were simulated for 100 years to establish steady long-term concentrations with time at the southern PSA 10 extent, along Woman Creek and the SID. Figure 6.45 shows simulated groundwater concentrations for the 16 sensitivity runs at selected groundwater discharge locations along Woman Creek and the SID. Simulated steady-state concentrations ranged from non-detect (zero) to a approximately 0.04 mg/L. Therefore, none of 16 simulated steady-state concentrations for PCE, or TCE at groundwater discharge locations along the SID, or Woman Creek were higher than the draft surface water PRGs.

On Figure 6.46, simulated steady-state VOC mass flux from the PSA 10 model area shows that the simulated dominant loss mechanism for VOCs was biodegradation. Only a small fraction is lost as baseflow to the SID and Woman Creek.

#### **6.2.1.7 PSA 12 (Central IA)**

The PSA 12 model area, located in the Central IA, contains buildings 551, 559, and 371. The model area is defined on Figure 6.1 and Figure 6.47. Three separate PSAs were defined within the PSA 12 model area (PSA 11, PSA 12, and PSA 13) for the PCE/TCE-chain, and only one PSA (described in Appendix B).

##### **6.2.1.7.1 Groundwater Water Model Results**

Constant heads were specified around the entire perimeter of the PSA 12 model area to simulate both groundwater inflow and outflow. The stream located between Buildings 371 and 771 was simulated as a discharge boundary using the MODFLOW river package.

Simulated water balance results indicated that discharge was mainly to ET, baseflow to the stream located between Building 371 and 771, and to drains in the area. Simulated heads compared well to observed groundwater levels; residuals were generally less than 1 meter (as shown on Figure 6.48).

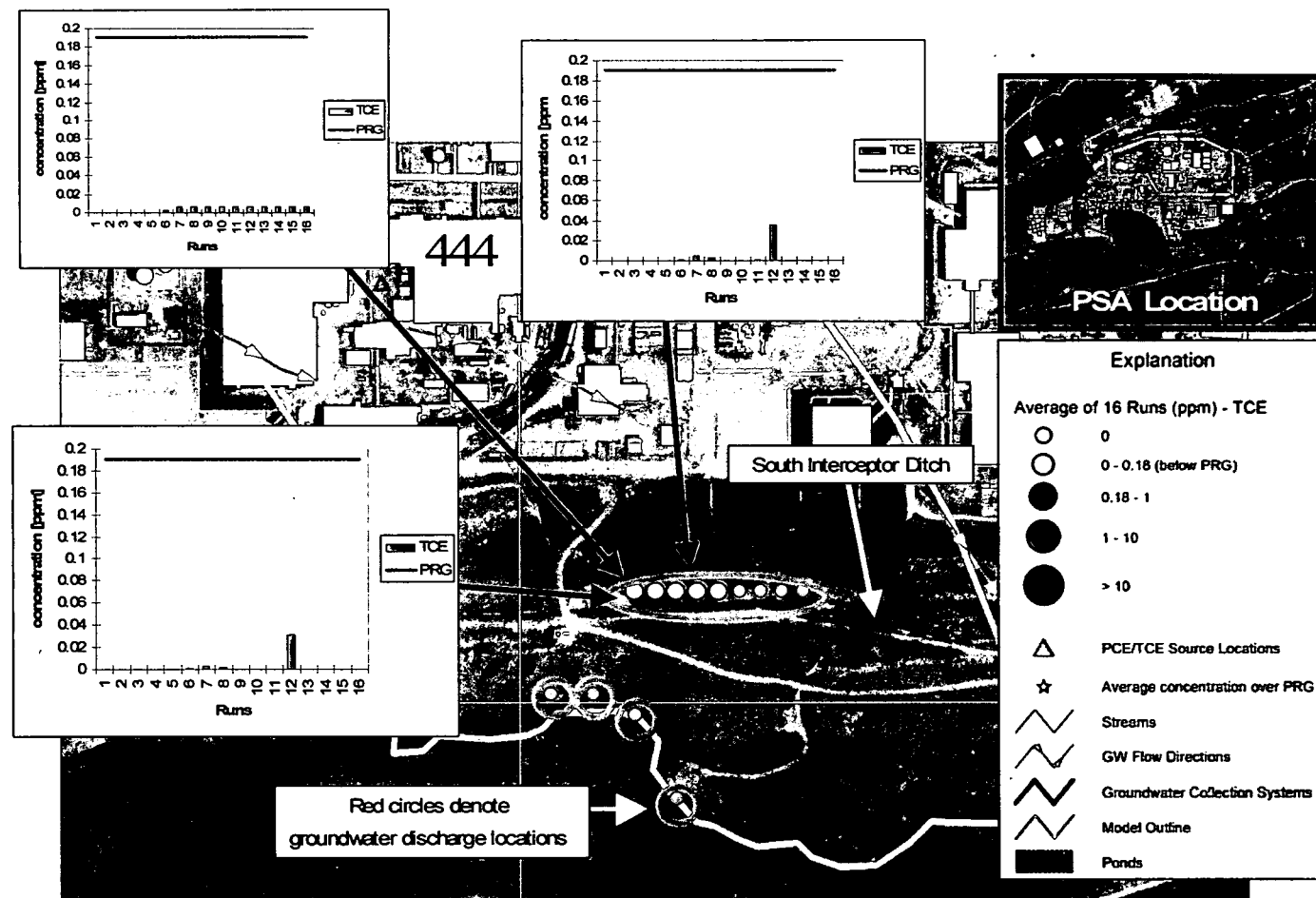
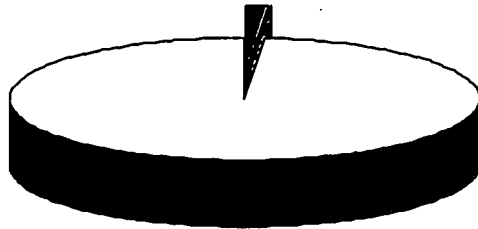
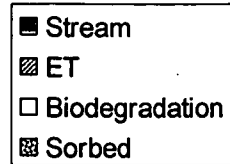
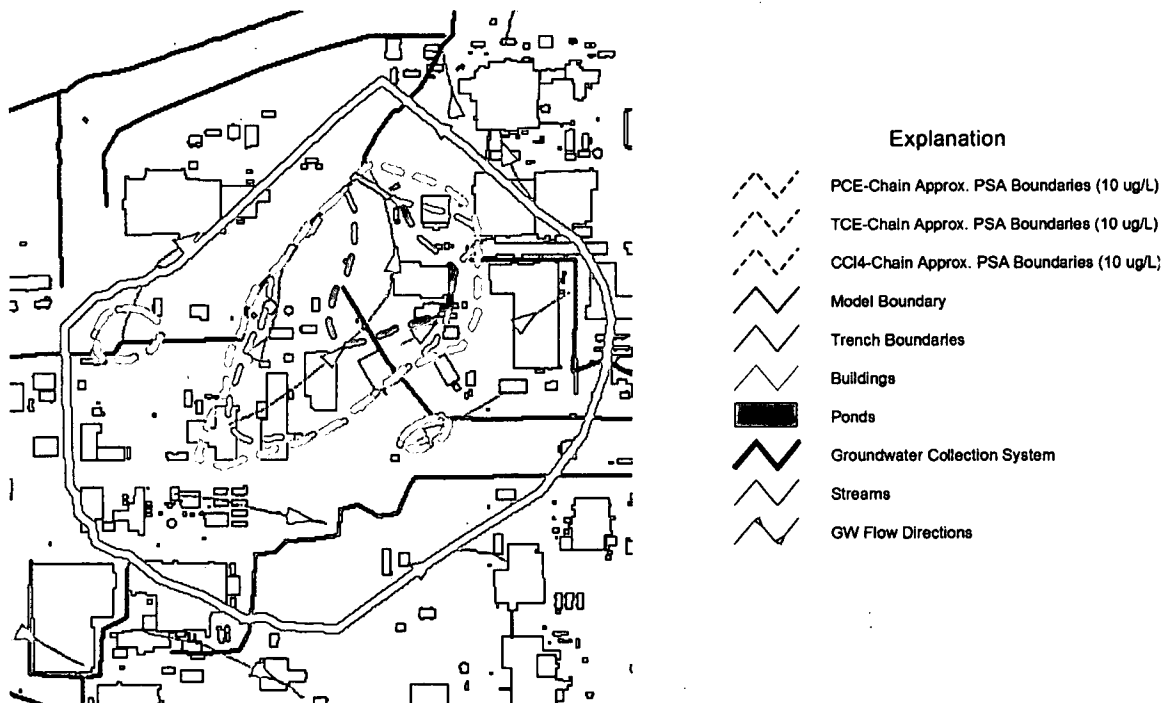


Figure 6.45. PSA 10 - Simulated steady-state TCE groundwater concentrations at groundwater discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

**PSA 10 - TCE**  
**Mass Flux [mg/day]**



**Figure 6.46. PSA 10 - Steady-state mass flux for TCE.**



**Figure 6.47. PSA 12 - Model area and PSA 11, PSA 12 and PSA 13.**

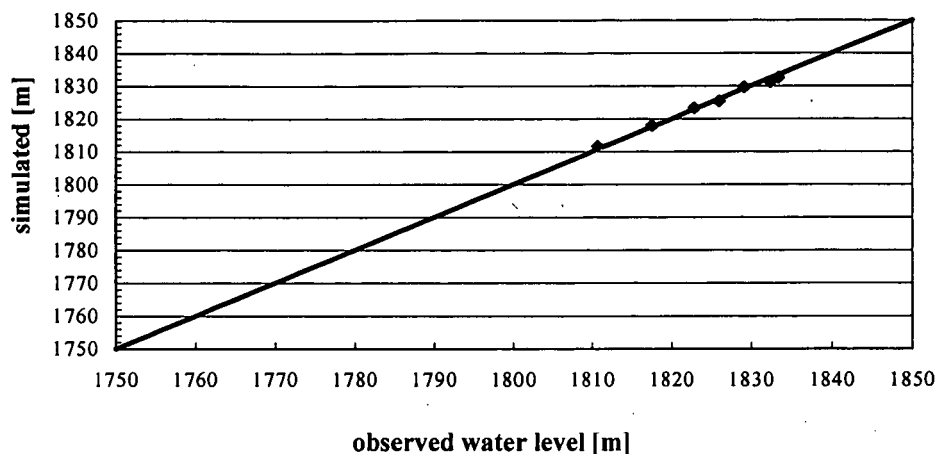


Figure 6.48. PSA 12 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.7.2 Simulated Groundwater VOC Sources

Several VOC source areas were inferred within the PSA 12 model area based on HRR information. Many Priority 1 and Priority 2 releases were identified (Appendix A). Within the PSA 12 model area, sources were inferred for each of the PSAs, for example:

- PSA 11, shown on Figures B-1 through B-4 (Appendix B), does not include Priority 1 or Priority 2 releases, but a source was inferred west of a former control building based on interviews with SMEs;
- PSA 12, shown on Figures B-1 through B-4 (Appendix B), contains several Priority 1 and Priority 2 releases. Releases include: "B371 Parking Lot", associated with solvent and oil grease in 1971; the "North Chemical Storage" and "Middle Chemical Storage", associated with solvent and oil grease (HRR Reference No. 11 and 320, respectively, in Appendix A) and identified between 1959 and 1971; "Building 551 Chemical Leaks", associated with solvent and oil grease and identified in 1974; "Valve Vaults", associated with solvent and oil grease and identified in 1971; and "Scrap Metal Sites", associated with oil grease and identified in 1958 (HRR Reference No. 12 in Appendix A); and
- Only VC within the PSA 13 area was higher than draft surface water PRG values. This area, shown on Figures B-1 through B-4 (Appendix B), contains two Priority 1 releases. The releases were associated with the "Oil Burn Pit" and the "Solvent Burning Ground" (HRR Reference No. 6



and 179, respectively, in Appendix A). These sites were identified between 1956 and 1961 and included oil grease and solvents.

#### **6.2.1.7.3 Particle Tracking Results – Historical Conditions**

Particles were introduced into the lower unconsolidated material model at inferred VOC source locations and allowed to travel for 50 years based on HRR information. Particles were introduced into the lower unconsolidated material because the relatively thick unconsolidated material remained saturated in steady-state flow model simulations.

Particle tracking results (shown on Figure 6.49) confirmed that assumed source depths, locations, and release timing were reasonable. Distances traveled in the assumed 50 years did not extend beyond any of the assumed PSAs except for PSA 13 (see Figures B-1 to B-4 in Appendix B).

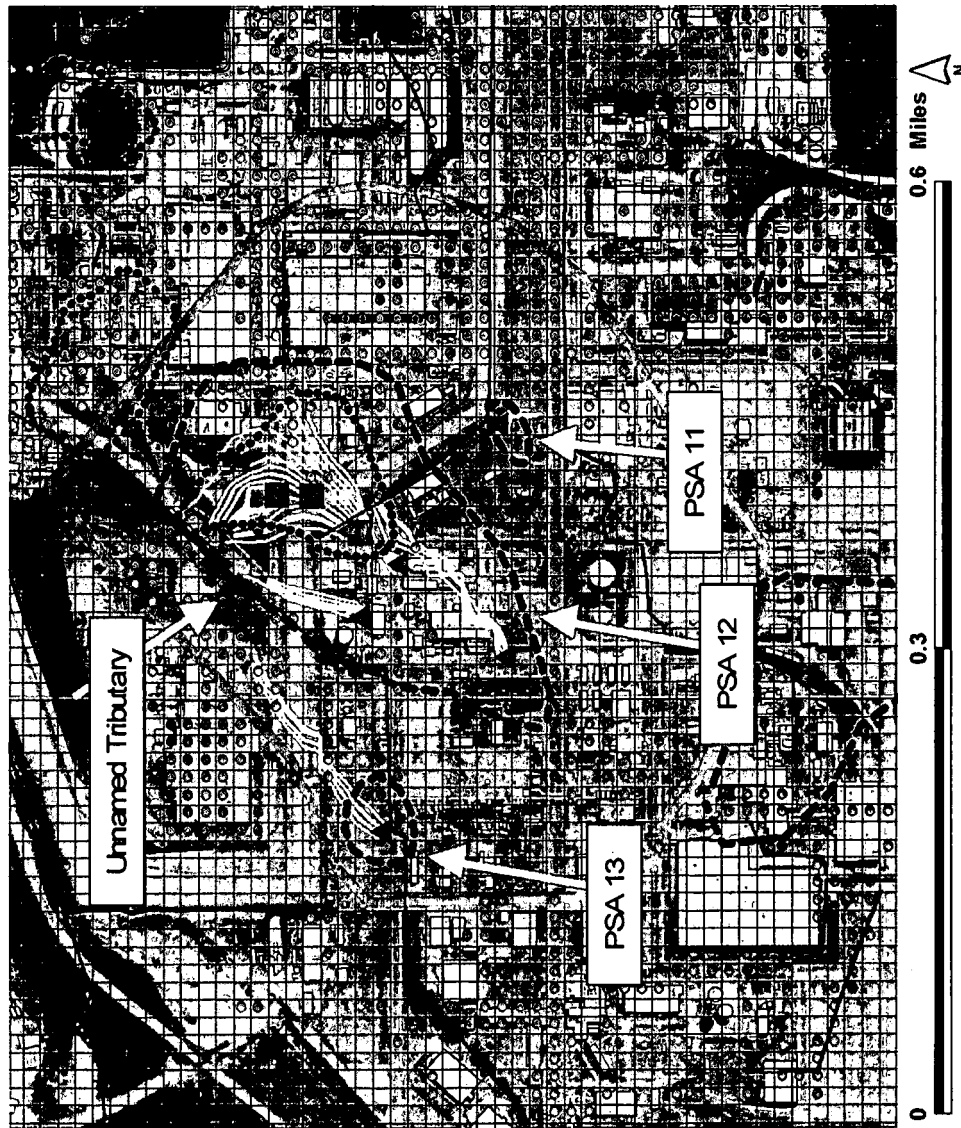
#### **6.2.1.7.4 Transport Model Results – Historical Conditions**

In the PSA 12 area, TCE was detected above the draft surface water PRG level (0.19 mg/L), PCE was detected at a maximum average historical concentration of 0.31 mg/L (below the draft surface water PRG level of 1.46 mg/L), and  $\text{CCl}_4$  was detected above the draft surface water PRG levels (0.58 mg/L). The  $\text{CCl}_4$ -chain was not observed at concentrations above the draft PRG for PSA 11 and 13. Only concentrations of VC at PSA 13 were above the draft PRG. A total of nine PCE/TCE chain sources were inferred. Only three  $\text{CCl}_4$  sources were inferred.

The PSA 13 area was unique in that observed VC concentrations at two sample locations exceeded the PRG, while PCE and TCE (parent compounds of VC) were not detected at or above PRG values. VC has a high vapor pressure and is typically present as a gas at temperatures above  $-14^\circ\text{C}$ . To reproduce the time-averaged concentration distribution of VC (as a degradation product) in the PSA 13 area, PCE and TCE concentrations had to be specified well above the time-averaged concentrations at the inferred VC source locations. Results of model simulations indicated that VC degraded to concentrations below PRG values for closure simulations using a range of VC degradation similar to those specified for other PSA models.

Plots showing the log residual concentrations for TCE (i.e., log of time-averaged minus simulated concentrations) are shown on Figures 6.50 and 6.51. Results of sensitivity runs indicated that the highest down-gradient concentrations (Figure 6.50) were produced with low degradation values and the lowest down-gradient concentrations (Figure 6.51) were produced with high porosity values. Emphasis was placed on reproducing down-gradient wells, as near-source well concentrations may not reflect well-mixed groundwater concentrations.

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Explanation	
CCl4 Source Locations	□
PCE/TCE Source Locations	△
Simulated Drain Locations	○
Simulated Flow Paths - PCE Chain	↘
Simulated Flow Paths - CCl4 Chain	↗
Streams	—
Groundwater Collection System	—
Buildings	—
Model Boundary	—
PCE-Chain Approx. PSA Boundaries (10 ug/L)	—
CCl4-Chain Approx. PSA Boundaries (10 ug/L)	—
Model Grid (60ft x 60ft)	—
Drainage	—

Figure 6.49. PSA 12 - Simulated flow paths (after 50 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

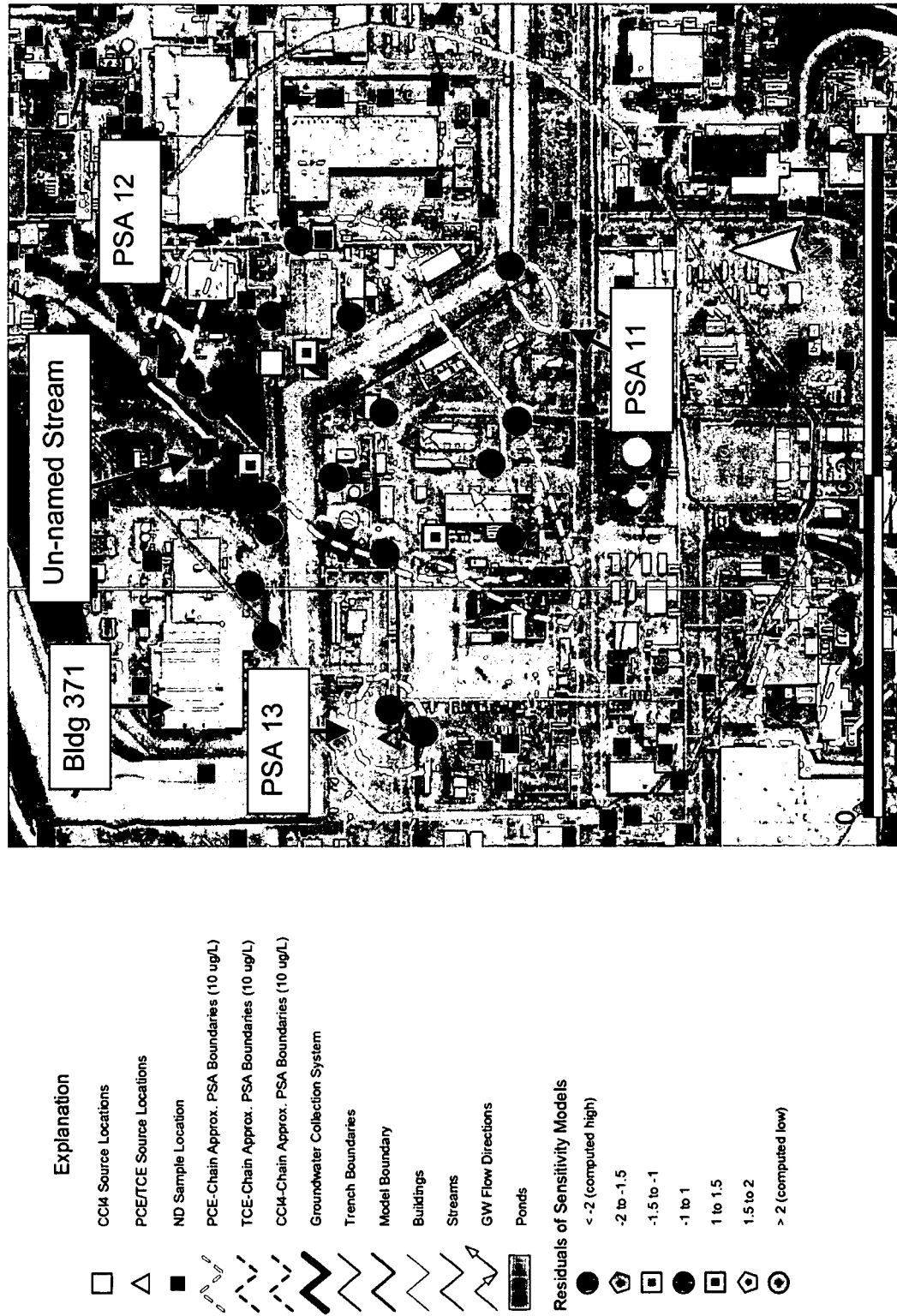
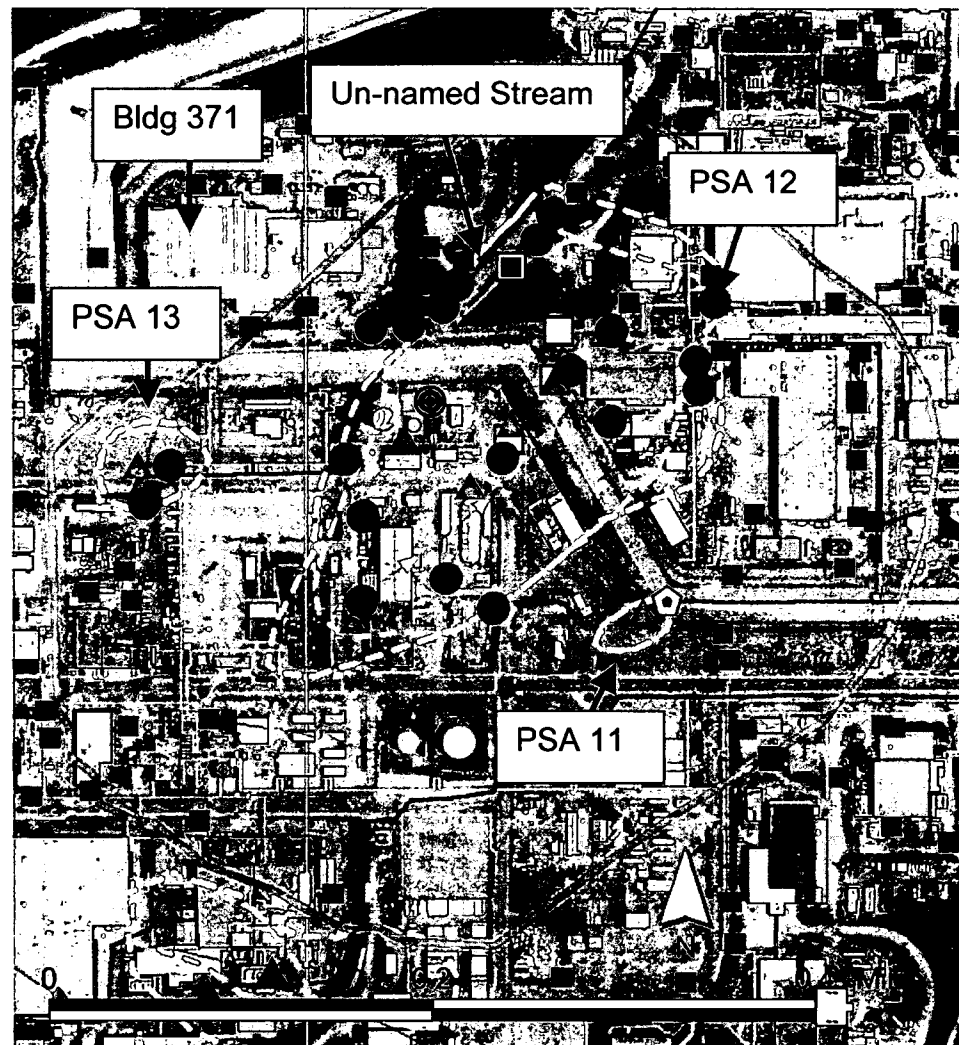
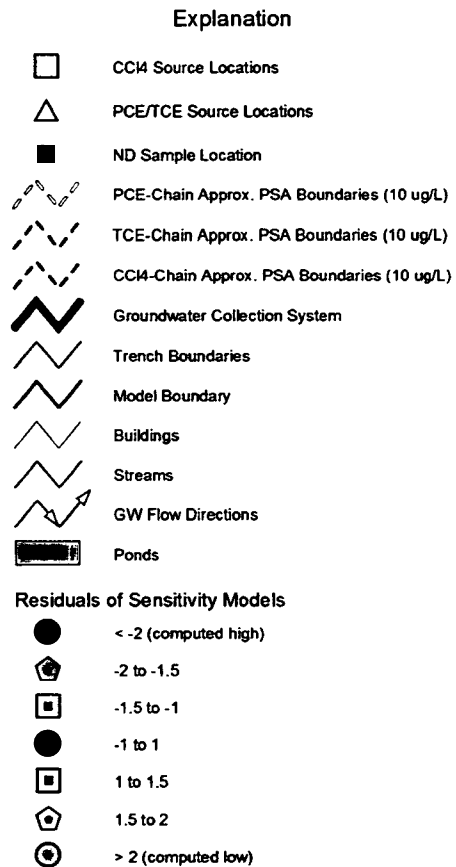


Figure 6.50. PSA 12 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (low degradation).



**Figure 6.51. PSA 12 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (high porosity).**

Model simulations indicated that groundwater travels in the upper unconsolidated material and discharges as ET, or baseflow to the IA drainage between buildings 371 and 771. Surface water VOC concentration data were unavailable to confirm simulated groundwater concentrations at the IA drainage in this PSA.

Sensitivity simulations bracketed the observed time-averaged parent/daughter concentrations of parent and daughter products and performed as expected for each parameter change (based on descriptions in Table 6.3). The time-averaged concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain and chloroform and methylene chloride for the CCl<sub>4</sub> degradation chain) was bracketed by the various sensitivity runs, with high concentrations (simulated high daughter product concentrations) and low degradation (simulated low daughter product concentrations) cases for the PCE degradation chain. Results are shown on Figure D-11 and Figure D-12 of Appendix D.

#### **6.2.1.7.5 Transport Model Results – Closure Configuration**

Groundwater in the PSA 12 model area under current conditions flowed to the north (towards Walnut Creek) within the upper unconsolidated material and lower weathered bedrock. Under the closure configuration changes to the groundwater flow directions did not significantly impact the PSA 12 area.

A total of 16 transport simulations were run using the PSA 12 model. All runs were simulated for at least 100 years to establish steady long-term concentrations with time at the PSA extent along the stream located between Building 371 and 771. Simulated long-term TCE and CCl<sub>4</sub> groundwater concentrations at discharge locations along the tributary in the northwestern model area for the 16 model simulations are summarized on Figures 6.52 and 6.53, respectively. For these runs, steady-state simulated concentrations ranged from zero to a maximum of approximately 0.18 mg/L for TCE and 0.1 mg/L for CCl<sub>4</sub>. Therefore, none of 16 simulated steady-state concentrations for PCE, TCE, or CCl<sub>4</sub>, at groundwater discharge locations along the SID, or Woman Creek, were higher than the draft surface water PRGs.

Results of the PSA 12 model transport simulations indicate that the dominant loss mechanisms for VOCs are biodegradation and ET (Figures 6.54 and 6.55).

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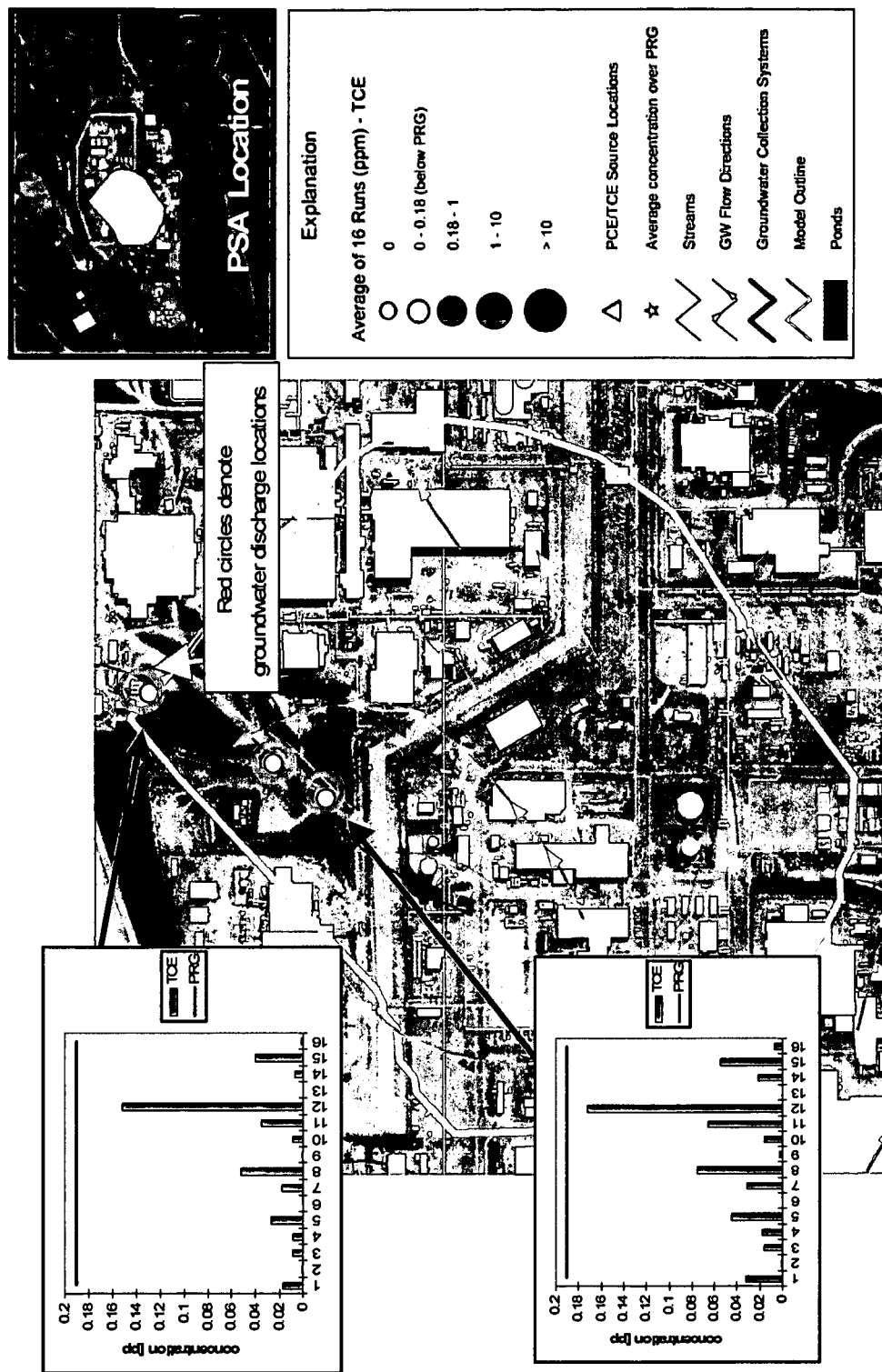


Figure 6.52. PSA 12 - Simulated TCE groundwater concentrations at groundwater discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

17574

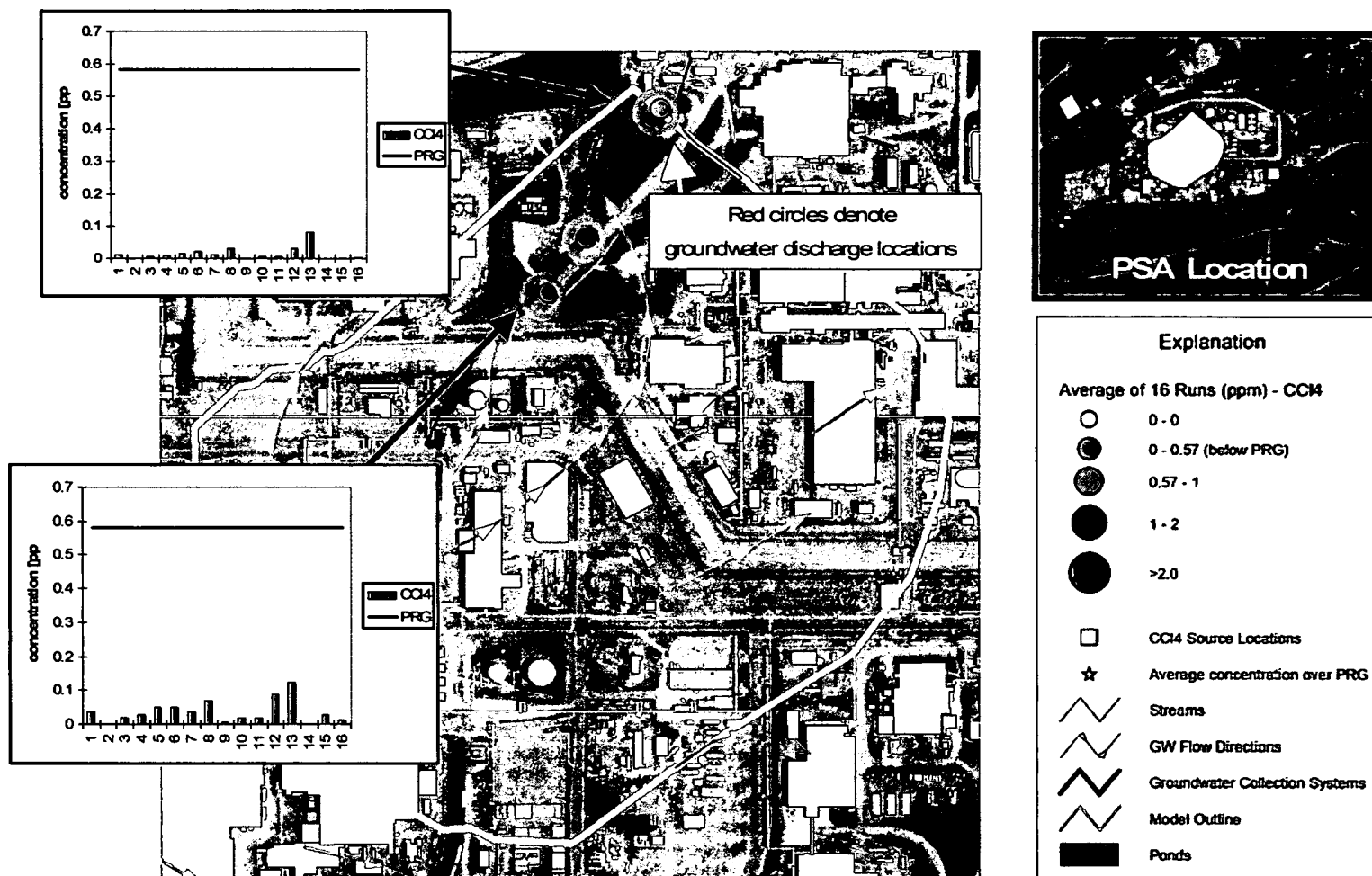


Figure 6.53. PSA 12 - Simulated CCl<sub>4</sub> groundwater concentrations at groundwater discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

**PSA 12 - TCE**  
**Mass Flux [mg/day]**

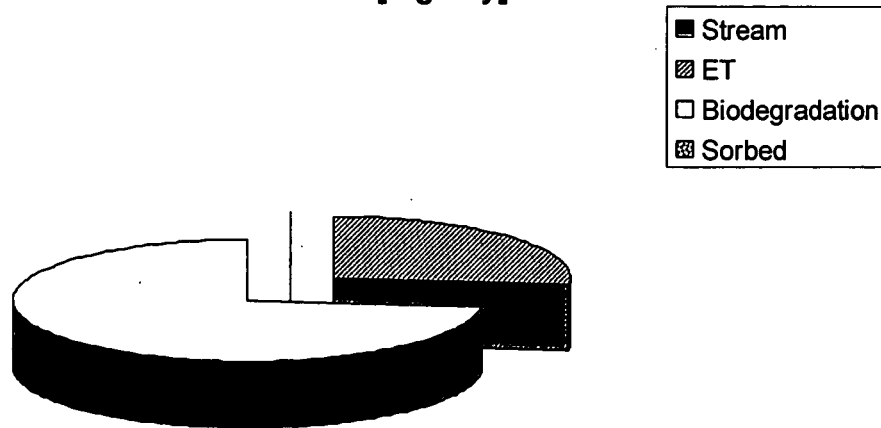


Figure 6.54. PSA 12 - Steady-state mass flux for TCE.

**PSA 12 - CCl<sub>4</sub>**  
**Mass Flux [mg/day]**

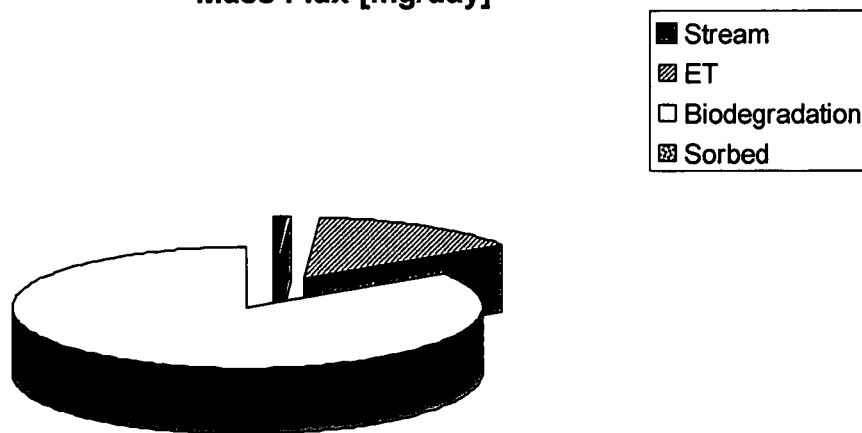


Figure 6.55. PSA 12 - Steady-state mass flux for CCl<sub>4</sub>.



#### 6.2.1.8 PSA 14 (Building 771 – IHSS118.1 Area)

The PSA 14 model area is located in the Building 771 area, and it is defined on Figure 6.1 and Figure 6.56. Only  $\text{CCl}_4$  was simulated in the model area because PCE-chain concentrations were lower than draft surface water PRGs.

##### 6.2.1.8.1 Groundwater Flow Model Results

Constant heads were specified along the entire model perimeter to simulate both groundwater inflow and discharge. Stream flow within the North Walnut Creek and tributary between Building 771 and 371 were simulated using the MODFLOW river package. Only groundwater discharge along these streams was simulated in the steady-state flow model.

Simulated hydraulic heads compare well with observed groundwater well water levels and residuals range from one to two meters (Figure 6.57). Simulated water balance results indicated groundwater discharged mostly as ET, and to a lesser extent, as baseflow or drain flow.

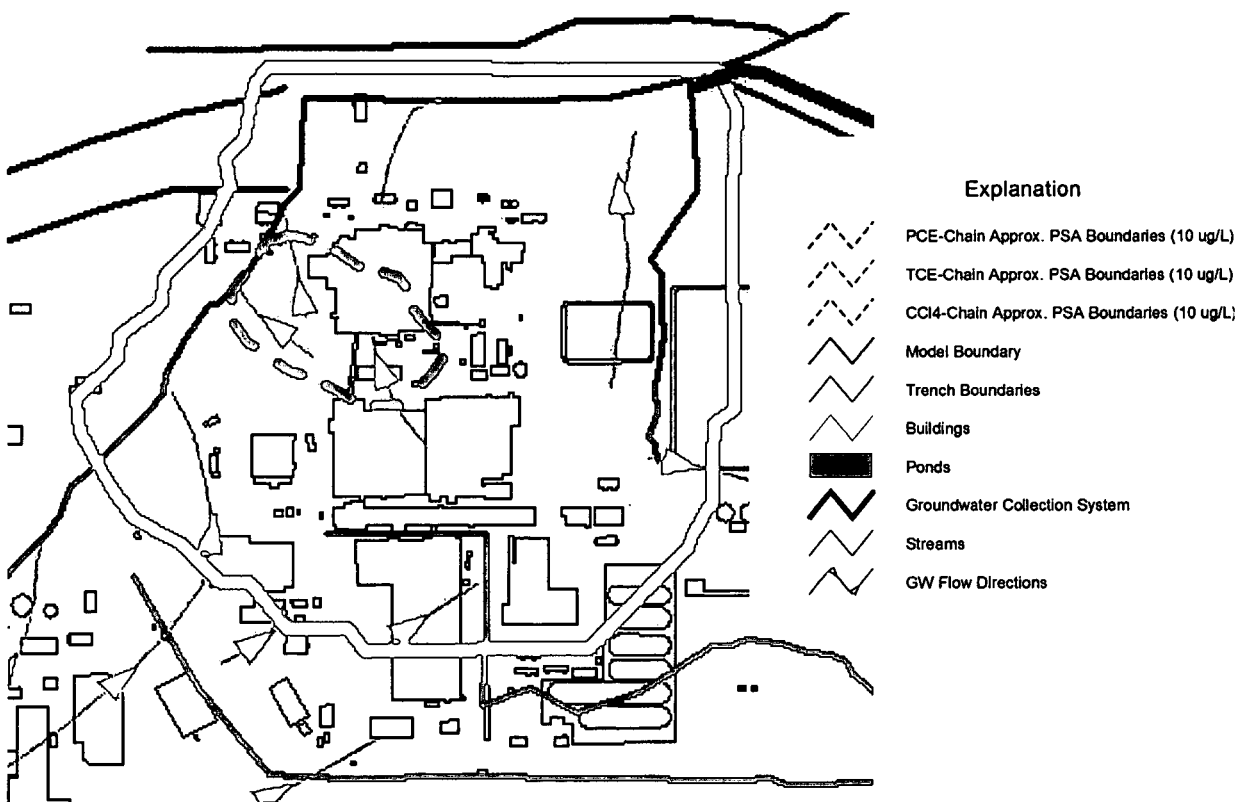


Figure 6.56. PSA 14 - Model area and PSA.

77, 176

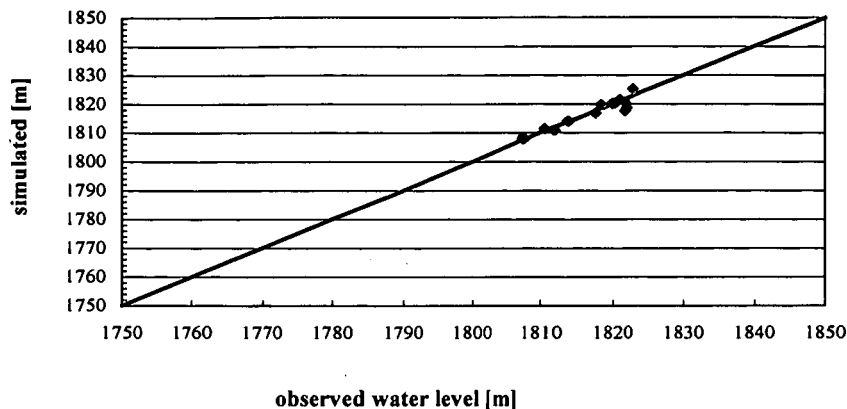


Figure 6.57. PSA 14 - Simulated versus average observed annual groundwater levels (meters above msl).

#### 6.2.1.8.2 Simulated Groundwater VOC Sources

Only the  $\text{CCl}_4$ -chain was observed at concentrations above the draft PRG. Estimated PSA boundaries for the  $\text{CCl}_4$ -chain are shown on Figures B-5 through B-7 (Appendix B). One known release is found in the area and is located north of the Building 776/777 area and south of Building 771. This source is well known (IHSS 118.1) and NAPL has been observed (based on information from SMEs) at the interface between the unconsolidated material and weathered bedrock during field characterization. Historical release information indicates this is a Priority 1 release, identified as "Multiple Solvent Spills west of Bldg. 730", and "South End of Building 776 Solvent Spill" (HRR Reference No. 18 through 20 in Appendix A). The HRR describes the priority area as a  $\text{CCl}_4$  tank discovered leaking prior to 1970. Several occurrences of leaking  $\text{CCl}_4$  (with vague or unknown dates and volumes) were noted in the HRR both before and after this time. The source area for this release is represented by three model cells.

Another  $\text{CCl}_4$  source was inferred west of the known release to reproduce the comparatively high time-averaged concentrations in wells located on the hillslope above the tributary between Building 771 and 371. Time-averaged concentrations in these wells are above the draft surface water PRG values. No known HRR release, or likely source area, could be clearly identified in the area to account for these higher concentrations. Though, model simulations indicated that an inferred source in this area could reproduce time-averaged parent and daughter concentrations in downgradient wells, the location, depth, and source concentration remain unknown.

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177

#### **6.2.1.8.3 Particle Tracking Results – Historical Conditions**

Particles were introduced into the model at the inferred VOC source locations described above, and then allowed to travel for 50 years (Figure 6.58). Particles were entered into the weathered bedrock because the relatively thin overlying unconsolidated material (<3 meters) was largely unsaturated in the source area. Particle tracking results generally confirmed that assumed source locations, depths, and travel times are reasonable. Flow paths to the north beneath the building over-predicted the PSA extent because the affects of the Building 771 footing drains were not captured accurately with the resolution of this model and the flow paths did not consider affects of attenuation processes (i.e. sorption or degradation) on the travel distances.

#### **6.2.1.8.4 Transport Model Results – Historical Conditions**

Plots showing the log residual concentrations for  $\text{CCl}_4$  (i.e., log of observed minus simulated concentrations) are shown on Figures 6.59 and 6.60. Results of sensitivity runs showed that the highest simulated concentrations (Figure 6.59) at well locations were produced with low degradation values, while the lowest concentrations (Figure 6.60) were produced with high degradation values. Emphasis was placed on reproducing down-gradient wells, as near-source well concentrations may not reflect well-mixed groundwater concentrations.

Overall, model simulations indicated that VOC concentrations were removed from the groundwater due to several processes:

- As groundwater moves away from the VOC source, flow was impeded when it reached the extent of the Arapahoe Sandstone to the north and west (Figure 3.1 and 3.2), where it encounters the relatively lower-conductivity weathered bedrock. Slower groundwater flow rates resulted in longer residence times, increasing the potential for degradation. The slower flow rates in the model are due to the thinning of sandstone in the direction of flow, and transition into the lower conductivity claystone/siltstone;
- Increasing ET effects as groundwater nears streams decreased concentrations at stream areas; and
- The effects of the footing drain for Building 771 (south end) removed contaminants and reduced migration of  $\text{CCl}_4$  from the inferred VOC source area to the south. Footing drains also decreased local heads by redirecting flows towards it.

Overall the PSA 14 model sensitivity simulations bracketed the observed average historical concentrations of parent and daughter products and performed as expected for each parameter change (based on descriptions in Table 6.3). The observed concentration distribution of daughter products (chloroform and methylene chloride for the  $\text{CCl}_4$  degradation chain) was bracketed by the various sensitivity runs, with high concentrations (producing high daughter products) and

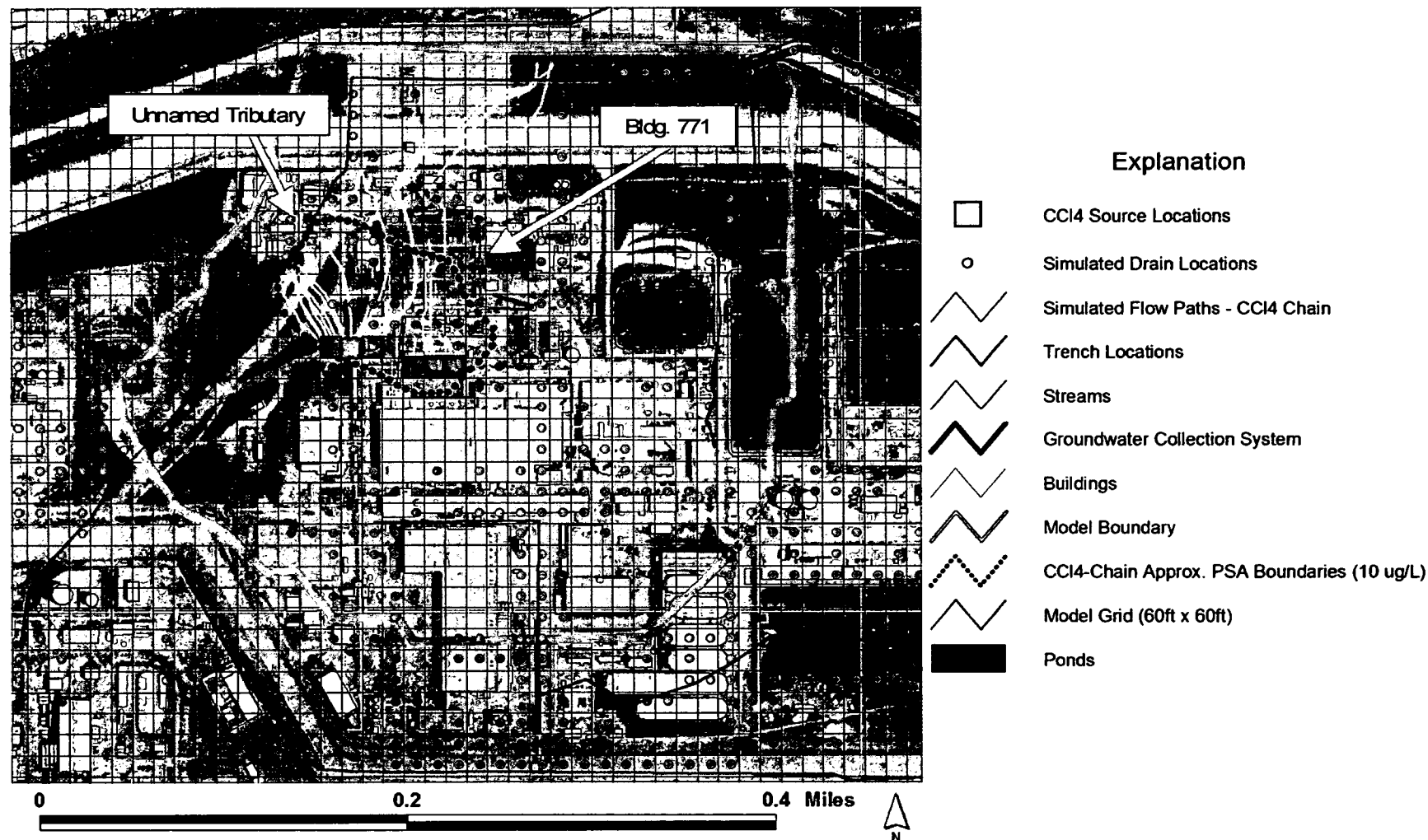


Figure 6.58. PSA 14 - Simulated flow paths (after 50 years). Particles introduced into the lower unconsolidated layer and weathered bedrock.

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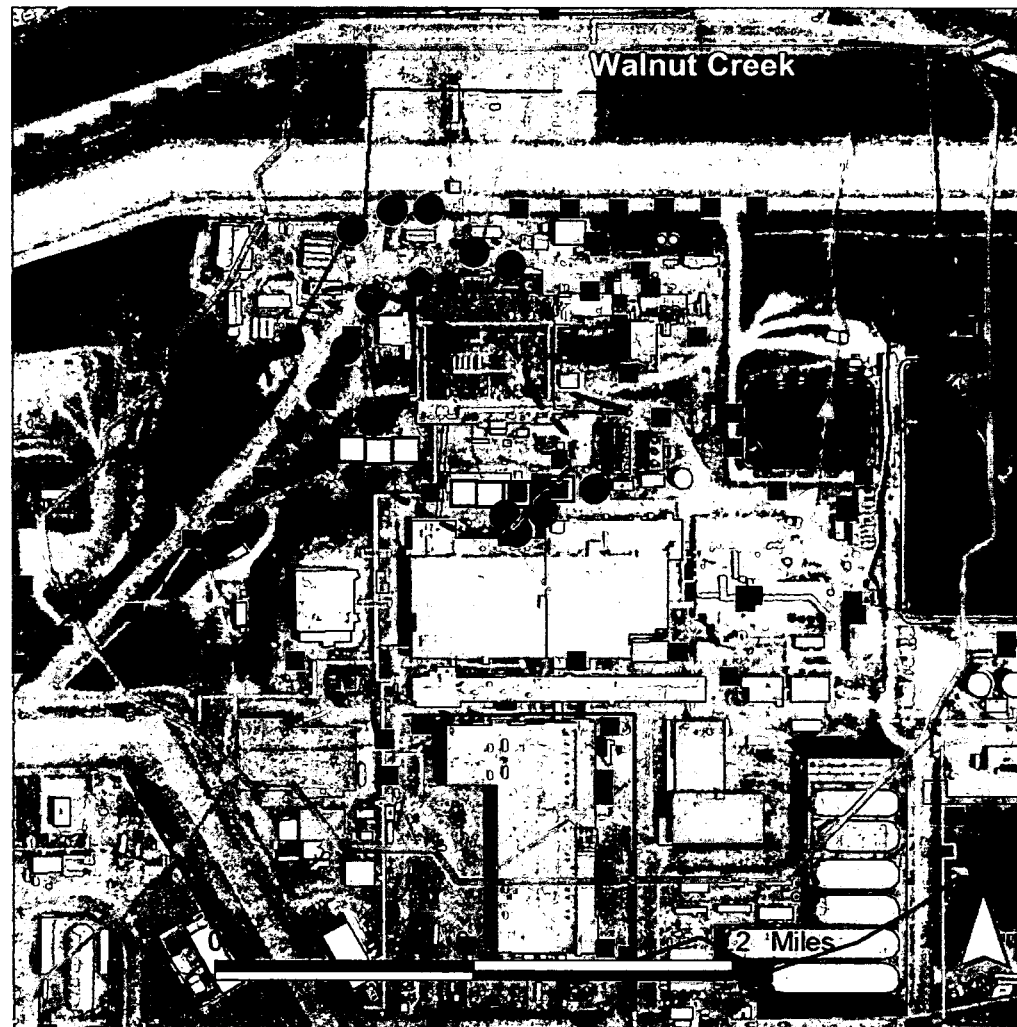
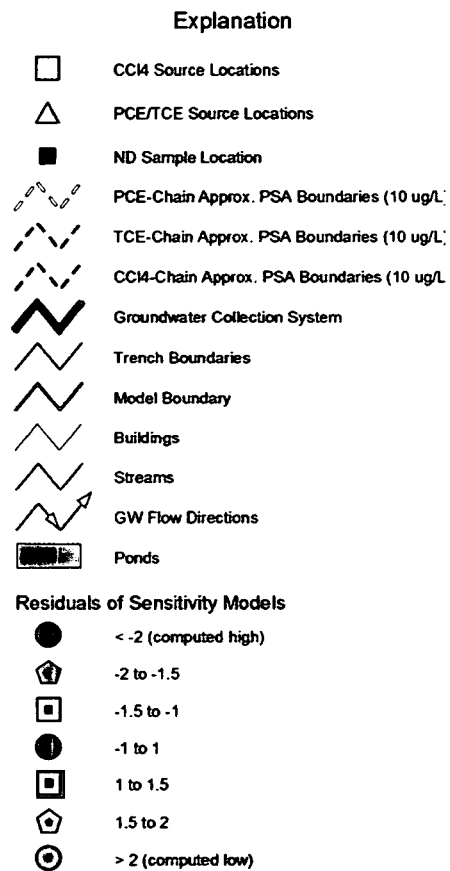


Figure 6.59. PSA 14 – Log residual concentrations. High predicted concentration (CCl<sub>4</sub>) sensitivity run (low degradation).

182-181

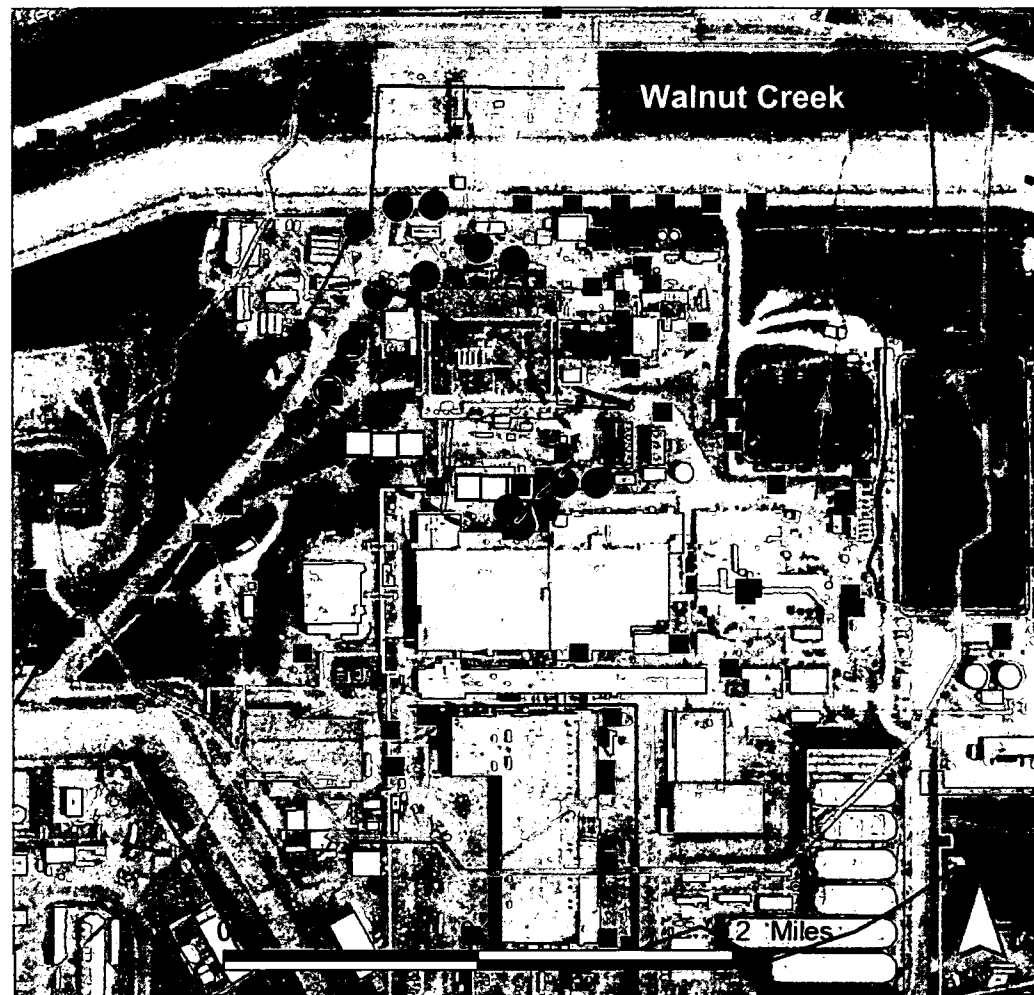
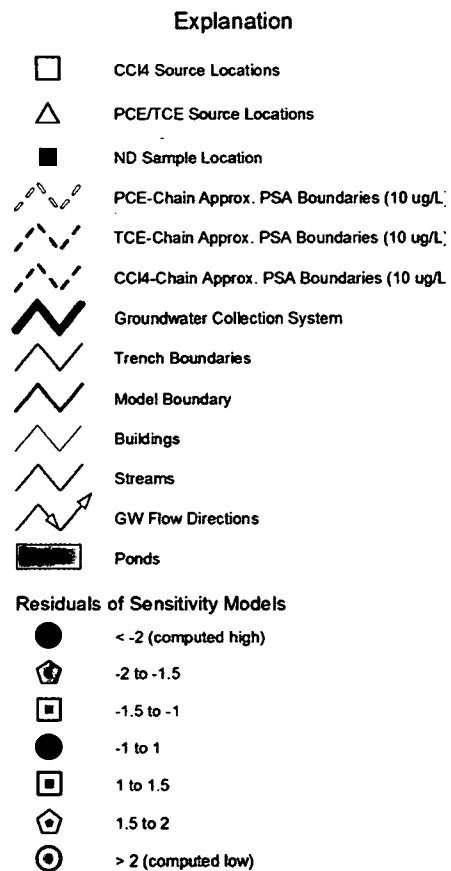


Figure 6.60. PSA 14 – Log residual concentrations. Low predicted concentration (CCl<sub>4</sub>) sensitivity run (high degradation).

low degradation (producing low daughter products) cases for the  $\text{CCl}_4$  degradation chain. The results are shown on Figure D-13 and Figure D-14 of Appendix D.

#### **6.2.1.8.5 Transport Model Results – Closure Configuration**

In the PSA 14 model area, groundwater under current conditions flowed to the north and northwest towards the nearby streams. Although, integrated flow modeling showed that closure configuration modifications increased heads in the area surrounding Building 771, changes to the flow path were not significant. The only exception was that groundwater north of Building 771 flowed more easterly.

Results of closure configuration transport simulations indicated that the time it takes for  $\text{CCl}_4$  to reach steady-state concentrations was approximately 100 years.

A total of 16 transport model runs were simulated. All runs were simulated for at least 100 years to establish steady long-term concentrations with time at the PSA extent along the stream located between Building 771 and 371 and along North Walnut Creek. The simulated groundwater concentrations for the 16 model runs for selected integrated flow groundwater discharge areas are shown on Figure 6.61.

The bar charts on Figure 6.61 show results for discharge locations that were most impacted by  $\text{CCl}_4$ . Simulated concentrations at these locations ranged from zero (not detected) to approximately 1.0 mg/L along the unnamed tributary west of Building 771. Model results also showed that no significant concentration of  $\text{CCl}_4$  occurred in the northern discharge area in the PSA 14 model (North Walnut Creek).

Three model sensitivity runs predicted that  $\text{CCl}_4$  above the draft surface water PRG value reached one potential discharge point, located directly west of Building 771 near the unnamed tributary (Figure 6.61). The three model runs that produced the highest concentrations included:

- A combination of low sorption, low porosity, and low dispersivity ;
- Low porosity; and
- Low degradation rates.

With the exception of these three model runs at one discharge point,  $\text{CCl}_4$  was not predicted to impact groundwater above draft surface water PRG levels (0.58 mg/L) at any other location. The daughter products of  $\text{CCl}_4$ , chloroform, and methylene chloride, have draft surface water PRG levels of 20 mg/L and 10 mg/L, respectively, which were not exceeded in any sensitivity simulation.

Attenuation was affected by the shallow groundwater gradient and slower groundwater travel times. These conditions allowed more time for the parent

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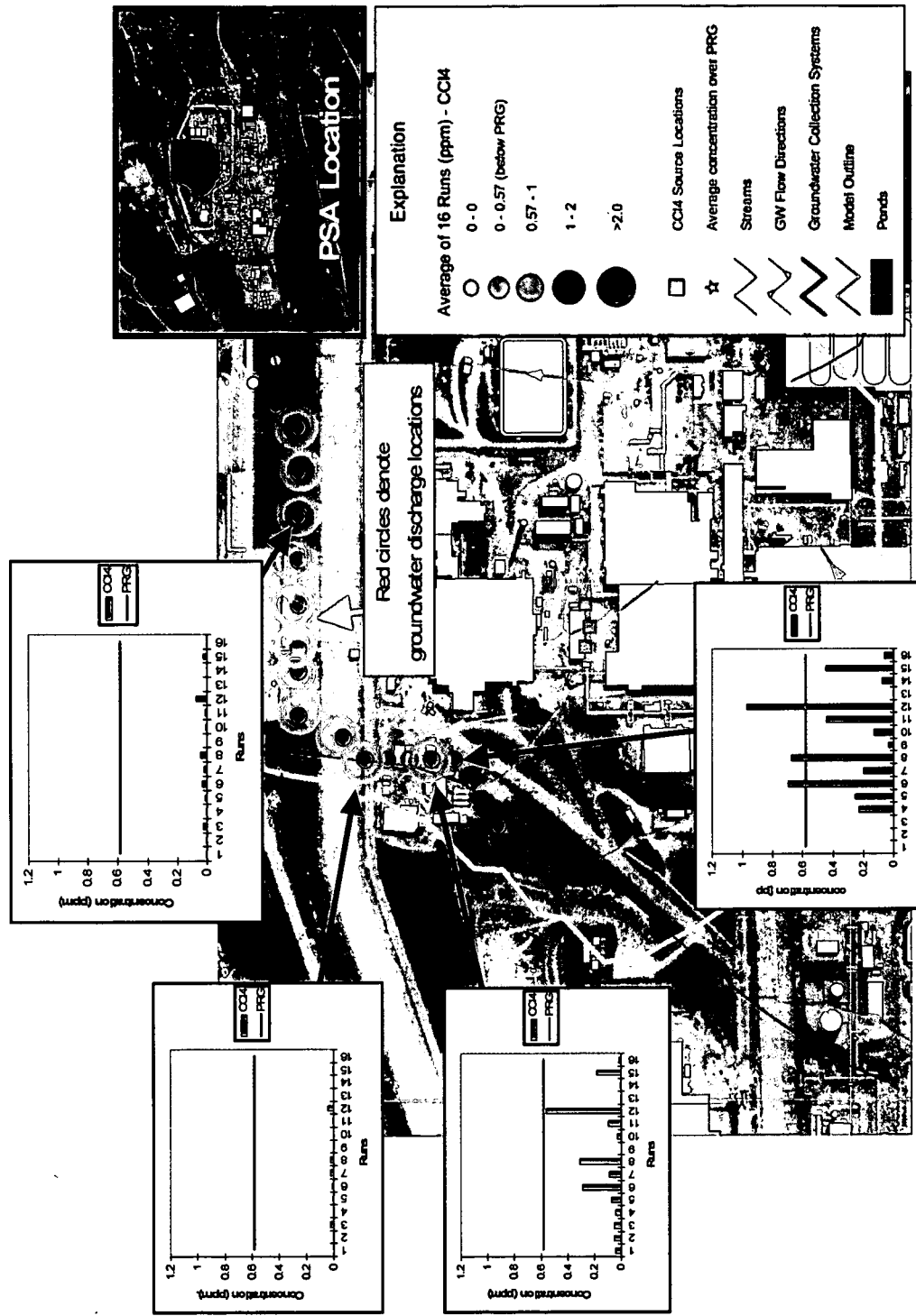


Figure 6.61. PSA 14 - Simulated CCl<sub>4</sub> groundwater concentrations at potential discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.



compound to degrade to daughter products (chloroform and methylene chloride). Additionally, a body of Arapahoe Sandstone within the lower weathered bedrock layers (shown on Figure 2.3 and Figure 2.4) allowed for groundwater to flow through the lower weathered bedrock layers, where it moved very slowly due to the discontinuous nature of the sandstone.

Mass flux from the transport model indicated that the simulated dominant loss mechanisms for VOCs were biodegradation and ET (Figure 6.62).

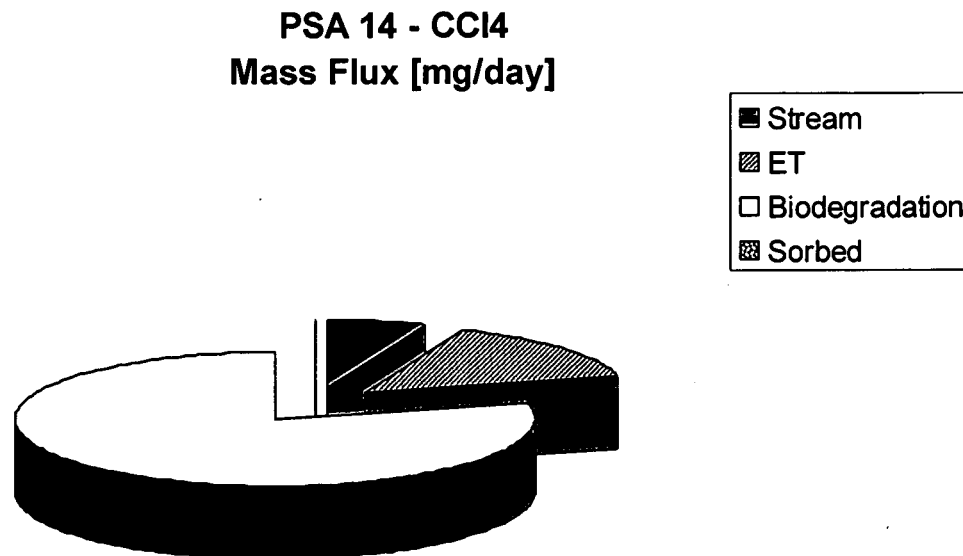


Figure 6.62. PSA 14 - Steady-state mass flux for CCl<sub>4</sub>.

#### 6.2.1.9 PSA 15 (Former Western Solar Ponds Area)

The PSA 15 model area is located within the "former Solar Ponds" area, and is defined on Figures 6.1 and 6.63.

##### 6.2.1.9.1 Groundwater Flow Model Results

Constant head boundaries for PSA 15 were placed at the western and southern up-gradient boundaries where groundwater flow enters the model, as well as the northern down-gradient boundary where flow exits the model.

Simulated water balance results indicated groundwater discharge occurred mainly as ET and to a lesser extent baseflow, near North Walnut Creek. It also

discharged to local drains and the Solar Ponds Groundwater Collection System. Simulated hydraulic heads compared well with observed groundwater well water levels; residuals ranged from 1 to 2 meters. (as shown on Figure 6.64).

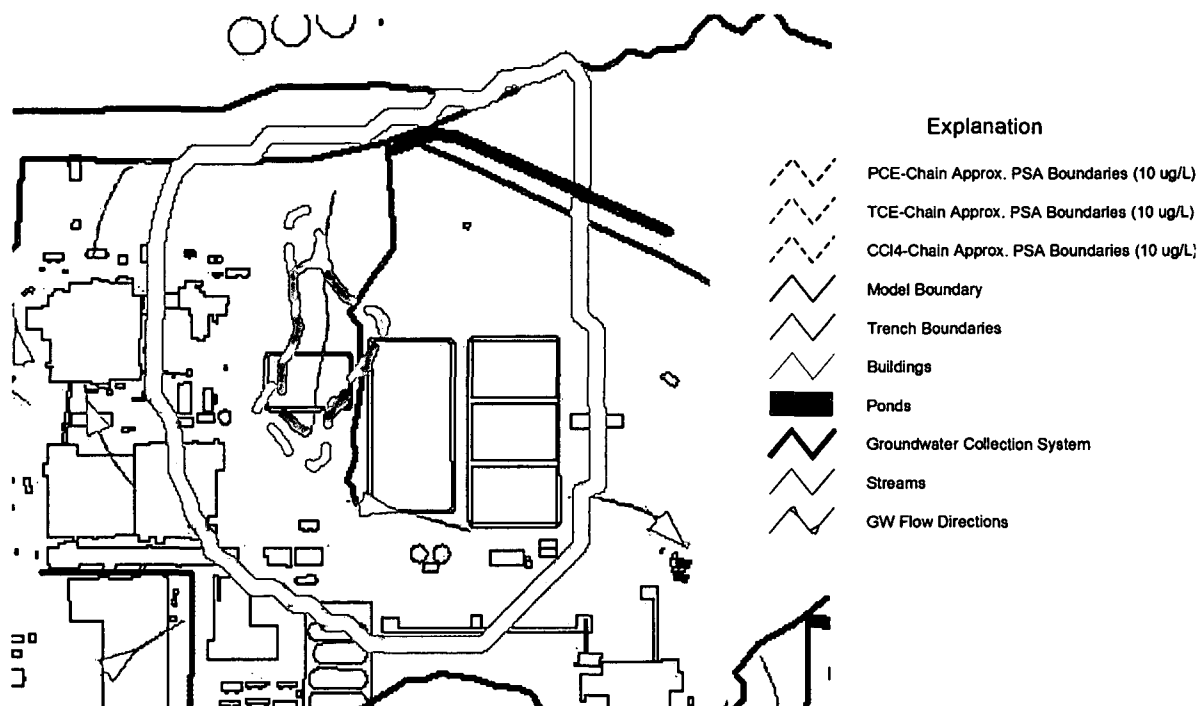


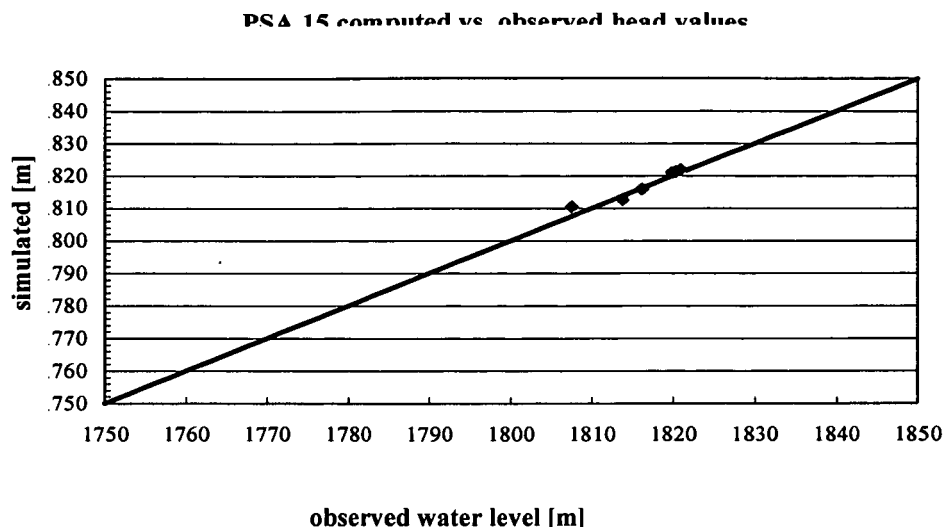
Figure 6.63. PSA 15 - Model Area and PSA.

#### 6.2.1.9.2 Simulated Groundwater VOC Sources

No sources were identified in the HRR information (summarized in Appendix A) for PSA 15. Particle tracking suggested that a source near the former western Solar Ponds area could account for observed well concentrations in the area. As a result, particles were introduced in this area and allowed to travel for 50 years (Figure 6.65). Particles were entered into the upper weathered bedrock because the overlying unconsolidated material is thin and generally unsaturated.

The PSA 15 model area is located within the "former Solar Ponds" area. PSA 15 does not include any priority sources listed in the HRR database. However, particle tracking results suggest that the source of down-gradient PCE/TCE-chain and CCl<sub>4</sub>-chain concentrations is likely associated with the (western) former Solar Ponds.

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**Figure 6.64. PSA 15 - Simulated versus average observed annual groundwater levels (meters above msl).**

Both TCE and the  $\text{CCl}_4$ -chains were observed at concentrations above the draft PRGs and simulated for PSA 15 using the same potential VOC source. PCE was not observed above the draft PRG value. Estimated PSA boundaries for each chain are shown on Figures B-1 through B-7 (Appendix B). The assumed source was represented by two modeled source cells.

#### **6.2.1.9.3 Particle Tracking Results – Historical Conditions**

Results shown on Figure 6.65 suggest that the assumed location, depth, and release timing were reasonable. Particles traveled in a north-northeast direction and were intercepted by the Solar Ponds Groundwater Collection System in the model simulations. The assumed PSA 15 extent is reasonable with these assumptions.

#### **6.2.1.9.4 Transport Model Results – Historical Conditions**

The simulated closure configuration in the integrated flow model indicated very little change to the groundwater flow paths in the PSA 15 area. This is partly because few changes were proposed in the inferred VOC source area near the western former Solar Ponds area, and no changes were proposed in the hillslope area south of North Walnut Creek.

TCE was detected above the draft surface water PRG level (0.19 mg/L) within the PSA 15 area, PCE was detected at a maximum average historical concentration of 0.008 mg/L (below the draft surface water PRG level of 1.4 mg/L), and  $\text{CCl}_4$  was detected above the draft surface water PRG levels (0.58 mg/L). Estimated PSA boundaries for each chain are shown on Figures B-1

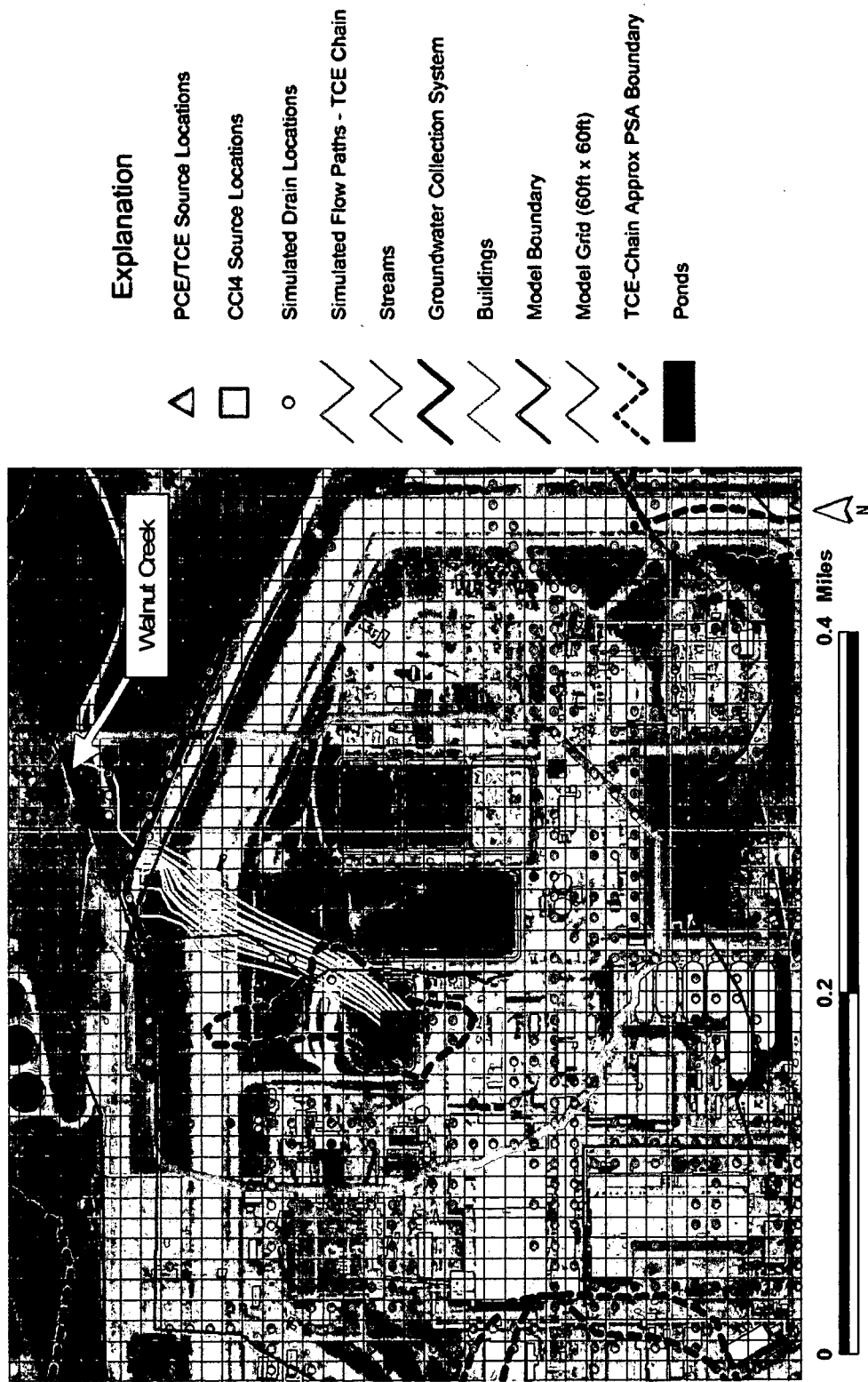


Figure 6.65. PSA 15 - Flow paths of particles introduced into the upper weathered bedrock. Particles were allowed to travel for 50 years.

through B-7 (Appendix B). The assumed source was represented by two modeled source cells.

Plots showing the log residual concentrations for TCE (i.e., log of observed minus simulated concentrations) are shown on Figures 6.66 and 6.67. Sensitivity runs indicated that the highest down-gradient concentrations (Figure 6.66) were produced with low degradation values and the lowest down-gradient concentrations (Figure 6.67) were produced with high degradation rates. Down-gradient (northern) concentrations were very low. Model sensitivities indicated that contaminants did not travel far, so these points were relatively insensitive to changing parameter values. Emphasis was placed on reproducing down-gradient wells, as near-source well concentrations may not reflect well-mixed groundwater concentrations.

Overall, model simulations indicated that VOC concentrations were removed from the groundwater due to the following processes:

- As groundwater moves away from the VOC source, flow is impeded when it reaches the extent of the Arapahoe Sandstone to the north, where it encounters the relatively lower-conductivity weathered bedrock. Slower groundwater flow rates resulted in longer residence times and increased potential for degradation. The reduction of flow rates may be due to the thinning of sandstone in the direction of flow; and
- Increasing ET effects as groundwater nears streams causing lower concentrations at stream areas.

PSA 15 model sensitivity simulations bracketed the range of time-averaged TCE parent and daughter concentrations. The observed concentration distribution of daughter products (cis-1,2-DCE and VC for the PCE degradation chain; chloroform and methylene chloride for the CCl<sub>4</sub> degradation chain) was bracketed by the various sensitivity runs, with high concentrations (producing high daughter products) and low degradation (producing low daughter products) cases for the PCE degradation chain. The results are shown on Figures D-15 and D-16 in Appendix D.

#### **6.2.1.9.5 Transport Model Results – Closure Configuration**

Groundwater flow paths determined for present conditions in the PSA 15 model area trend to the north-northeast (towards North Walnut Creek). Groundwater flows in the lower weathered bedrock layers. Changes to the groundwater system due to the proposed closure configuration did not significantly impact the PSA 15 area.

Future transport simulations indicated that the time it takes for TCE to reach steady-state concentrations was approximately 85 years. A total of 16 transport model runs were simulated. All runs were simulated for at least 150 years to

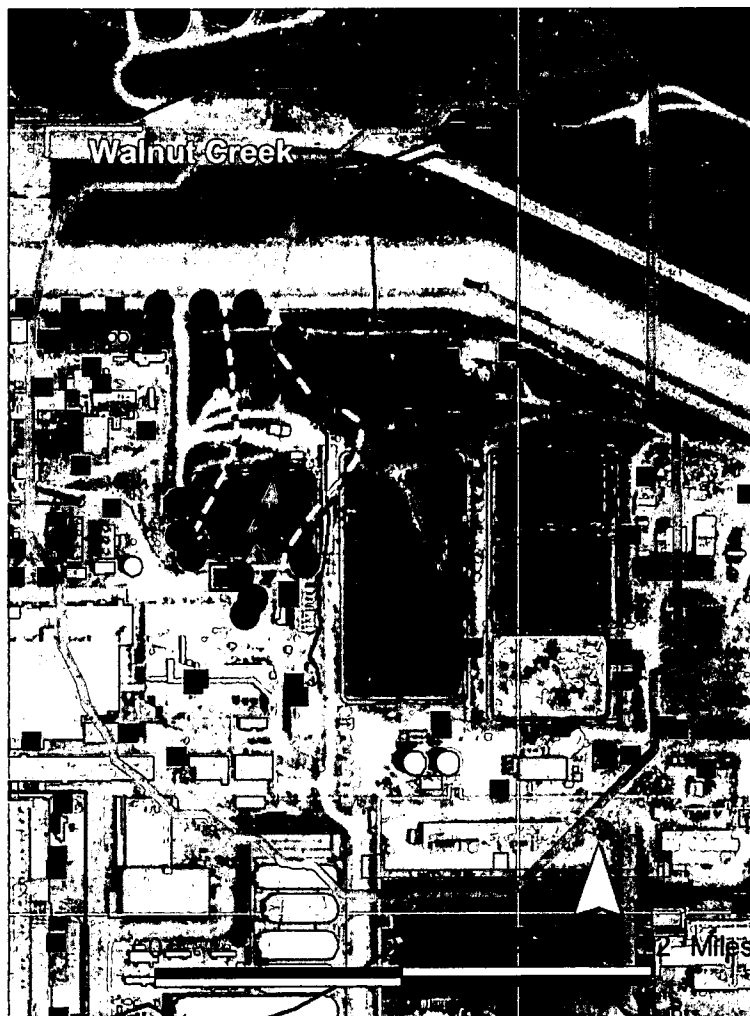
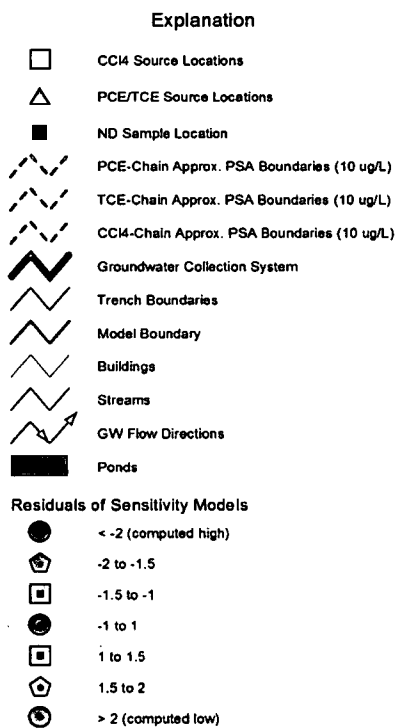


Figure 6.66. PSA 15 – Log residual concentrations. High predicted concentration (TCE) sensitivity run (low degradation).

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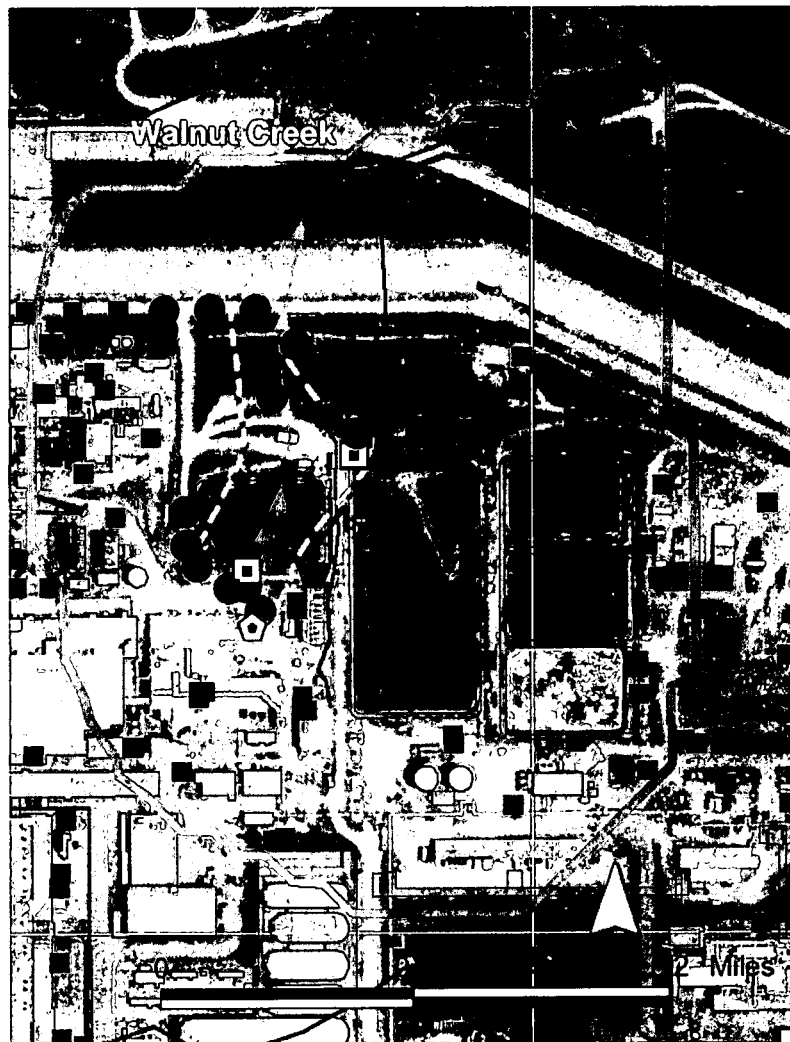
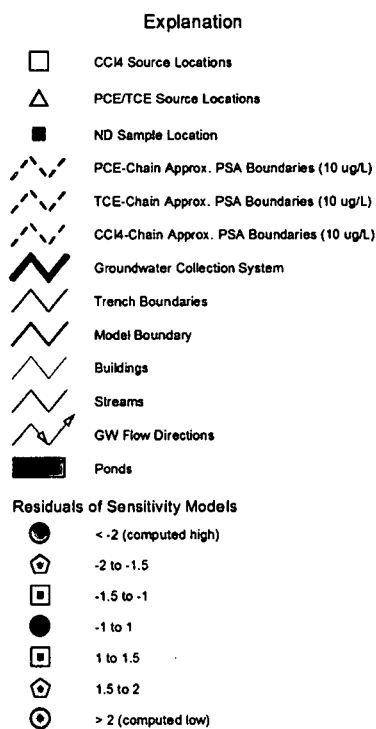


Figure 6.67. PSA 15 – Log residual concentrations. Low predicted concentration (TCE) sensitivity run (high degradation).

establish steady long-term concentrations with time at the PSA extent along North Walnut Creek to the north. Simulated groundwater concentrations for the 16 model runs for each integrated flow model-predicted discharge location are shown on Figure 6.68.

Future transport simulations indicated that the time it takes for  $\text{CCl}_4$  to reach steady-state concentrations was approximately 40 years. A total of 16 transport model runs were produced. All model runs were evaluated for a minimum of 100 years in order to estimate the maximum distance that the plume would travel. Transport flow model-simulated groundwater concentrations for the 16 model runs for selected integrated flow groundwater discharge areas are shown on Figure 6.69.

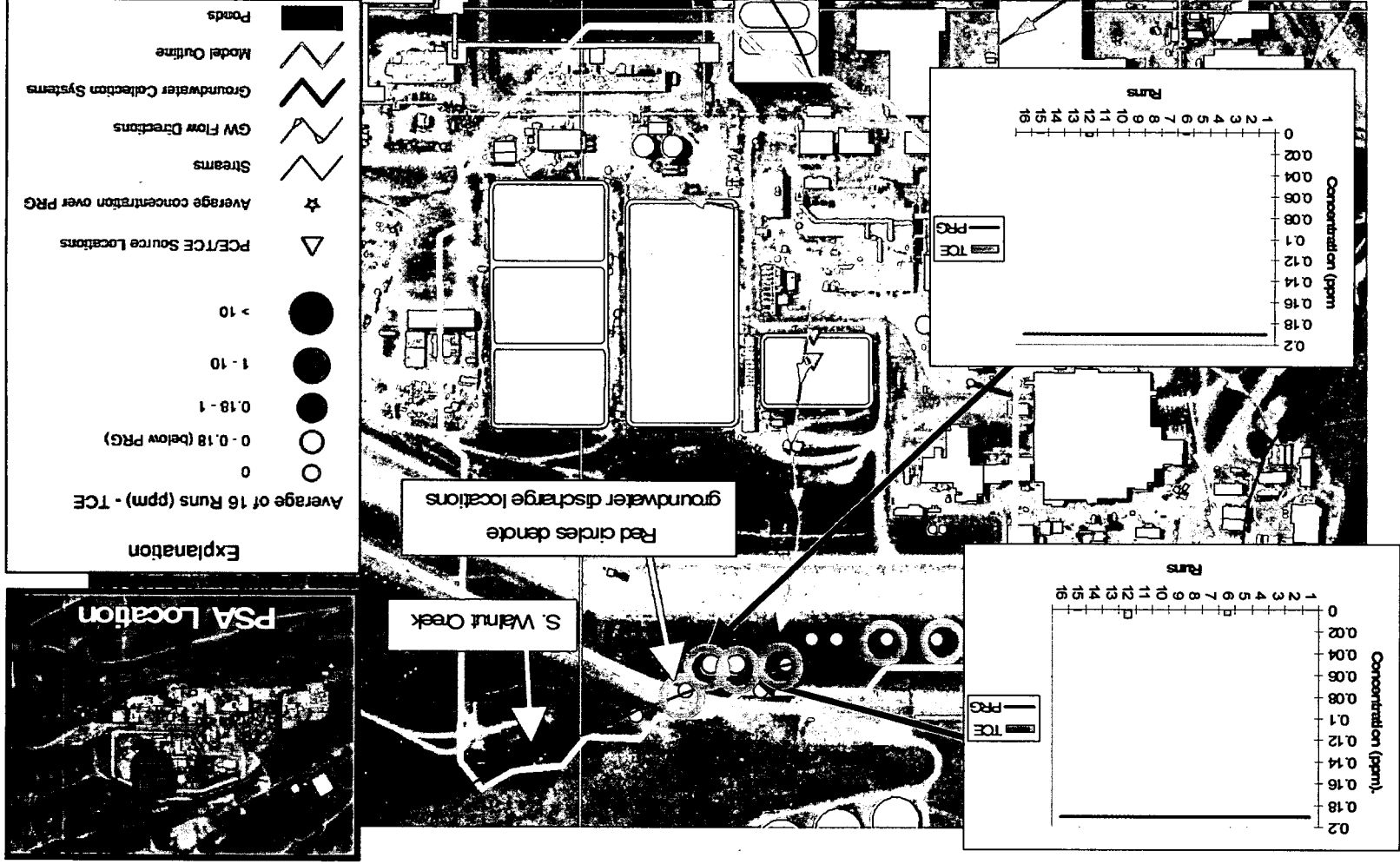
The bar charts on Figure 6.68 show results for discharge locations that were most impacted by TCE. Concentrations at these locations ranged from non-detect (zero) to approximately 0.01 mg/L down-gradient of the inferred VOC source. None of the long-term simulated sensitivity run concentrations at groundwater discharge locations were above draft surface water PRG levels.

The bar charts on Figure 6.69 show results for discharge locations that have the most potential to be impacted by  $\text{CCl}_4$ . Concentrations at these locations ranged from non-detect (zero) to approximately 0.3 mg/L down-gradient of the inferred VOC source. Simulated long-term groundwater discharge concentrations of  $\text{CCl}_4$  were also below draft surface water PRG levels.

Mass flux from the transport model indicated that the simulated dominant loss mechanism for VOCs was biodegradation (as shown on Figures 6.70 and 6.71).



Figure 6.68. PSA 15 - Simulated TCE groundwater concentrations at potential discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.



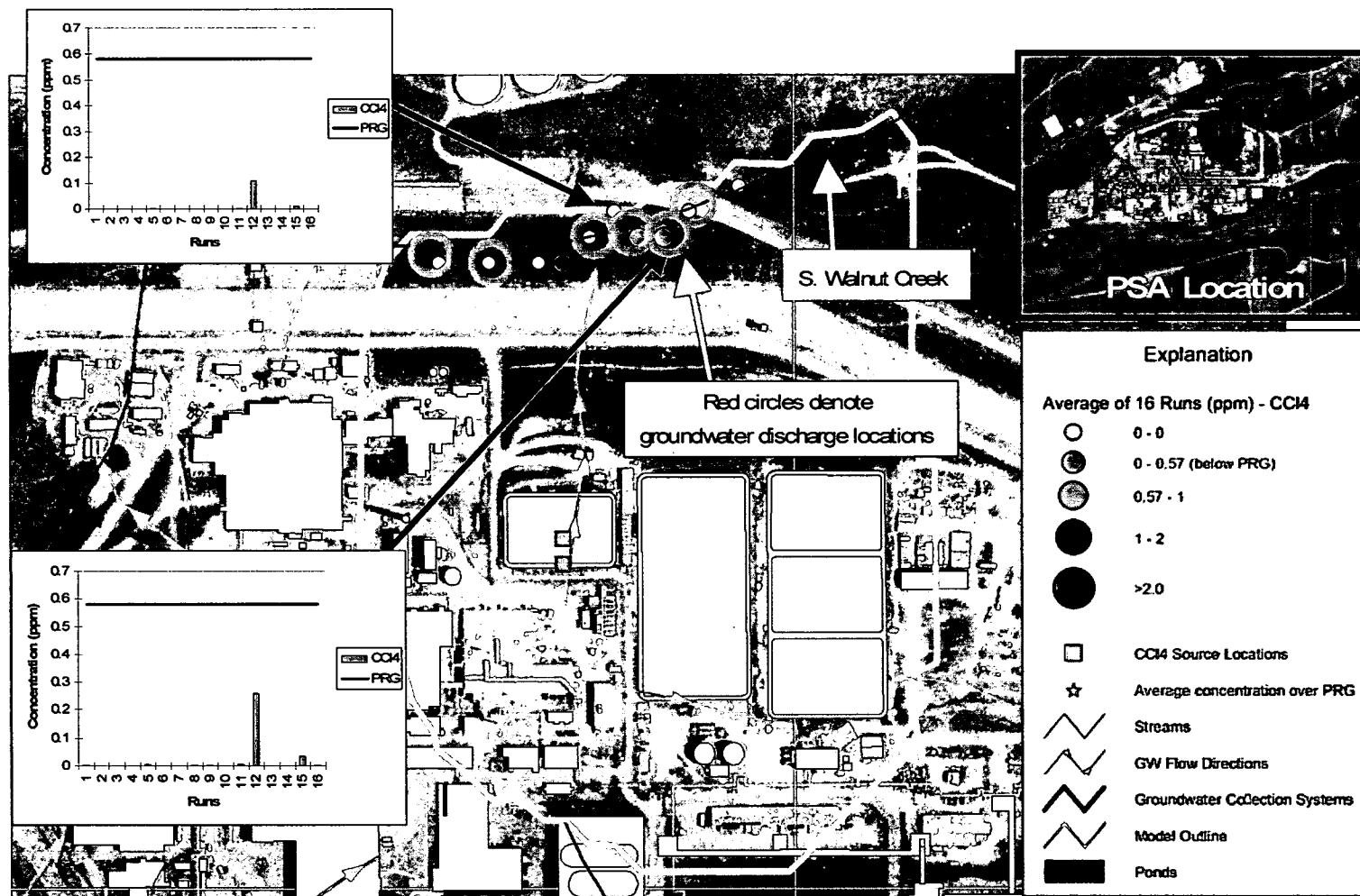


Figure 6.69. PSA 15 - Simulated CCl<sub>4</sub> groundwater concentrations at potential discharge locations. Run 1 used the parameters that best reproduced the time-averaged concentration distribution.

**PSA 15 - TCE**  
**Mass Flux [mg/day]**

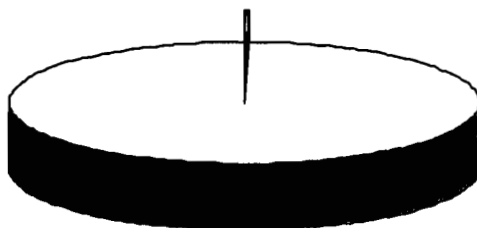
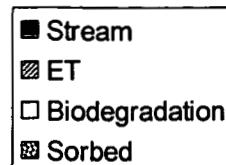


Figure 6.70. PSA 15. Steady-state mass flux for TCE.

**PSA 15 - CCl<sub>4</sub>**  
**Mass Flux [mg/day]**

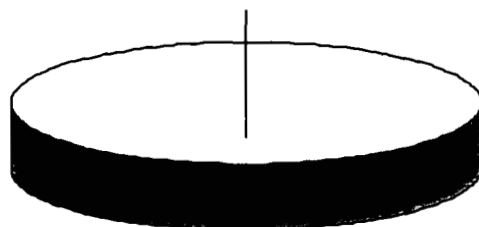
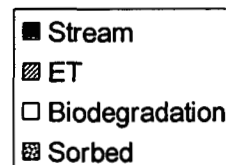


Figure 6.71. PSA 15. Steady-state mass flux for CCl<sub>4</sub>.

## **6.2.2 Transport Modeling – General Findings**

### **General Findings - Degradation Rates**

In general, RT3D simulations indicated that degradation rates at the Site were low relative to the published ranges (Table 6.1). This was reasonable considering that the limited amount of DO data indicated that groundwater over most of the Site was aerobic. The highly chlorinated contaminants (PCE, TCE, and  $\text{CCl}_4$ ) were more likely to degrade under anaerobic conditions by reductive de-halogenation whereas degradation of the lightly chlorinated contaminants (VC) was more likely in aerobic conditions by bio-oxidation (Mercer, et. al., 1998).

The presence of VOC degradation at the Site was likely, based on two main items of evidence. First, nearly all PSAs contain every major daughter product of the degradation chains. Second, cis-1,2-DCE, the primary isomer produced when TCE biodegrades (Mercer, 1998), was present in every PSA being evaluated at a high ratio relative to trans-1,2 DCE. A direct release of 1,2-DCE would normally contain similar amounts of the cis-and trans-isomers.

The degradation rates vary between individual PSAs, but were within a similar range (shown in Table 6.4). Higher rates occur in areas where VOCs were present with petroleum products, which provide a readily available carbon source. Near discharge areas where the groundwater table was shallow, anaerobic degradation can increase by two to three orders of magnitude (Clement, 2003). Along hill slopes, aerobic degradation was probably higher due to the lower water table and lack of paved areas. Unfortunately, there was not adequate spatial information about water chemistry or VOC time-series concentrations define spatial degradation zones within each PSA model.

### **General Findings – Sorption**

The refined range of sorption values (shown in Table 6.5) was generally at the lower end of the range calculated using Site-specific soil parameters. The low sorption values could be compensating for another parameter that affects transport similarly, such as porosity, roughly estimated from 0.03 to 0.3 for all layers and simulations. Increasing the sorption constant has the same effect as increasing porosity, slowing transport, except that the magnitude of the effect was chemical dependent for sorption.

**Table 6.4. Range of modeled degradation rates from all PSA models [1/day] (a degradation rate of 0.0019/day is equivalent to a half-life of one year).**

Chemical	low	high
PCE	0.0001	0.01
TCE	0.00005	0.005
Cis-1,2-DCE	0.00005	0.01
VC	0.01	0.2
CCl <sub>4</sub>	0.0001	0.0006
Chloroform	0.00005	0.002
Methylene chloride	0.00001	0.2

**Table 6.5. Range of modeled sorption constants [L/mg].**

Chemical	low	high
PCE	$0.8 \times 10^{-7}$	$1.5 \times 10^{-6}$
TCE	$2.5 \times 10^{-8}$	$5.0 \times 10^{-6}$
Cis-1,2-DCE	$1.3 \times 10^{-8}$	$2.6 \times 10^{-6}$
VC	$8.0 \times 10^{-10}$	$8.0 \times 10^{-6}$
CCl <sub>4</sub>	$0.9 \times 10^{-7}$	$1.8 \times 10^{-6}$
Chloroform	$0.9 \times 10^{-8}$	$1.9 \times 10^{-6}$
Methylene chloride	$1.4 \times 10^{-9}$	$2.8 \times 10^{-6}$

### **General Findings - Dispersion**

In general, dispersivity values between 5 and 20 meters reproduced the observed PSA concentration distributions. This was consistent with contaminant travel distances of 350 meters or less for most of the PSAs. For example, PSA 5, with a travel distance of less than 200 meters, had a modeled dispersivity of 10 meters whereas PSA 2S, with a traveled distance of about 350 meters, had a dispersivity of 15 meters.

### 6.2.2.1 Transport Modeling – Closure Configuration

Transport modeling of the closure configuration simulated the movement of known contaminants for the proposed closure configuration. A total of 16 transport simulations were run for PSA 15. Simulations were run until steady contaminant concentrations were obtained at groundwater discharge areas.

Table 6.6 summarizes transport simulations and results for the proposed closure configuration. Simulated groundwater TCE concentrations at discharge locations in PSA 2N, PSA 2S, and PSA 5 model areas were higher than draft PRGs at groundwater discharge areas (shown in bold).

Table 6.6. Summary of closure configuration transport simulations.

PSA	PCE/TCE chain	CCl <sub>4</sub> chain
<b>PSA 2N (6/7)</b>	PCE, TCE	CCl <sub>4</sub>
<b>PSA 2S</b>	PCE, TCE	CCl <sub>4</sub>
<b>PSA 5</b>	PCE, TCE	Below draft surface water PRG
PSA 9	TCE	CCl <sub>4</sub>
PSA 10	TCE	Below draft surface water PRG
PSA 12	TCE	CCl <sub>4</sub>
PSA 14	Below draft surface water PRG	CCl <sub>4</sub>
PSA 15	TCE	CCl <sub>4</sub>

Note: (PSAs with the average concentration of the sensitivity simulations above the draft surface water PRG level at one or more potential discharge cells are shown in bold.)

## 7.0 SUMMARY AND CONCLUSIONS

This section summarizes the key steps, results, and conclusions of this study. An approach was developed in this study to construct hydrologic flow and transport models to predict long-term VOC groundwater concentrations at groundwater discharge areas. Major results and key conclusions are described for each modeling step. Only VOCs detected in the IA and the area immediately east (East Trench area, 903 Pad area, and the Mound Groundwater Collection System area) were considered in this report. VOCs detected in the PU&D yard area will be evaluated under separate cover.

### Data Analysis

Available geologic, hydrologic, and VOC information were collected and synthesized into a digital database and GIS so that it could be efficiently evaluated for data quantity and quality. The digital database/GIS was also used extensively to develop spatial and temporal interpretations of a substantial database of containing this information. Although much of the geologic and hydrologic data had already been compiled and evaluated in the SWWB modeling (Kaiser-Hill, 2002), they were combined with all historical VOC and geochemical information to identify likely VOC groundwater sources, to define individual plume areas, and to associate the two.

The concept of PSAs was introduced in this evaluation. A PSA refers to distinct subsurface areas within which all sample locations with detectable groundwater VOCs are assumed to be associated with a single, or multiple, indistinguishable groundwater VOC sources (given groundwater flow paths and historical release information). Time-averaged historical VOC sample concentration data provided more data locations and allowed maximum PSA extents to be established with greater spatial constraint. The concept of PSAs was introduced, rather than specifying "plumes", to emphasize the degree of uncertainty involved in uniquely associating well concentrations with sources. Developing multiple PSAs for PCE and CCl<sub>4</sub> compounds and their degradation chains provided valuable information on their fate and transport in groundwater.

Results of detailed analysis of hydrogeologic data and flow conditions, combined with historical release information and VOC groundwater concentration data, resulted in definition of 19 distinct PSAs. Results also indicated that most contaminant sources correlated with known historical contaminant releases. Most historical releases did not result in groundwater VOC sources. Data evaluation also showed that observed VOC concentrations were typically highest near inferred VOC source locations.

Further data analysis suggested that most PSAs probably have already intercepted groundwater discharge areas. Moreover, relatively steady VOC concentrations in time observed at most sample locations suggested that PSAs have probably reached stable configurations, though some areas may still be

developing (i.e., 903 Pad area). The steady well concentration trends also suggested that VOC sources have probably reached steady concentrations in time. Non-aqueous phase liquid (NAPLs) VOC sources typically produce long-term, steady, dissolved-phase concentrations in groundwater.

Primary VOCs considered in this study were PCE and  $\text{CCl}_4$ , and their daughter products. Successive daughter products of PCE include TCE, cis-1,2-DCE, and then VC. Successive daughter products of  $\text{CCl}_4$  include  $\text{CCl}_3$  and then  $\text{CCl}_2$ . The occurrence of both parent and daughter product VOCs within most PSAs suggested that biodegradation occurs at RFETS, though an independent study suggested rates are variable but low throughout the model area. Additional evidence suggested that TCE occurred as a source in several areas, though it is probably also a degradation product of PCE.

Draft CRA surface water PRGs, which define the relative risks associated with each VOC, were used as the basis for determining whether individual PSAs would be modeled. The total number of PSAs modeled was reduced to nine for PCE, 10 for TCE, and seven for  $\text{CCl}_4$ .

Important PSA and source characteristics, combined with groundwater flow characteristics, were incorporated into a conceptual groundwater flow and fate and transport model. The conceptual model provided the basis for development of subsequent flow and transport models. In the conceptual model, the fate and transport of VOCs in groundwater at RFETS was described in detail from source areas to discharge areas. Results of flow and transport modeling were used to update and refine the conceptual model.

Modeling showed that three-dimensional groundwater flow is important in supporting a detailed flow and transport model for the Site. Groundwater in upper, flatter mesa areas is generally flows downward, but flows upward near the bottom of hillslopes, or streams (hillslope hydrology). This is an important feature of the conceptual model because slower flow rates from source allow more efficient degradation, mostly within bedrock, before it eventually emerges at stream areas. Because ET dominates near-stream hydraulics, model results indicated increasing amounts of VOCs are lost via ET closer to streams. This is significant because this loss to ET, during warmer months, helps attenuate the VOCs before they have the ability to discharge as baseflow to streams, seeps, ponds, or overland flow.

### **Current Conditions Simulations**

Current integrated flow conditions in the IA were first simulated using a higher resolution numerical grid than the original SWWB modeling (Kaiser-Hill, 2002). The MIKE SHE code was used to simulate the integrated flow conditions. Performance of the model was good, if not better, than the SWWB model because local-scale features were refined using the higher resolution grid.



Simulated groundwater levels, drain discharges, and stream flows compared well against observed data.

Next, an appropriate groundwater flow and transport code capable of simulating the complex three-dimensional groundwater flows and VOC fate and transport was selected based on specific selection criteria and modeling objectives. The GMS modeling software, developed by the DOD, was selected to simulate groundwater flow and VOC fate and transport at RFETS. Within this code, groundwater flows were modeled with the USGS MODFLOW, and the VOC fate and transport were modeled using the RT3D reactive transport code developed by DOE. Sorption, biodegradation, molecular diffusion, dispersion, and ET loss were simulated by RT3D.

Eight sub-scale MODFLOW groundwater flow models were developed for different sub areas of the Site. Some of these model areas include more than one PSA. Groundwater flow pathway analysis was an important step in identifying groundwater VOC source locations, the number of source locations, and likely source release times. The USGS code MODPATH (particle tracking software) was used in conjunction with the MODFLOW simulations to iteratively evaluate various source assumptions.

Results of the iterative groundwater flow path analysis for eight different model areas, that include at least one PSA each, confirmed initial assumptions about possible VOC source locations, the number of sources, timing of sources, and groundwater pathways and travel velocities. Results showed that it is reasonable to assume sources were introduced into groundwater about 30 to 50 years ago. HRR information supports this conclusion. Flow path analysis resulted in 22 different source areas that could explain concentration distributions in the 19 separate areas.

RT3D reactive transport modeling was conducted to evaluate the fate and transport of VOCs from time of assumed source release to current conditions. A sensitivity analysis was used to determine several things. First, the parameters which most sensitively control the fate and transport of VOCs were identified. Second, realistic ranges of these parameter values were obtained that reproduce time-averaged concentrations. This range effectively represents uncertainty in these model input parameters. Lastly, RT3D sensitivity simulations of current conditions were used to bound the range of effective source concentrations which reproduced time-averaged concentrations. Sensitivity simulations were also used to confirm assumed source depths and locations defined earlier in the MODPATH groundwater flow path analysis.

Several conclusions can be made from the reactive transport sensitivity analysis conducted for each PSA. First, many factors affected the fate and transport of VOCs from source release time to present. Results showed that the hydraulic conductivity, depth where source (mass flux) was introduced, and biodegradation rates influenced fate and transport the most (compared to other factors, such as

sorption, dispersion, source concentration, diffusion, and porosity). The range of effective source concentrations and source depths, determined through this modeling, reproduced the range of observed time-averaged concentrations within each PSA for both parent and daughter VOCs.

### **Closure Configuration Simulations**

The integrated VOC flow model input was adjusted to simulate the proposed closure land reconfiguration. The proposed closure configuration simulated in this evaluation is different than specified in the SWWB modeling (Kaiser-Hill, 2002) and only included the modeled IA area and areas east. The primary purpose for simulating the the proposed closure configuration with MIKE SHE was to use annual average groundwater levels as initial conditions throughout the GMS MODFLOW model domain and as boundary conditions along the model perimeter. It was also to identify groundwater discharge areas within the integrated VOC flow model boundary.

Discharge frequency and flow rates were calculated by the model only for internal streams. Discharge to streams along model boundaries along North Walnut and Woman Creek, and South Walnut in the B-pond area were not simulated. As a result, results of the original SWWB model were used to assess discharge locations, rates, and frequency.

Simulated closure configuration groundwater flow velocities remain largely unchanged from current configuration because gradients did not change dramatically and are controlled more by the hillslope morphology (surface and bedrock topographies). The largest flow direction changes occurred near buildings with deep foundations where footing drains were deactivated (371, 771, 881, and 991), and along South Walnut Creek east of 991, and where the channel has been re-engineered to eliminate roadways, fenced areas, and associated culverts. Local flow directions near the current Mound Groundwater Collection System changed notably due to this proposed reconfiguration.

Simulated closure scenario groundwater levels increased throughout the model area due to changes in the proposed closure configuration. In some seep areas, groundwater discharged to three of four modified IA streams (drainage between Buildings 371 and 771, the drainage along South Walnut south of Building 991, and in the drainage west of Building 371). Average conditions indicated the discharge occurred only in the drainage between Buildings 371 and 771.

Each PSA steady-state MODFLOW groundwater flow model was updated with the proposed IA configuration modifications and MIKE SHE output. MODPATH was used to estimate simulation times necessary to reach steady travel times. Results showed this was between decades to hundreds of years and accounts for both flow regime adjustments and subsequent VOC transport. In addition, areas where groundwater discharged either to the ground surface (i.e., overland flow) or directly to streams (i.e., baseflow) were computed by the integrated

MIKE SHE flow model for the closure configuration. They were specified for each PSA model area so that the potential for VOC-impacted water at discharge locations could be assessed.

A total of 16 reactive transport model simulations produced a range of possible long-term concentrations at groundwater discharge areas that reflect the approximate range of uncertainty in key input parameter values which strongly control VOC concentrations. Input model parameters were adjusted over ranges that reproduced the range of observed historical time-averaged VOC concentrations. In other words, some parameter values specified in the 16 simulations probably conservatively over-predict long-term groundwater concentrations at discharge locations, while others may under-predict them. Average concentrations at groundwater discharge areas from all 16 model runs were compared against proposed draft surface water PRG values.

Several conclusions were made based on results of the reactive transport modeling and are summarized below:

- Assuming constant source concentrations with time (conservative), in four of the eight sub-scale PSA models, at least one of the 16 simulations produce long-term groundwater concentrations for TCE, or  $\text{CCl}_4$  at groundwater discharge areas that were above the draft surface water PRGs. These PSAs include the following:
  - the East Trenches area,
  - the Mound Groundwater Collection System area,
  - the Building 771 area, and
  - the area southeast of the 903 Pad area (Ryan's Pit area);
- Of these areas, only the Building 771 area showed that the average of long-term simulated groundwater concentrations for the 16 runs were below draft PRGs at groundwater discharge areas.
- Integrated flow modeling showed the following in these areas:
  - in the drainage west of Building 771, groundwater discharged to surface water, even under average climate conditions, due to shallow bedrock and Arapahoe Sandstone in the area;
  - in the South Walnut Creek drainage north and down-gradient of the Mound Groundwater Collection System, discharge occurred in some areas for average conditions, but increased during precipitation events;
  - in the area southeast and down-gradient of the Ryan's Pit area, groundwater discharged to the SID and Woman Creek, only during higher precipitation events, and
  - although actual discharge into the B-pond series north and down-gradient of the East Trench VOC source area was not calculated by

the model, pond water was derived by local runoff and groundwater inflows.

In general, only parent compounds  $\text{CCl}_4$  and TCE were above PRGs at groundwater discharge areas. All other daughter products and PCE did not. For the remainder of PSAs not listed above, simulated groundwater VOC concentrations in groundwater discharge areas for all 16 simulations were below draft surface water PRGs. This is due to a combination of the following:

- slower groundwater velocities in bedrock (generally unconsolidated material becomes unsaturated at the top of hillslopes, but then resaturated close to streams);
- the simultaneous combined effect of attenuation processes (such as biodegradation, sorption, and ET loss) reduced concentrations in groundwater discharge to surface areas.

In most PSAs, the dominant attenuation process appeared to be low rates of biodegradation, though ET loss becomes more significant in the eastern PSAs (i.e., southeast of the 903 Pad area, and in the East Trenches area) as VOCs in near-stream areas. Although, parameter values for each closure configuration model reproduced observed time-averaged concentrations, some combinations underestimated concentrations, while others over-predicted concentrations. The latter case likely over predicted long-term closure concentrations at groundwater discharge areas. As such, a single run should not be considered to be an accurate representation of closure concentrations, or even the most reasonable. Rather, the range of predicted output should be used in assessments.

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REFERENCE NUMBER	YEAR	UNIT NAME	AREA	COORDINATES Easting (ft) Northing (ft)	DESCRIPTION	CONTAMINANT CHARACTERISTICS												VOLUME (gallons)	AREA (sq ft)	CONCENTRATION
						PCB	PAH	PCP	PCB	PAH	PCP	PCB	PAH	PCP	PCB	PAH	PCP			
20	1981	SEVEN ENDS OF BUILDING 178 SOLVENT SPILL	700	E 2084000 N 747000	Carbon tetrachloride, toluene, and xylene (see HRS 118.11)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
21	1982	OUTSIDE AREA	800	E 2084100 N 747100	Oil spilled from tank, unknown	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
22	unknown	Chemical Building	800	E 2084150 N 747110	Area known to be contaminated, used to store unknown chemicals	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
23	unknown	Utility Building	800	E 2084300 N 747300	CEAOP Phase I and II areas, used for disposal of unknown liquids and drums	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
24	1973	Building 811 HRRS Oil Leaks	800	E 2083500 N 747500	Oil discovered flowing on floor S. of 811	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
25	1973	8881 HRRS Oil Leaks	800	E 2083500 N 747500	Oil discovered flowing from floor drain of 8881 (used to be the loading area for 8881)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	PCE = 120ppb, TCE = 40ppb (both samples from north end of building)
26	1988	Transformer 881-4	800	E 2084000 N 747000	PCB-contaminated transformer of tank onto concrete pad	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	110 ppm PCBs
27	1984	Trench 1-1	800	E 2084000 N 747000	Drum burial area (approx. 170 drums)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
28	1984	Trench 1-1	800	E 2084000 N 747000	Drum burial area (approx. 170 drums)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
29	1984	Trench 1-2	800	E 2083800 N 746500	Drum burial area	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
30	1989	Room 411 (Trench 1-2)	800	E 2083800 N 746500	Site involved solvent disposal, other chemical wastes, paint thinner	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
31	1989	Trench 1-2 (aka 411)	800	E 2083800 N 746500	Area primarily used for disposal of solvents, paint thinner, diesel fuel, and other construction related materials	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
32	1989	Trench 1-2 (aka 411)	800	E 2083800 N 746500	Trench primarily used for disposal of solvents, paint thinner, diesel fuel and other construction related materials	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
33	1989	803 Pond	800	E 2083800 N 746500	Leaking drums with waste oil	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
34	1984	Mixed Area	800	E 2083800 N 746500	Drum burial in yellow trench	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
35	1984	Mixed Area	800	E 2083800 N 746500	Mounding contaminated combustibles from B444. Drums contained petroleum fuels. Later, drums from B444, B889, B889, B771, B776 placed in mound area. In 1959, burning of oil became a common occurrence and mounding discontinued.	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	PCE before excav = 8000ppb TCE before excav = 8000ppb PCE after excav = 400ppb TCE after excav = 400ppb
36	1984	OUTSIDE AREA	800	E 2083500 N 747500	As a result of separation of primary and secondary piping with OJZ collection system, surface water containing hazardous substances released to environment (soil) from influent pipe system from WYBAC Chemical OJZ treatment system.	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	PCE before excav = 8000ppb TCE before excav = 8000ppb
37	1987	OUTSIDE AREA	800	E 2083500 N 747500	Drum burial area (approx. 170 drums)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
38	1984	803 Pond	800	E 2083800 N 746500	contaminated surface soils attributed to storm and wind dispersion from the 803 Pond Drum Storage Area (see HRRS 118.11)	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
39	1987	OUTSIDE AREA	800	E 2083500 N 747500	In 1957 through 1961 (total of at least 1354 drums) drums of coolant, oil bottoms, and waste oil were burned in an open pit and groundwater and incidental storm events routinely entered the burn pit and could become contaminated.	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
40	1984	Mixed Area	800	E 2083800 N 746500	Drums were mounded at this location between August 1954 and Sept 1958. Waste contained oil and solvents. All drums were removed by May 1970. Source removal efforts involved removal, treatment, and replacement between March and September 1997. In situ treatment system installed on 100 to 150 gallons of diesel fuel leaked from tanker truck to drainage excavation site south of building.	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified
41	1988	Drum Fuel Leaks - Building 450	400	E 2081000 N 749400	100 to 150 gallons of diesel fuel leaked from tanker truck to drainage excavation site south of building.	0	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified

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REFERENCE NUMBER	YEAR	UNIT NAME	AREA	COORDINATES		DESCRIPTION	Contaminant Characteristics												Other	Volume (gallons)	Area (sq ft)	Concentration
				Eastings (E)	Northings (N)		PCB	VOC	TCF	PCB	VOC	TCF	PCB	VOC	TCF	PCB	VOC	TCF				
148	1983	Valve Vault 7	700	E2083700	N749855	Southwest of B707, valve vault 7 controls 800 Area main process waste line.	0	0	0	0	0	0	0	0	1	1	0	nitric acid, hydrochloric acid, uranium, Ba, F	not specified	not specified	not specified	
149	1958	Valve Vault West of Building 707	700	E2083700	N749855	Leaking process waste line flowed into ditch N.E. of present location of 707 (707 not there at the time). Process waste contained solvents, oil, nitric acid.	0	0	0	0	0	0	0	0	1	1	0	0	4050	not specified	54 mg/L NO <sub>3</sub>	
150	1953	Tanks #68, 67 and 68	700	E2084145	N751035	Overflow of tanks with process waste water east of Building 374. Water high in nitrates. (See HSS 125)	0	0	0	0	0	0	0	0	0	0	1	0	500	not specified	not specified	
151	1952	Holding Tank (Tank #68)	700	E2084145	N751035	Process waste spill from tank (see HSS 124, 2)	0	0	0	0	0	0	0	0	0	0	1	0	not specified	not specified	not specified	
152	1953	Out-of-Service Process Waste Tanks	700	E2083715	N751185	Out-of-service process waste tanks. Overflowed several times prior to 1958. High in nitrates.	0	0	0	0	0	0	0	0	0	0	1	0	not specified	not specified	not specified	
153	1956	Radioactive Site - 700 Area - Site Number 4	700	E2083910	N750710	Tanks between B730, B771 and B776 (a.k.a. T-9 and T-10 Tanks)	0	0	0	0	1	0	0	0	1	0	0	laundry waste, VOCs	not specified	not specified	not specified	
154	1955	Process Waste Line Leaks	700	E2083705	N749455	Process waste line leaks near 881, 441, and 444. Nitrates and solvents possibly in process waste.	0	0	0	0	0	0	0	0	1	0	1	0	not specified	not specified	>100 ppm	
155	1972	Effluent Pipe	700	E2084220	N751052	Process waste line leaks and breaks near 774	0	0	0	0	0	0	0	0	0	0	1	0	not specified	not specified	10,000 mg/L nitrate	
156	1957	Radioactive Site North of Building 771	700	E2083883	N751235	Several incidents of leaking drums with potential releases of solvents or nitrates.	0	0	0	0	0	0	0	0	1	0	1	0	not specified	not specified	not specified	
157	1971	Radioactive Site Between Buildings 771 and 774	700	E2083880	N751000	Leaks in concrete tunnel with process waste lines. Flange broken in late 1970s.	0	0	0	0	0	0	0	0	0	0	1	0	not specified	not specified	not specified	
158	1953	Radioactive Site West of Building 707	700	E2083800	N749831	Leaks of process waste from valve vault 7 (west of Bldg 707)	0	0	0	0	0	0	0	0	1	1	1	0	not specified	not specified	not specified	
159	1959	Radioactive Site South of Building 776	700	E2083775	N750350	Oil and gravel placed over soil contaminated with Pu from fire	0	0	0	0	0	0	0	0	0	1	0	0	not specified	not specified	not specified	
160	1986	Solvent Spill	700	E2084075	N749885	Approx. 4 gallon spill of TCA to ground near southeast dock of 707	0	0	1	0	0	0	0	0	1	0	0	0	4	implied small area, but unspecified	not specified	not specified
161	1986	Solvent Spill	700	E2084075	N749885	Forklift punctured 55-gal drum of TCA on southeast dock of B707. Leaked onto loading dock and adjacent pavement.	0	0	1	0	0	0	0	0	1	0	0	0	4	not specified	not specified	not specified
162	1986	750 Pkg Pondcrete and Sulfate storages	700	E2084500	N750100	Periodic spills of pondcrete and sulfate, exposed to elements and impacted runoff.	0	0	0	0	0	0	0	0	0	0	1	0	not specified	not specified	not specified	
163	1958	Oil Spillage Pit	800	E2084189	N747740	30-50 drums nonradioactive materials dumped in pit south of B881. Material consisting of oil sludge from tank cleanouts. No. 8 fuel oil known to be stored.	0	0	0	0	0	0	0	0	0	1	0	No. 8 fuel oil	not specified	25 x 50	not specified	
164	1958	B881 East and West Out-of-Service Fuel Tanks	800	E2083948	N748158	Diesel fuel tanks	0	0	0	0	0	0	0	0	0	0	0	diesel fuel	not specified	not specified	not specified	
165	1952	Outfall	800	E2083937	N748028	1950s and 1960s intermittent discharging of untreated sanitary waste occurred south of B881	0	1	0	1	0	0	0	0	1	0	1	untreated sewage and other waste	not specified	not specified	not specified	
166	1985	Transformer B83-4	800	E2084000	N749000	PCB-contaminated transformer oil leak onto pad	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	>500 ppm PCBs	
167	1985	Transformer B83-4	800	E2084000	N749000	Transformer B83-4 at southeast corner of B883 leaked on concrete pad.	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	Historical records PCBs in oil = 500ppm. In 1992, PCBs in oil = 80ppm. Soil PCBs up to 160ppm.	
168	1987	Transformer B81-4	800	E2084000	N749000	Transformer B81-4 north side of B881 leaked on valves top and pad	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	Oil contained PCBs at 110ppm	
169	1991	Leaking transformers, 800 Area	800	E2084000	N749000	PCB-contaminated transformers may have leaked oil, stains at valves	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	not specified	
170	1991	Leaking Transformers, 800 Area	800	E2084000	N749000	Three transformers on the North side of B883 leaked prior to 1987 retrofitting. Oil stains visible. Area bermed, no drains.	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	In 1985, oils = 11-84ppm. In 1992, oils = 3-35ppm. In 1991, soils = 7ppm	
171	1987	Transformers B85-1 and B85-2	800	E2084000	N749000	PCB-contaminated transformer oil leaked	1	0	0	0	0	0	0	0	0	0	0	0	not specified	not specified	110 ppm PCBs	
172	1988	Capacitor Leak, B83	800	E2084000	N749000	Small oil leak (1 cm)	0	0	0	0	0	0	0	0	0	0	1	0	0	0.125	not specified	not specified
173	1992	B888 Sump	800	E2084400	N749100	Lubricating oil residue in sump pit determined to have originated in waste tanks.	0	0	0	0	0	0	0	0	0	0	1	0	lubricating oil	35	not specified	not specified
174	1964	B881 Conversion Activity Contamination Area	800	E2083705	N749455	Interviews say miscellaneous equipment stored in area such as lathe parts and rolling mill parts.	0	0	0	0	0	0	0	0	0	0	1	0	Cd, Zn	not specified	50 x 150	not specified
175	1953	Building B81 Drum Storage Area, Room 185	800	E2083975	N749500	Drum storage area	0	0	0	0	0	0	0	0	0	0	0	0	chemical materials	not specified	not specified	not specified
176	1970	Building B83 Drum Storage Area, Room 185	800	E2084100	N749500	Drum storage area	0	0	0	0	0	0	0	0	0	0	1	0	0	not specified	not specified	not specified
177	1981	Building B83 Drum Storage Area, Room 184	800	E2083910	N749925	Drum storage area	0	0	0	0	0	0	0	0	0	0	1	0	0	not specified	not specified	not specified

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## HRR Summary Table

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HRR Summary Table

REFERENCE NUMBER	YEAR	LEAK NAME	AREA	LEAKING DOI	COORDINATES	CONTAMINANT CHARACTERISTICS													COMPLETION
						PCB	TCF	THC	CHC	CMC	CHC	CMC	CHC	CMC	CHC	CMC	CHC	CMC	
231	1982	PG-2 Limited Information	unknown	not specified	not specified	17 feet drums with water sand and styrene glycol dumped to ground and released to storm drain	0	0	0	0	0	0	0	0	0	0	0	0	not specified
232	1985	Drain Oil Spill North of Building 555	500	E705250	N750250	A portable air compressor was fipped over releasing 1 gallon of diesel fuel onto road surface. (April)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
233	1986	Drain Oil Spill North of Building 778	700	E708350	N750850	One gallon of diesel oil was spilled in heavy north of 778 (March 19)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
234	1987	Drain Oil Spill - Building 107	700	E708400	N750800	20 gallons of diesel fuel released when fuel line damaged during construction activities (April 6)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
235	1988	Hydraulic Oil Leaks - Point 2	300	E708400	N751000	Hydraulic Oil Leaks - One in November and one in December	0	0	0	0	0	0	0	0	0	0	0	0	not specified
236	1988	Oil Spill - Building 888	800	E708250	N748000	Portable air compressor released hydraulic oil onto concrete (October)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
237	1988	Engine Oil Spill - Building 443	400	E708200	N748100	Release of 15 gallons of engine oil from dead air cooling system generator. Some seems to have leaked through concrete and not to environment (May 25)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
238	1988	Asphalt Thinner Spill	900	E708500	N748200	Five gallons of asphalt thinner spilled east of 804 and (January 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
239	1988	Air Compressor Release - Building 440	400	E708250	N748250	Leaks of hot line of compressor air compressor. Spilled to ground (May 6)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
240	1988	Gassing Soil Spill South of Building 707	700	E708400	N750000	One gallon of gassing soil spilled into soil (September 26)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
241	1988	Coating Spill - Building 728	700	E708250	N750250	5 gallons of radiator coolant leaked onto ground outside Building (November 13)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
242	1988	Hydraulic Fluid Leak - East Access Road	not specified	not specified	not specified	2 to 3 gallons of hydraulic fluid released from backhoe to ground (August 24)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
243	1989	Hydraulic Oil Spill - Building 107	700	E708400	N750000	2.5 gallons of hydraulic oil released from broken line to ground (December 7)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
244	1989	Gassing Soil Spill East Side Building 882	800	E708350	N748800	Unspecified volume of gassing soil released from broken line to ground (January 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
245	1989	Coating Spill - Building 334	300	E708250	N748800	Twenty one gallon of radiator coolant leaked from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
246	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
247	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
248	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
249	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
250	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
251	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
252	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
253	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
254	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
255	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
256	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
257	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
258	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
259	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
260	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
261	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
262	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
263	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
264	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
265	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
266	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
267	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
268	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
269	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
270	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
271	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
272	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
273	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
274	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
275	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
276	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
277	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
278	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
279	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
280	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
281	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
282	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
283	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
284	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
285	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
286	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
287	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
288	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
289	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
290	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
291	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
292	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
293	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
294	1989	Air Compressor Oil Spill - Building 885	800	E708400	N748025	1 quart of air compressor oil was released to the ground west of 885 (August 28)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
295	1989	Transmission Fluid Spill - Building 331	300	E7081950	N7481700	3 gallons of transmission fluid spilled to asphalt and concrete west of 331 (April 4)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
296	1989	Hydraulic Oil Spill - Building 115	100	E7081475	N748075	One gallon of hydraulic oil released from backhoe to soil west of 115 (September 12)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
297	1989	Hydraulic Oil Spill - Building 120	100	E7081475	N748000	One gallon of hydraulic oil released from backhoe to asphalt south of 125 (September 17)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
298	1989	Hydraulic Oil Spill - Building 331	300	E7081950	N748150	One gallon of hydraulic oil released from backhoe to asphalt south of 331 (September 10)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
299	1989	Air Compressor Oil Spill - Building 107	700	E708400	N748150	One gallon of radiator coolant released from broken line to ground (August 22)	0	0	0	0	0	0	0	0	0	0	0	0	not specified
300	1989	Air Compressor Oil Spill - Building 885	800	E708400															



Reference Number	Date	Site Name	Area	C. COORDINATES										Description	Volume (gallons)	Area (sq ft)	Concentration
				East (E)	North (N)	West (W)	South (S)	East (E)	North (N)	West (W)	South (S)	East (E)	North (N)				
1865	1985	Trinch 1.4	NE	E 2081733	N 7108400									Sludge material disposal site, miscellaneous waste disposal including drums, asphalt and construction debris.	not specified	11325	not specified
1866	1975	Trinch 1.4, Box L11	NE	E 2081475	N 7108900									asphalt, concrete, masonry and tank	not specified		not specified
1867	1987	NE Buffer Zone Gas Line Bank	NE	E 2080800	N 7109500									NE Buffer Zone Gas Line Bank	5 MCF	130,000	not specified
1868	1983	East Tank Gas PCB Soil	NE	E 2084500	N 7108300									PCB-contaminated transformer oil tank onto asphalt	not specified		not specified
1869	1981	Gasoline Soil	NE	E 2083300	N 7108500									Gasoline Soil 1980 Gasoline	not specified	1	not specified
1870	1972	Gasoline Soil at Road B-2 Station	NE	E 2081500	N 7108000									18 and 18 and 18 and 18	not specified	18	not specified
1871	1992	Gasoline Soil at Road B-2 Station	NE	E 2081500	N 7108000									18 and 18 and 18 and 18	not specified	18	not specified
1872	1981	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081777	N 7108971									Confined diesel fuel tank. Spillage onto gravel, white bottom asphalt, and asphalt.	not specified	20	not specified
1873	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1874	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1875	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1876	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1877	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1878	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1879	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1880	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1881	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1882	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1883	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1884	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1885	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1886	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1887	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1888	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1889	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1890	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1891	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1892	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1893	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1894	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1895	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1896	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1897	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1898	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1899	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified
1900	1983	Oil 2, Phase 2, Field Investigation Unit	NE	E 2081700	N 7109000									Groundwater spilled when casing inserted into new backhoe monitoring well forced water out of hole and leaked around.	not specified	10	not specified



Reference Number	Year	Unit Name	Area	Coordinates		Contaminant Data												Remarks	Area (sq ft)	Volume (gallons)	Date	Remarks
				North (N)	West (W)	PCB	PAH	TCF	PCB	PAH	TCF	PCB	PAH	TCF	PCB	PAH	TCF					
212	1989	Building 131 Transformer Tank	100	E2085500	N749000			1	0	0	0	0	0	0	0	0	0	not specified, but limited spill	Oil contained 19 ppg PCB			
213	1980	Building 121 Security Kennel	100	E2081400	N749250			1	0	0	0	0	0	0	0	0	0	not specified	not specified			
214	1984	3334 Cargo Container Area	300	E2082325	N749745			0	0	0	0	0	0	0	0	1	1	machinery oils and coolants	not specified			
215	1980	B371 Drum Storage Unit 63	300	E2082235	N750380			0	0	1	0	0	0	0	0	0	0	Isoline	not specified			
216	1992	Archives East North of B373	300	E2082033	N750872			0	0	0	0	0	0	0	0	0	0	archives (50/50 engine oil and water)	not specified			
217	1984	Air Conditioner Compressor Release	400	E2082124	N749887			0	0	0	0	0	0	0	0	1	1	air conditioner oil	not specified			
218	1983	Archives Surface West Building 358	500	E2083500	N750100			0	0	0	0	0	0	0	0	0	0	VOCs	not specified			
219	1987	Spills of Unknown Oil in PAC 600-1001 (SR 418)	800	E2083000	N749000			0	0	0	0	0	0	0	0	0	1	0	oil	not specified		
220	1985	Chemical Storage - South 200	800	E2082025	N749001													2.3 L	not specified			
221	1982	NFRC Acid Tank (AKA Nitric, Muriatic Acid Spills)	600	E2082111	N749888														unknown			
222	1986	Transformer Tank - 774-4	700	E2083500	N750500			1	0	0	0	0	0	0	0	0	0	0	not specified by limited spill	Soil samples showed 4,900 ppm in 1982 (Oil contained between 5 and 500 ppm PCB)		
223	1972	771074 Epsilon Drum Depot (AKA Brownies Pond)	700	E2084087	N751245			1	0	0	0	0	0	0	0	0	0	0	not specified	not specified		
224	1986	Releases During Liquid Transfer Operations from B774	700	E2084189	N751128			0	1	1	0	0	0	0	0	0	0	0	not specified	not specified		
225	1983	Valve Unit 7	700	E2083100	N749855			0	0	0	0	0	0	0	0	1	1	0	not specified	not specified		
226	1983	Reductive Sun West of Building 707	700	E2083800	N749831			0	0	0	0	0	0	0	0	1	1	0	not specified	not specified		
227	1983	B381 Drum Storage Area	800	E2083375	N749570			0	0	0	0	0	0	0	0	0	0	0	VOCs	not specified		
228	1970	Building 665 Drum Storage Area	800	E2084000	N749015			0	0	0	0	0	0	0	0	1	0	0	VOCs, chlorinated solvents	not specified		
229	1970	Building 665 Drum Storage Area	800	E2084000	N749015			0	0	0	0	0	0	0	0	0	0	0	oil, chlorinated solvents	not specified		
230	1981	Building 665 Drum Storage Area	800	E2083310	N749875			0	0	0	0	0	0	0	0	0	1	0	0	not specified		
231	1981	Building 665 Drum Storage Area Unit 28	800	E2083375	N749875			0	0	0	0	0	0	0	0	0	1	0	0	materials containing solvents	not specified	
232	1984	Truck T-1	800	E2083048	N749481			1	0	0	0	0	0	0	0	0	0	0	VOCs and water coolant	not specified		
233	1993	Gasoline Spill Near Southwest Corner of B380	800	E2085075	N750150			0	0	0	0	0	0	0	0	0	0	0	gasoline	not specified		
234	1993	Outside the Southwest Corner of B380	800	E2085075	N750150			0	0	0	0	0	0	0	0	0	0	0	gasoline	not specified		
235	1993	Oil 2 Field Through Unit	800	E2082000	N750000			0	1	0	0	1	1	1	1	1	0	0	chloroform, dichloromethane (DCA)	not specified		
236	1982	Septic Tank East of Building 661	900	E2083338	N750100			0	0	0	0	0	0	0	0	0	0	0	VOCs	not specified		
237	1994	Oil 2 Water Soil	900	E2083348	N750103			0	1	1	0	0	0	0	0	0	0	0	chloroform, VOCs	not specified		

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14528

Spill of organic contaminant (including oil) > 100 gal (

HRR Summary Table

[illegible]

REFERENCE NUMBER	HRR NUMBER (by HRR)	HRR IDENTIFIER	FAC REFERENCE NUMBER	PC REFERENCE NUMBER	OU REFERENCE NUMBER	DURATION	STATUS	STATUS ACTION	REFERENCE DOCUMENT	STATUS ACTION	STATUS ACTION
1	118	118.2	700-118.2	N/A	8	short-term	OK	Truck and area of spill were cleaned up.	HER 1992	5,000 gal carbon tetrachloride tank rupture. Not specified date released. Tank between N side of 707 and S side of 708.	1
2	119	119.2	800-119.2	N/A	1	unknown	OK	Not specified.	HER 1992	50,000 gallons of oil leaked out of a tank. No documentation found detailing time of constituents. SE of the site.	1
3	120	120.2	800-120.2	N/A	1	unknown	OK	Not specified.	HER 1992	No documentation found detailing time of constituents. SE of the site.	1
4	121	121.2	800-121.2	N/A	1	unknown	OK	Not specified.	HER 1992	No documentation found detailing time of constituents. SE of the site.	1
5	122	122.2	800-122.2	N/A	1	unknown	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
6	123	123.2	800-123.2	N/A	1	unknown	OK	Concrete retaining pond and interceptor trench built (1970).	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
7	124	124.2	800-124.2	N/A	1	unknown	OK	Transformer replaced in 1987.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
8	125	125.2	800-125.2	N/A	1	unknown	OK	Transformer replaced in 1989.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
9	126	126.2	800-126.2	N/A	2	8	OK	Remediated in 1989 by removal.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
10	127	127.2	800-127.2	N/A	2	8	OK	Remediated in 1989. Excavation occurred from June 10 to August 20, 1989.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
11	128	128.2	800-128.2	N/A	2	8	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
12	129	129.2	800-129.2	N/A	2	24	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
13	130	130.2	800-130.2	N/A	2	2	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
14	131	131.2	800-131.2	N/A	2	2	OK	Trench backfilled in 1971. ~150 yd <sup>3</sup> excavated in 1985 and treated by low temp. flaming.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
15	132	132.2	800-132.2	N/A	2	3	OK	Trench backfilled in 1987. ~150 yd <sup>3</sup> excavated in 1985 and treated by low temp. flaming.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
16	133	133.2	800-133.2	N/A	2	2	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
17	134	134.2	800-134.2	N/A	2	2	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
18	135	135.2	800-135.2	N/A	2	10	OK	Water remediation continued for Oct. 2002.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
19	136	136.2	800-136.2	N/A	2	4	OK	Drums removed in 1970. Contaminated soil removed March, August 1987.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
20	137	137.2	800-137.2	N/A	2	4	OK	Drum removal completed in 1970. Treated soil and placed back in pits in 1987.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
21	138	138.2	800-138.2	N/A	2	1.5hr	OK	None of the material that wetted soil of flowed into the storm drain.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
22	139	139.2	800-139.2	N/A	2	7	OK	Approx. 240 boxes of soil removed in 1978.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
23	140	140.2	800-140.2	N/A	2	8	OK	Drum removal, grading and construction of up area around the BCI Pit were graded.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
24	141	141.2	800-141.2	N/A	2	8	OK	Approximately 240 boxes of soil were removed in 1978 and shipped offsite for treatment.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
25	142	142.2	800-142.2	N/A	2	8	OK	Drums were removed by May 1970. Sources of contamination were removed.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
26	143	143.2	800-143.2	N/A	2	4	OK	Removal efforts involved removal, treatment, and replacement between March and September 1987. In Situ treatment system installed in 1988 to treat groundwater.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
27	144	144.2	800-144.2	N/A	2	4	OK	Fire alert constructed due to prevent further spread. Soil applied to ensure constituents. No record of removal of soil or further cleanup.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1
28	145	145.2	800-145.2	N/A	2	1	OK	Not specified.	HER 1992	Originally thought to be from two tanks, but being from one tank. After 20 years of leakage, an extent it leaked out of the tank. Another theory is that it originated from oil storage tank 102. VOCs in groundwater. Well to the West of the site.	1

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Reference Number	HRR Number	Site Name	Location	Owner	Operator	Product	Volume	Weight	Concentration	Incident Description	Investigation	Remediation	Product
101	101	000-101	N/A	4	38					1981	1981-1982	1981-1982	2
102	102	000-102	N/A	11	3.5					1981	1981-1982	1981-1982	2
103	103	000-103	N/A	11	3.5					1981	1981-1982	1981-1982	2
104	104	000-104	N/A	8	1 day					1981	1981-1982	1981-1982	2
105	105	000-105	N/A	N/A	40					1981	1981-1982	1981-1982	2
106	106	000-106	N/A	N/A	15					1981	1981-1982	1981-1982	2
107	107	000-107	N/A	N/A	47					1981	1981-1982	1981-1982	2
108	108	000-108	N/A	13	72					1981	1981-1982	1981-1982	2
109	109	000-109	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
110	110	000-110	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
111	111	000-111	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
112	112	000-112	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
113	113	000-113	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
114	114	000-114	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
115	115	000-115	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
116	116	000-116	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
117	117	000-117	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
118	118	000-118	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
119	119	000-119	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
120	120	000-120	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
121	121	000-121	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
122	122	000-122	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
123	123	000-123	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
124	124	000-124	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
125	125	000-125	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
126	126	000-126	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
127	127	000-127	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
128	128	000-128	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
129	129	000-129	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
130	130	000-130	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
131	131	000-131	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
132	132	000-132	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
133	133	000-133	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
134	134	000-134	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
135	135	000-135	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
136	136	000-136	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
137	137	000-137	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
138	138	000-138	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
139	139	000-139	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
140	140	000-140	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
141	141	000-141	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
142	142	000-142	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
143	143	000-143	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
144	144	000-144	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
145	145	000-145	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
146	146	000-146	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
147	147	000-147	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
148	148	000-148	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
149	149	000-149	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
150	150	000-150	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
151	151	000-151	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
152	152	000-152	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
153	153	000-153	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
154	154	000-154	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
155	155	000-155	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
156	156	000-156	N/A	N/A	100-500					1981	1981-1982	1981-1982	2
157	157	000-157	N/A	N/A	100-500					1981	1981-1982	1981-1982	2



HRR Summary Table

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Reference Number	HRR	Unit	Area	Volume	Depth	Material	Activity	Notes	Comments	Priority
123	123.1	700-123.1	N/A	N/A	1.0m	not specified	not specified	Spot release in Pond B-1 and Pond B-2 3 weeks before release to Great Western Res. Visible contamination in North St. ditch. No evidence of release.	HRR Update 1987	2
123	123.2	700-123.2	N/A	9	Short-term event	not specified	not specified	not specified	HRR Update 1987	2
124	124.1, 124.2, 124.3	700-124.1, 700-124.2, 700-124.3	N/A	9	36	Area of soil paved to cover Pu contamination (1981)	Area of soil paved to cover Pu contamination	Area of soil paved to cover Pu contamination	HRR Update 1987	2
125	125	700-125	N/A	9	37	Area of soil paved to cover Pu contamination (1981)	Area of soil paved to cover Pu contamination	Area of soil paved to cover Pu contamination	HRR Update 1987	2
126	126.1, 126.2	700-126.1, 700-126.2	N/A	9	31	not specified	not specified	not specified	HRR Update 1987	2
127	127	700-127	N/A	1A	28	not specified	not specified	not specified	HRR Update 1987	2
128	128	700-128	N/A	9	29	2 bags of soil removed in 1984	2 bags of soil removed in 1984	2 bags of soil removed in 1984	HRR Update 1987	2
129	129	700-129	N/A	9	8	Removal of debris in 1973	Removal of debris in 1973	Removal of debris in 1973	HRR Update 1987	2
130	130	700-130	N/A	8	17	Vertical removal of debris for different analysis	Vertical removal of debris for different analysis	Vertical removal of debris for different analysis	HRR Update 1987	2
131	131	700-131	N/A	8	10	Tunnel sealed. Soil cleanup (mainly for rad contamination)	Tunnel sealed. Soil cleanup (mainly for rad contamination)	Tunnel sealed. Soil cleanup (mainly for rad contamination)	HRR Update 1987	2
132	132	700-132	N/A	8	30	Oil and gravel removed approximately 2 months after fire was spotted	Oil and gravel removed approximately 2 months after fire was spotted	Oil and gravel removed approximately 2 months after fire was spotted	HRR Update 1987	2
133	133	700-133	N/A	8	<1	4 bags of absorbent used to cleanup spill	4 bags of absorbent used to cleanup spill	4 bags of absorbent used to cleanup spill	HRR Update 1987	2
134	134	700-134	N/A	10	1.0m	Material was cleaned up and placed in drums	Material was cleaned up and placed in drums	Material was cleaned up and placed in drums	HRR Update 1987	2
135	135	700-135	N/A	10	1.0m	Commercial equipment cleaned up and placed in drums	Commercial equipment cleaned up and placed in drums	Commercial equipment cleaned up and placed in drums	HRR Update 1987	2
136	136	700-136	N/A	10	1.0m	Equipment cleaned up as a time of rain	Equipment cleaned up as a time of rain	Equipment cleaned up as a time of rain	HRR Update 1987	2
137	137	800-102	N/A	1	1	not specified	not specified	not specified	HRR Update 1987	2
138	138	800-102	N/A	1	10	not specified	not specified	not specified	HRR Update 1987	2
139	139	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
140	140	800-102	N/A	1	4	not specified	not specified	not specified	HRR Update 1987	2
141	141	800-102	N/A	1	3	not specified	not specified	not specified	HRR Update 1987	2
142	142	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
143	143	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
144	144	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
145	145	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
146	146	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
147	147	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
148	148	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
149	149	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
150	150	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
151	151	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
152	152	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
153	153	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
154	154	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
155	155	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
156	156	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
157	157	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
158	158	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
159	159	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
160	160	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
161	161	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
162	162	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
163	163	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
164	164	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
165	165	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
166	166	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
167	167	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
168	168	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
169	169	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
170	170	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
171	171	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
172	172	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
173	173	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
174	174	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
175	175	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
176	176	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
177	177	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
178	178	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
179	179	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2
180	180	800-102	N/A	1	unknown	not specified	not specified	not specified	HRR Update 1987	2

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HRR Summary Table

REFERENCE NUMBER	HRR INTEGER (or database search)	HRR IDENTIFIER	PAC (or HRR) REFERENCE NUMBER	PC (or HRR) REFERENCE NUMBER	PC (or HRR) REFERENCE NUMBER	DURATION (in years)	TIME OF INCIDENT	TIME AFTER INCIDENT	REFERENCE DOCUMENT	COMMENTS	PRIORITY FACTOR (see Table 1)
221	N/A	N/A	N/A	52	N/A	1 day	No record of cleanup	No record of cleanup	HRR 1992		2
222	N/A	N/A	N/A	8	N/A	1 day	Send spread on road surface to soak up. Send to be picked up next day, but no record		HRR 1992		2
223	N/A	N/A	N/A	22	N/A	1 day	No record of cleanup	No record of cleanup	HRR 1992		2
224	N/A	N/A	N/A	6	N/A	1 day	reported, but no record of remediation		HRR 1992		2
225	N/A	N/A	N/A	1	N/A	1 day	Cleaned off equipment, but allowed to soak into ground		HRR 1992		2
226	N/A	N/A	N/A	13	N/A	not specified	compressor removed leaving behind oil stained concrete. No record of further cleanup		HRR 1992		2
227	N/A	N/A	N/A	11	N/A	1 day	Liquid captured by concrete pad was cleaned up. No record of cleanup of surrounding release		HRR 1992		2
228	N/A	N/A	N/A	45	N/A	1 day	Cleanup documented to have occurred, but no details		HRR 1992	East of 804 Pad	2
229	N/A	N/A	N/A	12	N/A	1 day	Immediately contained with "oil dry". No record of final disposition		HRR 1992		2
230	N/A	N/A	N/A	2	N/A	1 day	gasoline saturated soil was excavated and removed to a drum		HRR 1992		2
241	N/A	N/A	N/A	5	N/A	1 day	No record of cleanup	No record of cleanup	HRR 1992		2
242	N/A	N/A	N/A	27	N/A	1 day	No record of cleanup		HRR 1992	East Access Road	2
243	N/A	N/A	N/A	7	N/A	1 day	"Releases material secured to be disposed of"		HRR 1992	Poor description of remedial action	2
244	N/A	N/A	N/A	10	N/A	1 day	Haz Mat response team responded and "contained the situation"		HRR 1992		2
245	N/A	N/A	N/A	21	N/A	1 day	leak stopped and spill was "cleaned up and disposed of properly"		HRR 1992		2
246	N/A	N/A	N/A	23	N/A	1 day	HAZMAT response team applied "OH-On", then packaged and removed material to 334		HRR 1992	HAZMAT team indicates no likely release to environment	2
247	N/A	N/A	N/A	24	N/A	1 day	HAZMAT response team applied "OH-On", then packaged and removed material to 334		HRR 1992	HAZMAT team indicates no likely release to environment	2
248	N/A	N/A	N/A	25	N/A	1 day	HAZMAT response team applied "OH-On", then packaged and removed material to 334		HRR 1992	HAZMAT team indicates no likely release to environment	2
249	N/A	N/A	N/A	26	N/A	1 day	HAZMAT response team removed 1.5 cubic feet of contaminated material		HRR 1992	HAZMAT team indicates no likely release to environment. Location area is the indistinct	2
250	N/A	N/A	N/A	28	N/A	1 day	1.2 CF of contaminated material removed by HAZMAT team		HRR 1992	HAZMAT team indicates no likely release to environment	2
251	N/A	N/A	N/A	29	N/A	1 day	2 CF of waste material removed		HRR 1992	Report indicates no likely release to environment	2
252	N/A	N/A	N/A	30	N/A	1 day	3 CF of waste material removed		HRR 1992	Report indicates no likely release to environment	2
253	N/A	N/A	N/A	31	N/A	1 day	1.5 CF of contaminated material removed by HAZMAT team		HRR 1992	HAZMAT team indicates no likely release to environment	2
254	N/A	N/A	N/A	32	N/A	1 day	75 CF of contaminated material removed		HRR 1992	Report indicates no likely release to environment	2
255	N/A	N/A	N/A	33	N/A	1 day	1 CF of contaminated material removed		HRR 1992	Report indicates no likely release to environment	2
256	N/A	N/A	N/A	34	N/A	1 day	4 lbs of contaminated material removed by HAZMAT		HRR 1992	HAZMAT team indicates no likely release to environment	2
257	N/A	N/A	N/A	35	N/A	1 day	No cleanup because material evaporated before discovery		HRR 1992		2
258	N/A	N/A	N/A	36	N/A	1 day	0.5 CF of contaminated material removed by HAZMAT		HRR 1992	HAZMAT team indicates no likely release to environment	2
259	N/A	N/A	N/A	37	N/A	1 day	40 gallons of soil removed		HRR 1992	Report indicates no likely release to environment	2
260	N/A	N/A	N/A	38	N/A	1 day	3 CF of waste material removed		HRR 1992	Report indicates no likely release to environment	2
261	N/A	N/A	N/A	39	N/A	1 day	1 CF of contaminated material removed by HAZMAT		HRR 1992	HAZMAT team indicates no likely release to environment	2
262	N/A	N/A	N/A	40	N/A	1 day	21 CF of contaminated material removed		HRR 1992	Report indicates no likely release to environment	2
263	N/A	N/A	N/A	48	N/A	1 day	Cleaned by Site Haz Mat team		HRR 1992		2
264	N/A	N/A	N/A	49	N/A	unknown	1 CF of soil removed and leak fixed		HRR 1992		2
265	110	110	NE-110	N/A	2	13	not specified	not specified	HRR 1992	T-2 thru T-11 125,000 kg of sludge disposed during 1995-1998	2
266	110	110	NE-110	N/A	2	13	not specified	Source removal action completed in summer 1998, excavating and removing VOCs from soil using low-temperature thermal desorption	HRR 2000	Treated soils were returned to the excavation. Clean-up levels were 11 ppm for COH and 11.5 ppm TCE. Detectable VOCs in excavation verification samples: 1.8 ppm COH and 0.74 to 8.2 ppm TCE.	2

### HRR Summary Table

[illegible]

232

82928



### HRR Summary Table

8272

234

(or suspected > 100 gal.)  
(or suspected < 100 gal.)

242

Table C-1  
Modeled Parameter Ranges  
(Report Section 6)

PSA No.	Chain Modeled	Dispersivity (m <sup>2</sup> /day)			Sorption Coefficient (L/mg)				Porosity			Degradation (day <sup>-1</sup> )			
		Low Values	Mod- erate	High Values	Sorption Analyte	Low Values	Moderate	High Values	Low Values	Mod- erate	High Values	Degradation Analyte	Low Values	Mod- erate	High Values
2 North/ 6 and 7	CCl <sub>4</sub>	5	10	20	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.5 x 10 <sup>-7</sup>	0.05	0.1	0.2	CCl <sub>4</sub>	0.00004	0.0004	0.004
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.8 x 10 <sup>-8</sup>				CCl <sub>3</sub>	0.00003	0.0003	0.003
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-9</sup>				CCl <sub>2</sub>	0.00002	0.0002	0.002
2 North/ 6 and 7	PCE/TCE	8	10	20	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.05	0.1	0.2	PCE	0.00004	0.0004	0.004
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-7</sup>				TCE	0.00004	0.0004	0.004
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-7</sup>				DCE	0.0002	0.002	0.02
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-9</sup>				VC	0.002	0.02	0.2
2 South	CCl <sub>4</sub>	5	15	25	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.5 x 10 <sup>-7</sup>	0.05	0.1	0.2	CCl <sub>4</sub>	0.00004	0.0004	0.004
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.8 x 10 <sup>-8</sup>				CCl <sub>3</sub>	0.00003	0.0003	0.003
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-9</sup>				CCl <sub>2</sub>	0.00002	0.0002	0.002
2 South	PCE/TCE	5	15	25	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.05	0.1	0.2	PCE	0.00003	0.0003	0.003
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-7</sup>				TCE	0.00001	0.0001	0.001
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-7</sup>				DCE	0.00001	0.0001	0.001
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-9</sup>				VC	0.001	0.01	0.1
5	PCE/TCE	5	15	25	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	PCE	0.00003	0.0003	0.003
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-7</sup>				TCE	0.00005	0.0005	0.005
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-7</sup>				DCE	0.0002	0.002	0.02
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-9</sup>				VC	0.02	0.2	0.2
9	CCl <sub>4</sub>	5	15	30	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	CCl <sub>4</sub>	0.0002	0.002	0.02
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				CCl <sub>3</sub>	0.0001	0.001	0.01
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-6</sup>				CCl <sub>2</sub>	1E-06	1E-05	0.0001
9	PCE/TCE	5	15	30	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	PCE	0.001	0.01	0.1
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				TCE	0.0005	0.005	0.05
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-6</sup>				DCE	0.001	0.01	0.1
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-6</sup>				VC	0.01	0.1	1

Table C-1  
Modeled Parameter Ranges  
(Report Section 6)

PSA No.	Chain Modeled	Dispersivity (m <sup>2</sup> /day)			Sorption Coefficient (L/mg)				Porosity			Degradation (day <sup>-1</sup> )			
		Low Values	Mod- erate	High Values	Sorption Analyte	Low Values	Moderate	High Values	Low Values	Mod- erate	High Values	Degradation Analyte	Low Values	Mod- erate	High Values
10	CCl <sub>4</sub>	5	15	30	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	CCl <sub>4</sub>	0.0002	0.002	0.02
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				CCl <sub>3</sub>	0.0001	0.001	0.01
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-6</sup>				CCl <sub>2</sub>	0.0003	0.003	0.03
12	CCl <sub>4</sub>	5	15	30	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	CCl <sub>4</sub>	0.002	0.02	0.2
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				CCl <sub>3</sub>	0.001	0.01	0.1
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-6</sup>				CCl <sub>2</sub>	0.003	0.03	0.3
12	PCE/TCE	5	15	30	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	PCE	0.0005	0.005	0.05
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				TCE	0.0005	0.005	0.05
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-6</sup>				DCE	0.001	0.01	0.1
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-6</sup>				VC	0.001	0.01	0.1
14	CCl <sub>4</sub>	2	5	20	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	CCl <sub>4</sub>	0.00002	0.0002	0.002
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				CCl <sub>3</sub>	0.0001	0.001	0.01
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-6</sup>				CCl <sub>2</sub>	0.0003	0.003	0.03
15	CCl <sub>4</sub>	5	15	30	CCl <sub>4</sub>	1.8 x 10 <sup>-8</sup>	1.8 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	CCl <sub>4</sub>	0.00005	0.0005	0.005
					CCl <sub>3</sub>	1.9 x 10 <sup>-9</sup>	1.9 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				CCl <sub>3</sub>	0.00005	0.0005	0.005
					CCl <sub>2</sub>	3.0 x 10 <sup>-10</sup>	2.8 x 10 <sup>-9</sup>	2.0 x 10 <sup>-6</sup>				CCl <sub>2</sub>	0.00005	0.0005	0.005
15	PCE/TCE	5	15	30	PCE	1.5 x 10 <sup>-8</sup>	1.5 x 10 <sup>-7</sup>	3.7 x 10 <sup>-6</sup>	0.03	0.1	0.3	PCE	0.0002	0.002	0.02
					TCE	5.0 x 10 <sup>-9</sup>	5.0 x 10 <sup>-8</sup>	3.0 x 10 <sup>-6</sup>				TCE	0.0001	0.001	0.01
					DCE	3.0 x 10 <sup>-9</sup>	2.6 x 10 <sup>-8</sup>	2.0 x 10 <sup>-6</sup>				DCE	0.0003	0.003	0.03
					VC	8.0 x 10 <sup>-11</sup>	8.0 x 10 <sup>-10</sup>	2.0 x 10 <sup>-6</sup>				VC	0.003	0.03	0.3

Table C-2  
Modeled Concentration Ranges at Inferred Sources

PSA	Row	Column	Layer (see definition below)	CCl4 (ppm)	PCE (ppm)	TCE (ppm)
5	43	93	3		150.0	
5	43	92	3		150.0	
5	42	89	3		18.0	11.0
9	69	69	3	0.3	0.2	0.4
9	69	70	3	0.3	0.2	1.7
10	57	25	2		0.5	2.0
10	60	30	2		1.0	
10	61	27	2		0.1	
12	36	41	2	2.0		
12	33	43	2	0.7		
12	31	43	2	0.7		
12	33	43	2		0.2	0.5
12	38	39	2		0.5	0.5
12	35	41	2		0.5	0.5
12	37	24	2		0.3	0.2
12	44	47	2		0.1	3.0
12	36	36	2		1.0	2.0
12	44	34	2		1.5	0.5
14	24	54	3	4.5		
14	24	53	3	4.5		
14	24	52	3	4.5		
14	23	50	4	1.9		
14	23	49	4	1.9		
14	23	48	4	1.9		
15	23	64	3	8.0	0.1	1.0
15	22	64	3	8.0	0.1	1.0
2N	51	88	3	10.0	30.0	4.0
2N	51	89	3	10.0	10.0	2.0
2N	52	87	3	10.0	10.0	2.0
2N	52	88	3	2.0	2.0	
2N	52	89	3	2.0	2.0	
2N	51	88	3	10.0	10.0	4.0
2N	51	89	3	10.0	10.0	2.0
2N	52	87	3	10.0	10.0	2.0
2N	52	88	3	2.0	2.0	
2N	52	89	3	2.0	2.0	
2N	38	109	3	2.0	2.0	
2N	38	110	3	2.0	2.0	
2N	38	111	3	2.0	2.0	
2N	38	112	3	2.0	2.0	
2N	39	104	3	2.0	2.0	
2N	39	105	3	2.0	2.0	
2N	39	106	3	2.0	2.0	
2N	39	107	3	2.0	2.0	
2N	38	113	3	2.0	2.0	
2N	39	112	3	2.0	2.0	
2N	39	113	3	2.0	2.0	
2N	40	108	3	2.0	2.0	
2N	40	109	3	2.0	2.0	
2N	40	110	3	2.0	2.0	
2N	40	112	3	2.0	2.0	
2N	40	113	3	2.0	2.0	
2N	40	112	3	2.0	2.0	
2N	40	113	3	2.0	2.0	

Table C-2  
Modeled Concentration Ranges at Inferred Sources

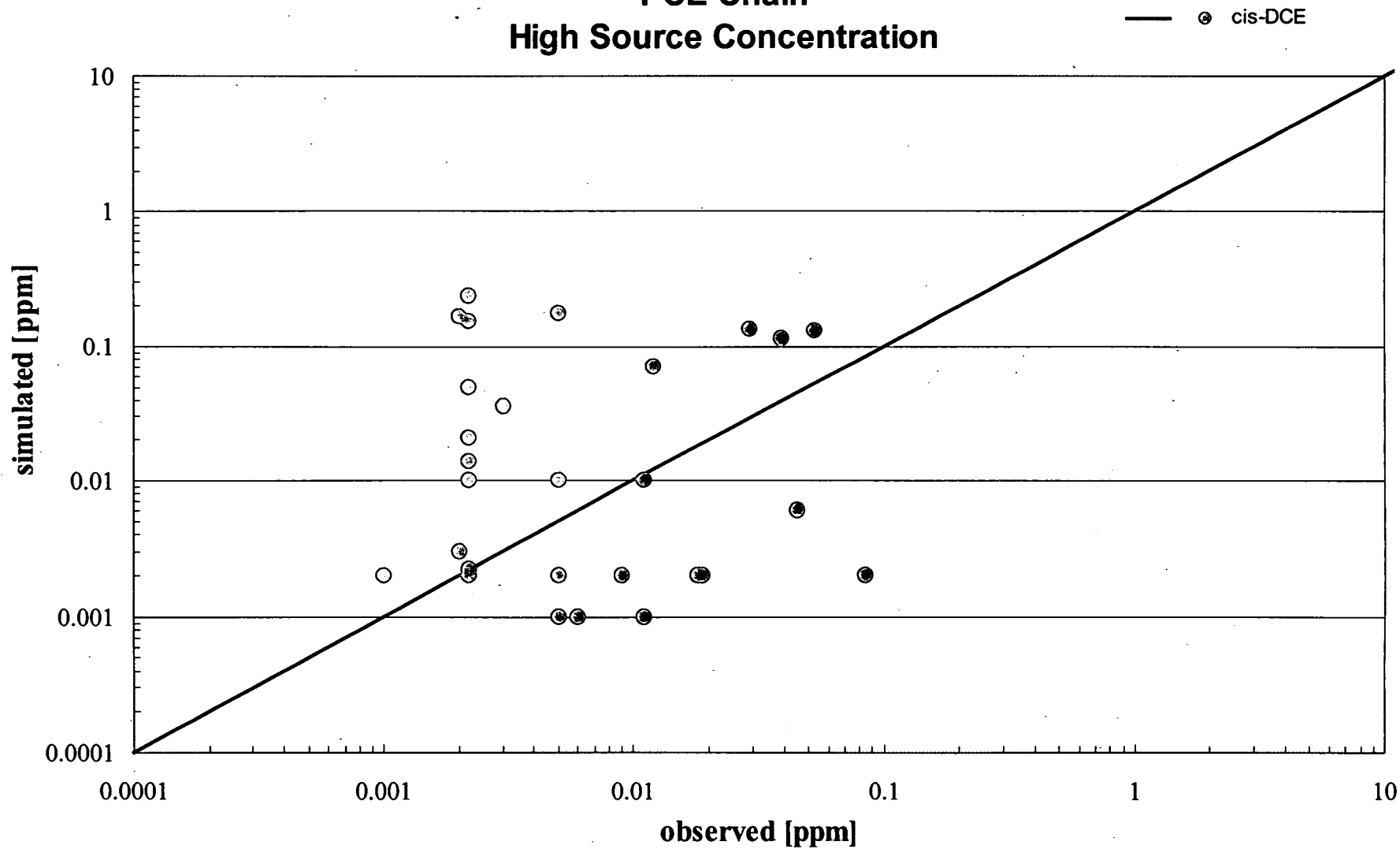
PSA	Row	Column	Layer (see definition below)	CCl4 (ppm)	PCE (ppm)	TCE (ppm)
2S	50	87	3	10.0	20.0	6.0
2S	51	83	3	10.0	1.0	1.0
2S	51	85	3	40.0	2.0	0.2
2S	51	88	3	4.0	5.0	2.0
2S	51	89	3	40.0	10.0	2.0
2S	55	88	3		0.2	
2S	58	88	3	1.0	4.0	40.0
2S	59	87	3	2.0	4.0	40.0

2: Unconsolidated Material  
3: Upper Weathered Bedrock  
4: Lower Weathered Bedrock

245

912

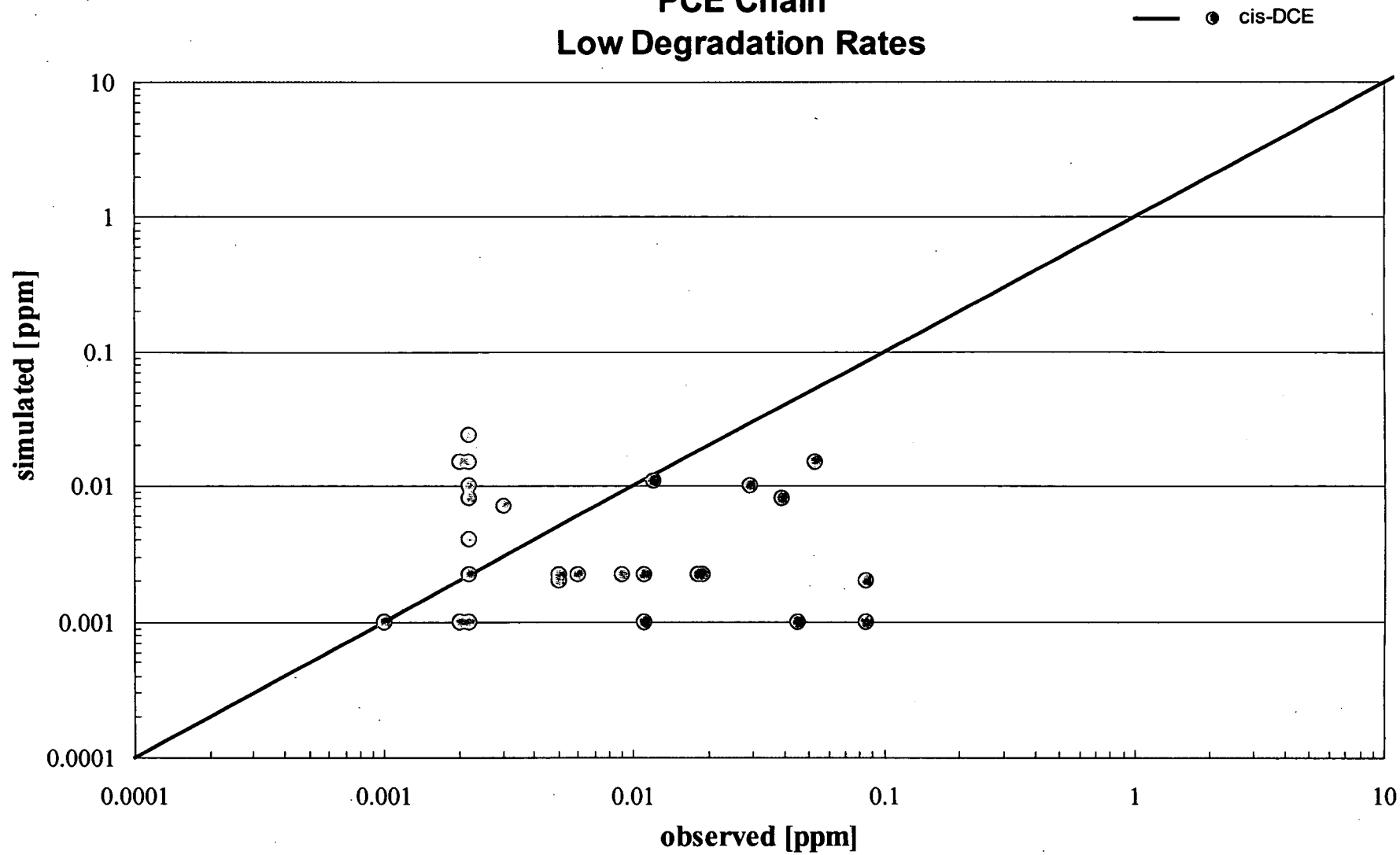
**PSA 6 and 7  
PCE Chain  
High Source Concentration**



**Figure D.1. PSA 6 and 7 - High PCE Daughter Products.**

247

**PSA 6 and 7  
PCE Chain  
Low Degradation Rates**



**Figure D.2. PSA 6 and 7 - Low PCE Daughter Products.**



2/2

PSA 2S  
PCE Chain  
High Conductivity

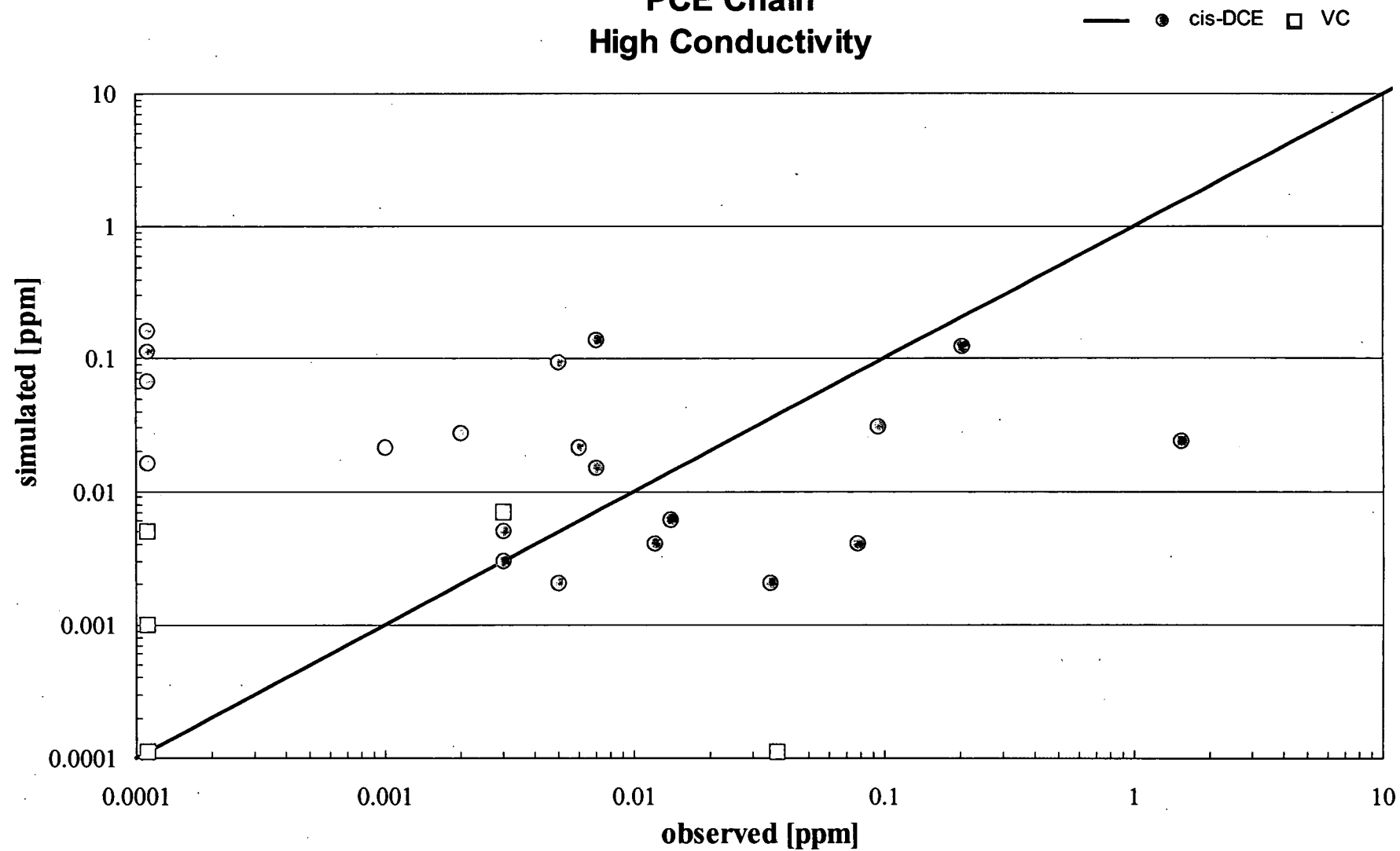


Figure D.3. PSA 2S - High PCE Daughter Products.

24

PSA 2S  
PCE Chain  
Low Degradation Rates

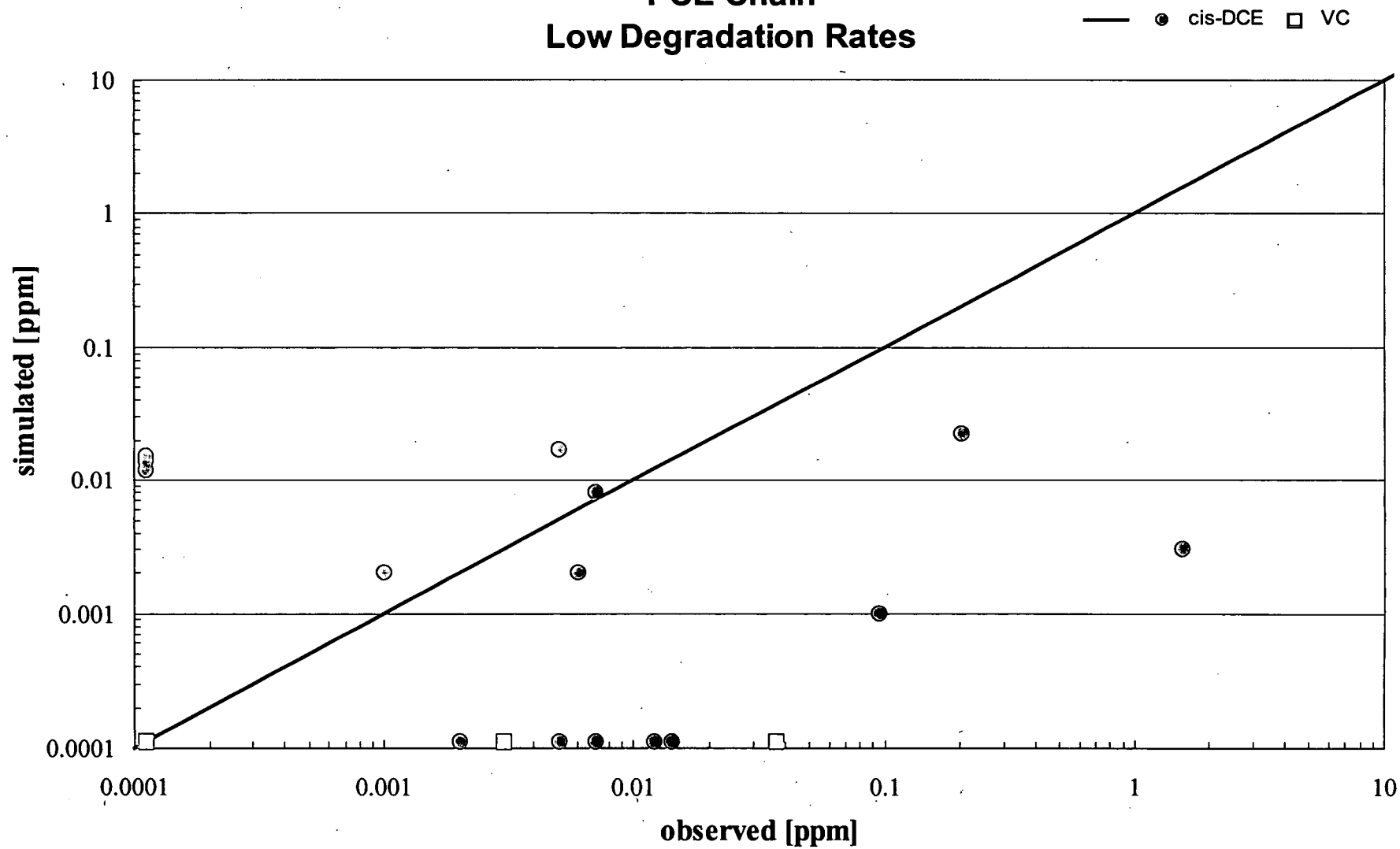


Figure D.4. PSA 2S - Low PCE Daughter Products.

PSA 5  
PCE Chain  
High Conductivity

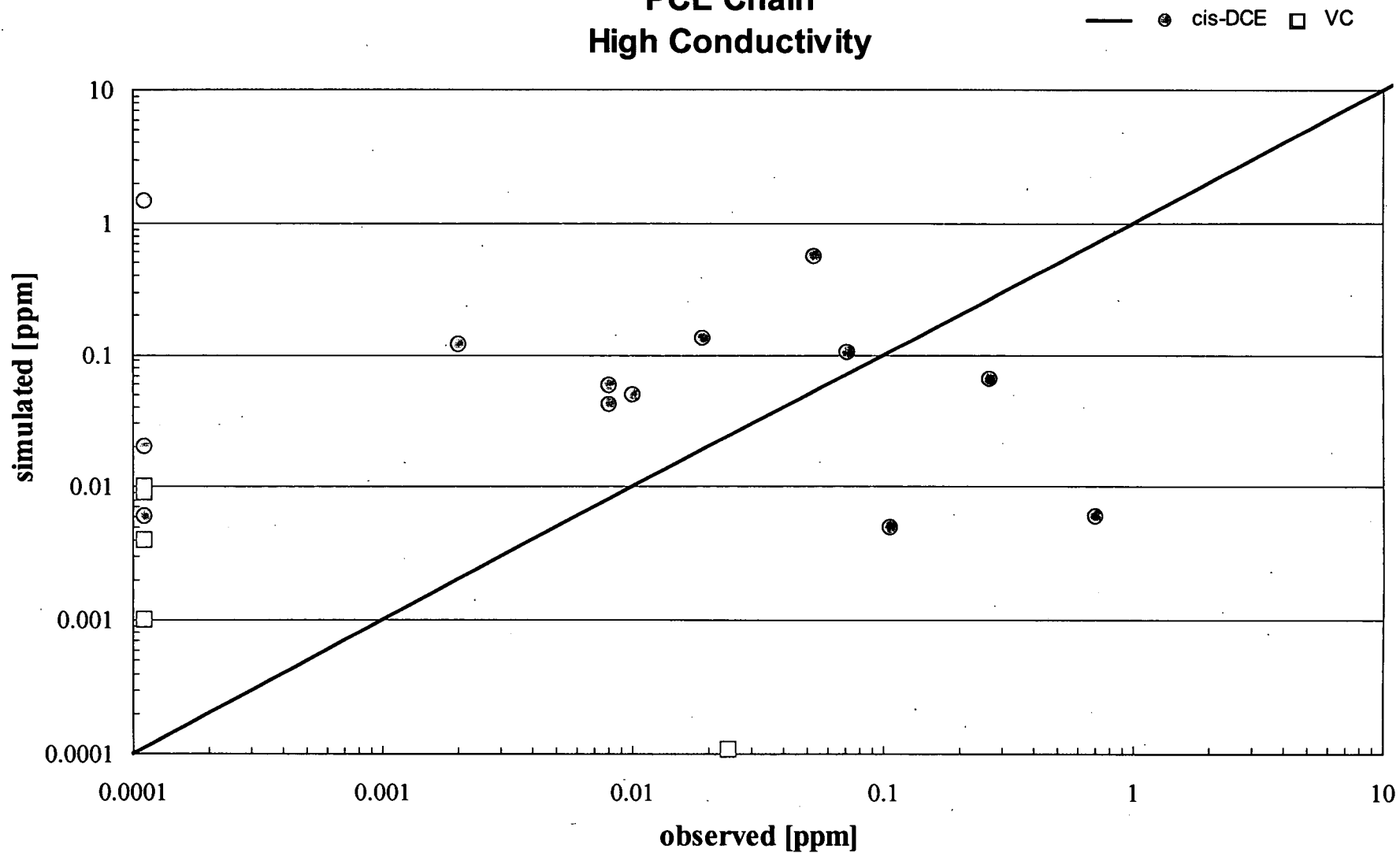


Figure D.5. PSA 5 - High PCE Daughter Products.

251

PSA 5  
PCE Chain  
Low Degradation Rates

— ● cis-DCE □ VC

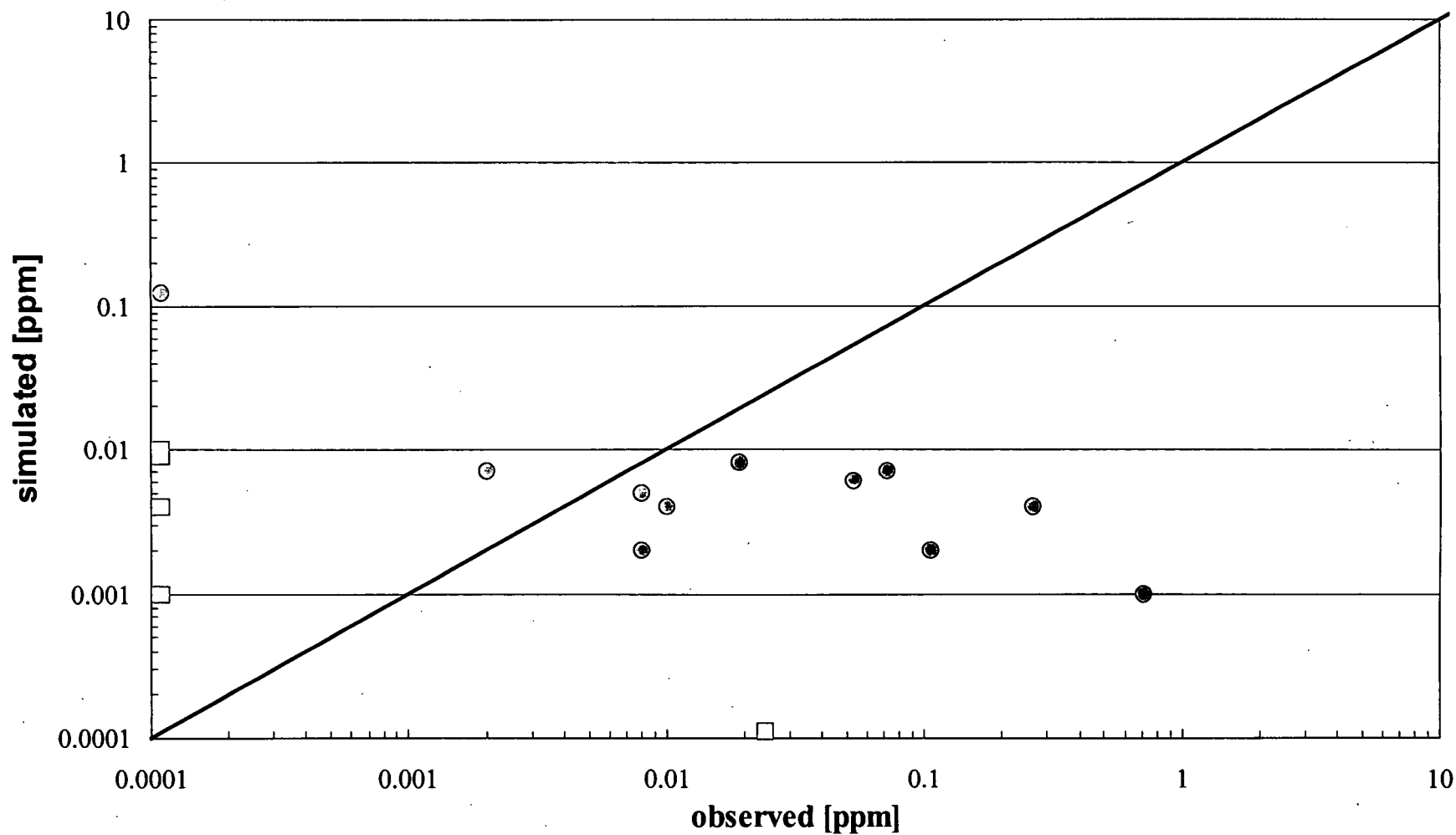
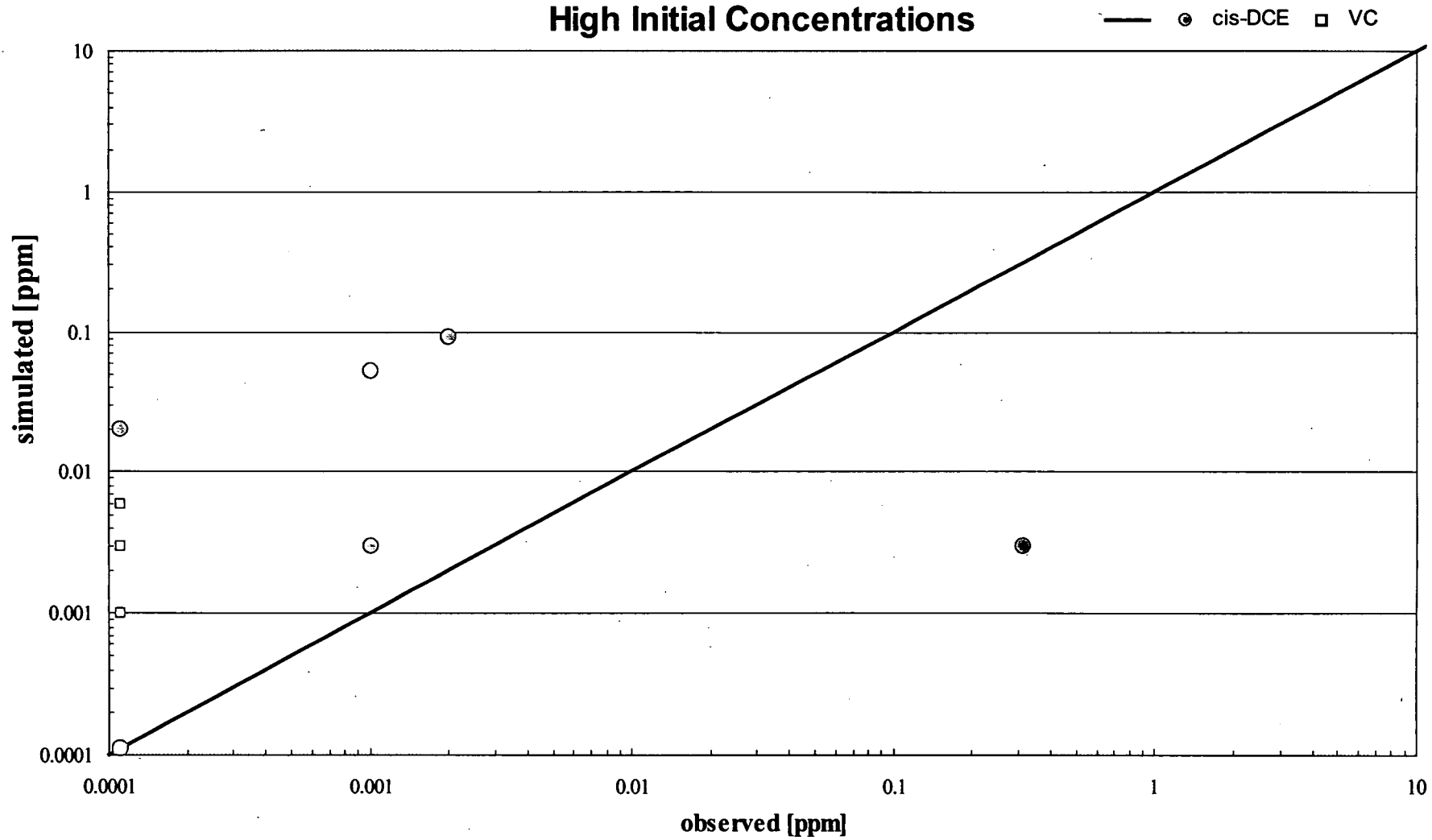


Figure D.6. PSA 5 - Low PCE Daughter Products.

252

**PSA 9  
PCE Chain  
High Initial Concentrations**



**Figure D.7. PSA 9 - High PCE Daughter Products.**

PSA 9  
PCE Chain  
Low Degradation Rates

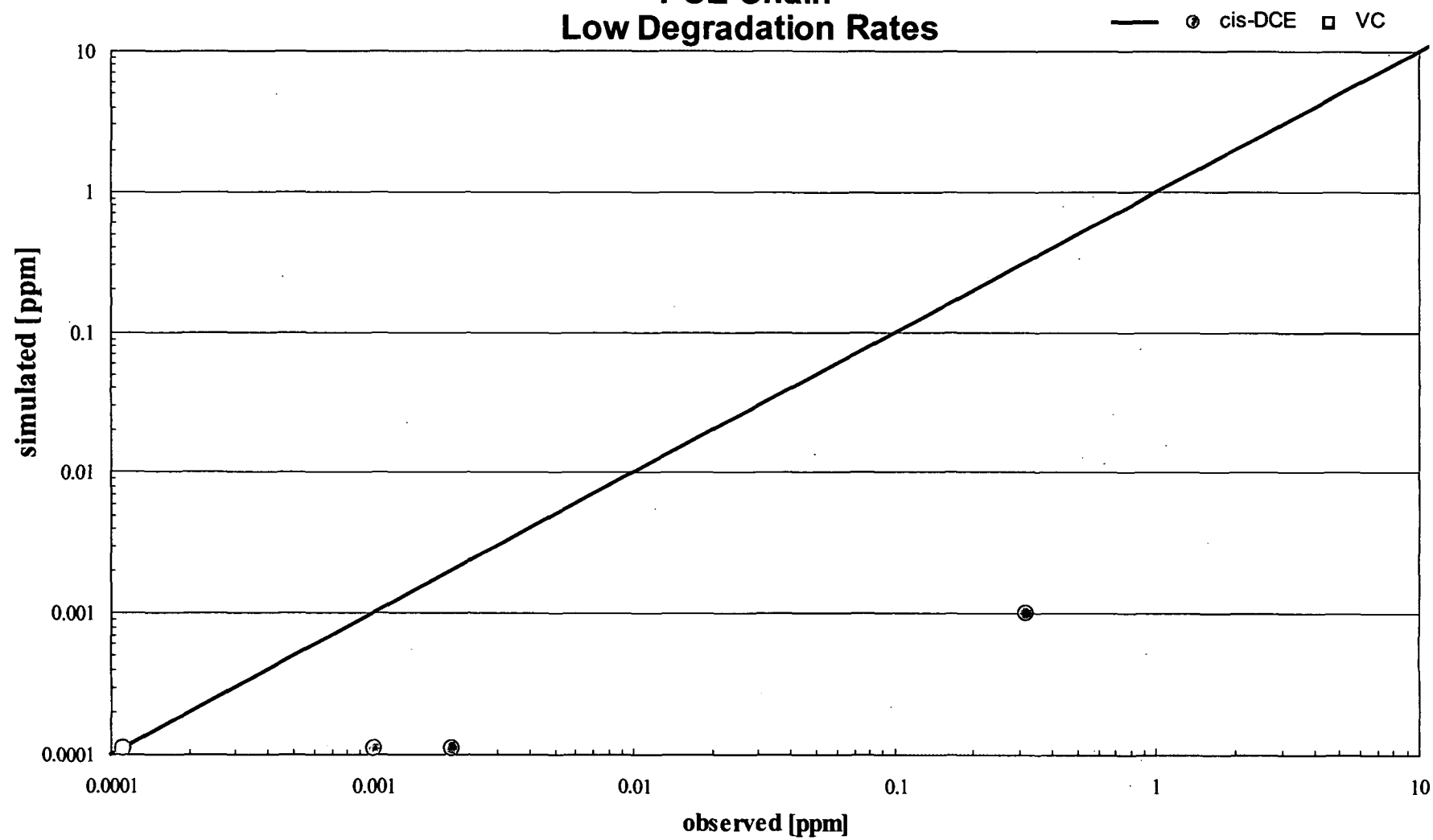


Figure D.8. PSA 9 - Low PCE Daughter Products.

254

PSA 10  
PCE Chain  
High Initial Concentrations

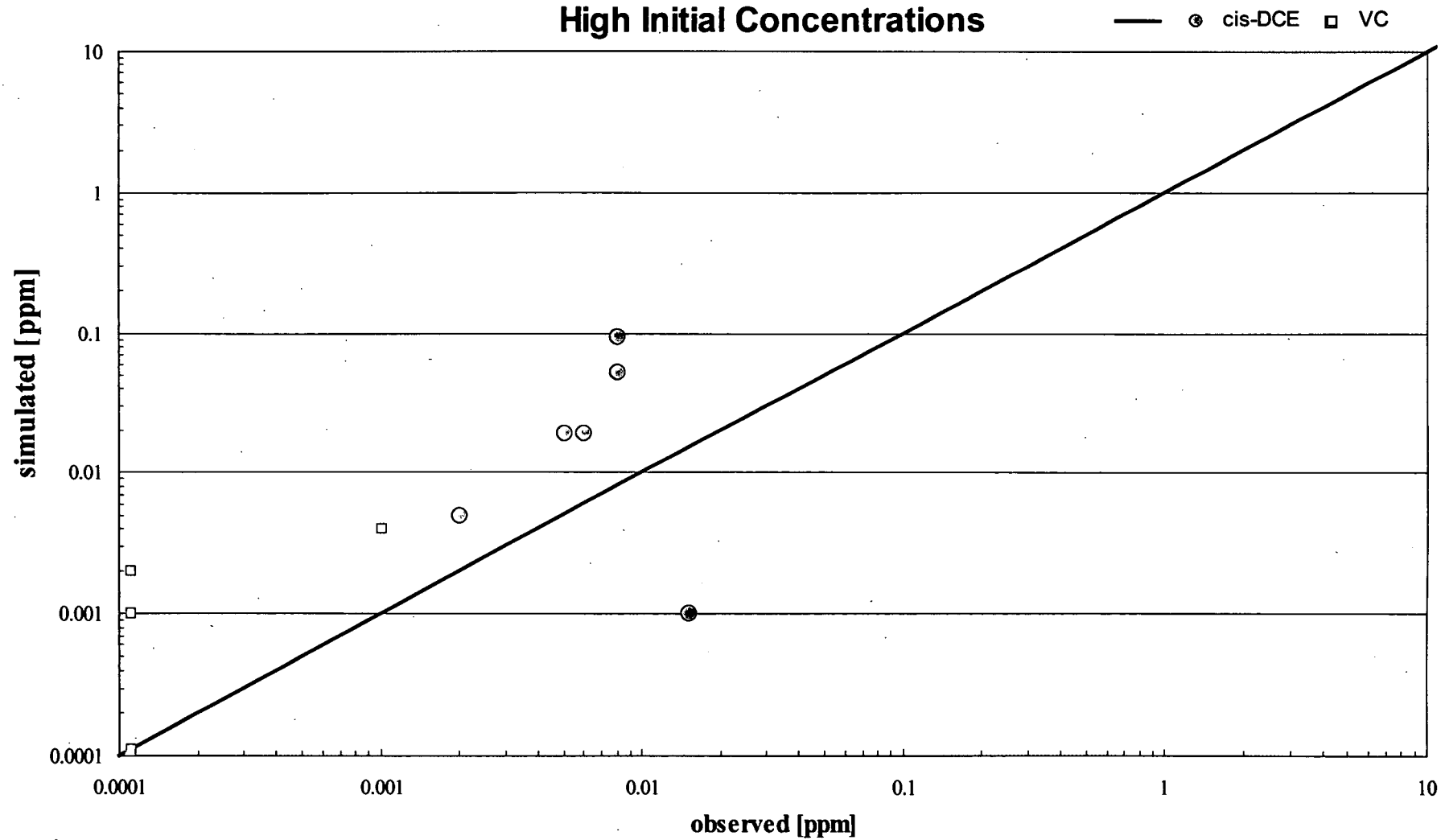


Figure D.9. PSA 10 - High PCE Daughter Products.

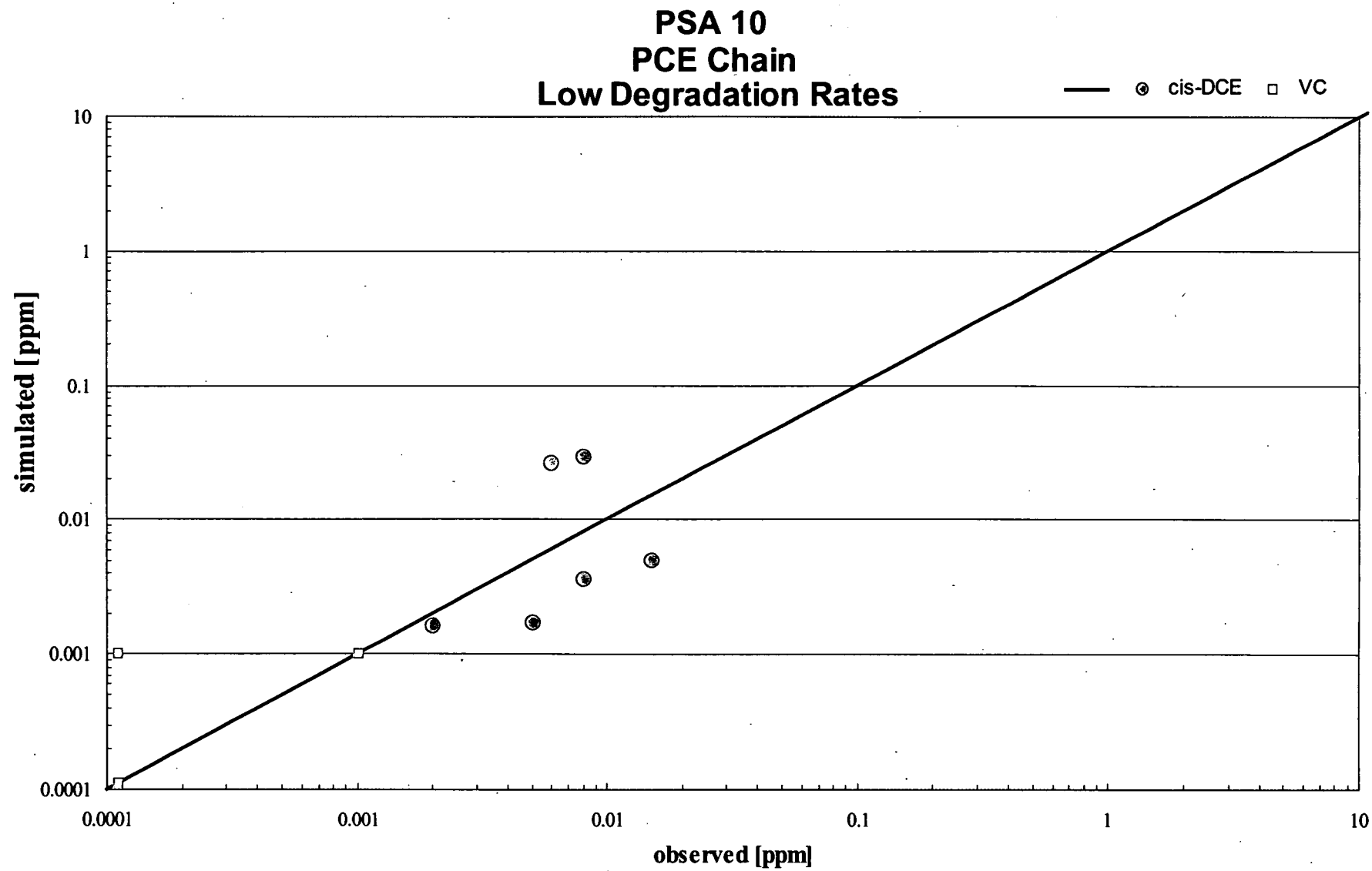


Figure D.10. PSA 10 - Low PCE Daughter Products.



256

PSA 12  
PCE Chain  
High Initial Concentrations

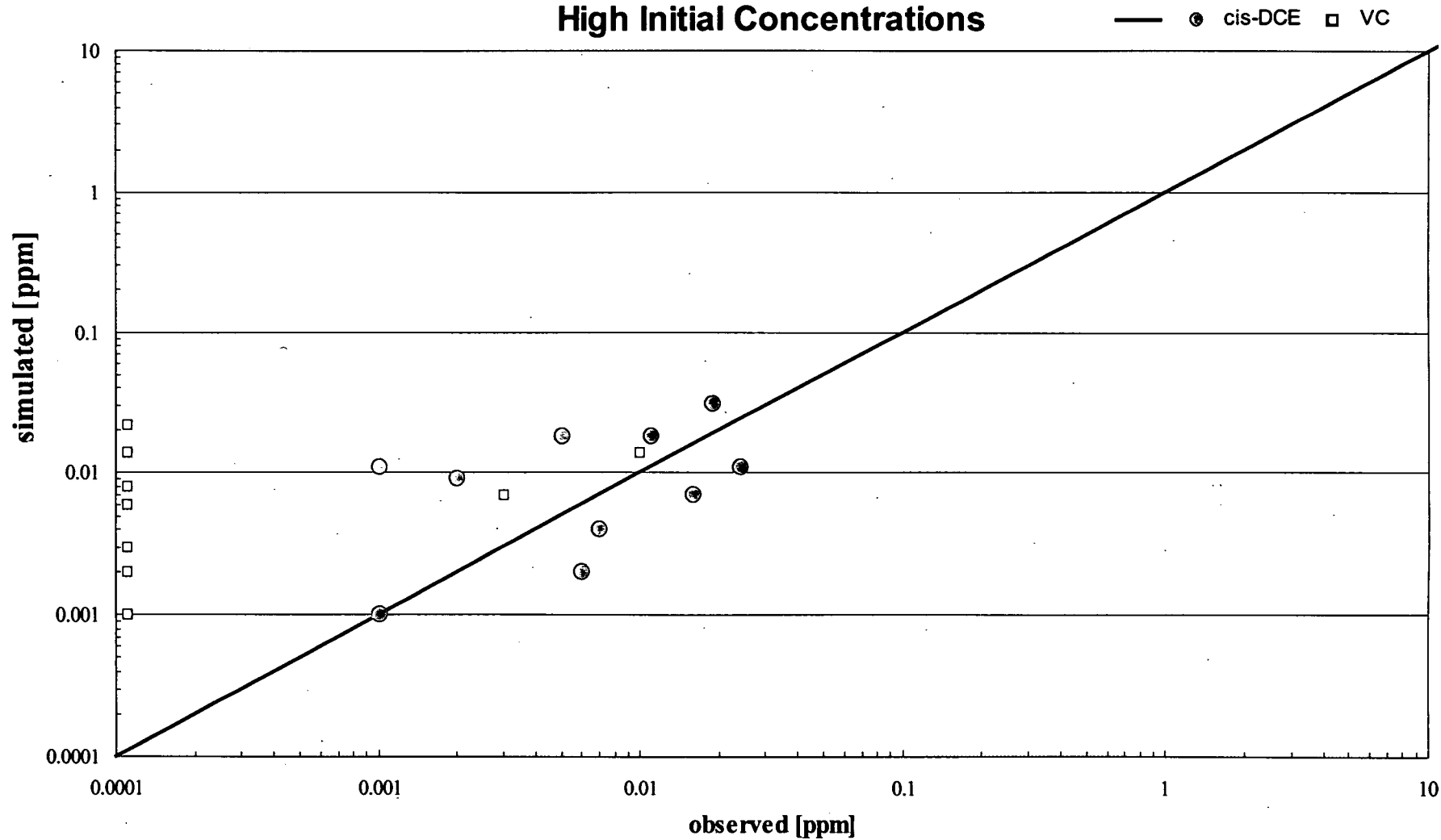
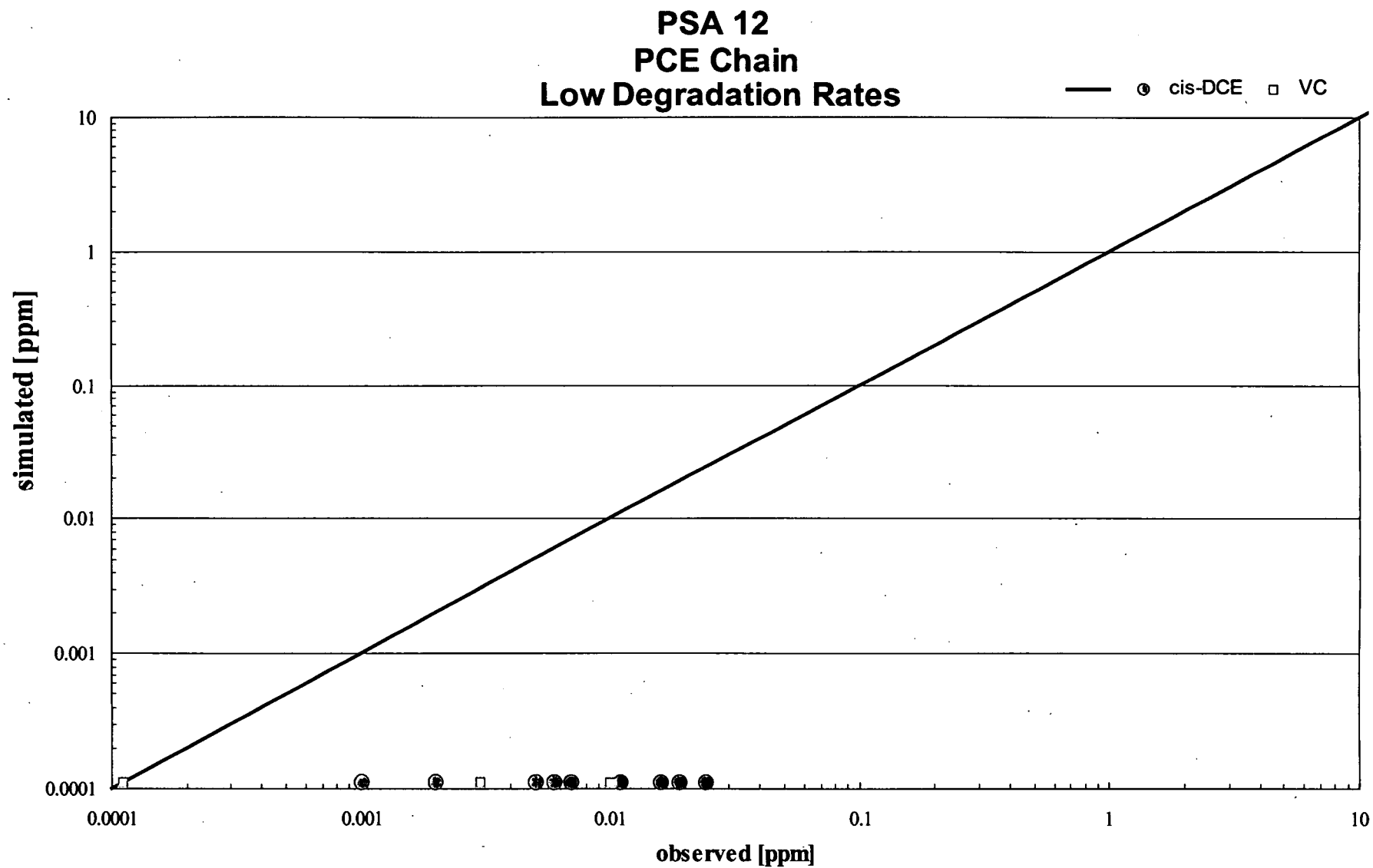


Figure D.11. PSA 12 - High PCE Daughter Products.



**Figure D.12. PSA 12 - Low PCE Daughter Products.**

PSA 14  
CCl<sub>4</sub> Chain  
High Initial Concentrations

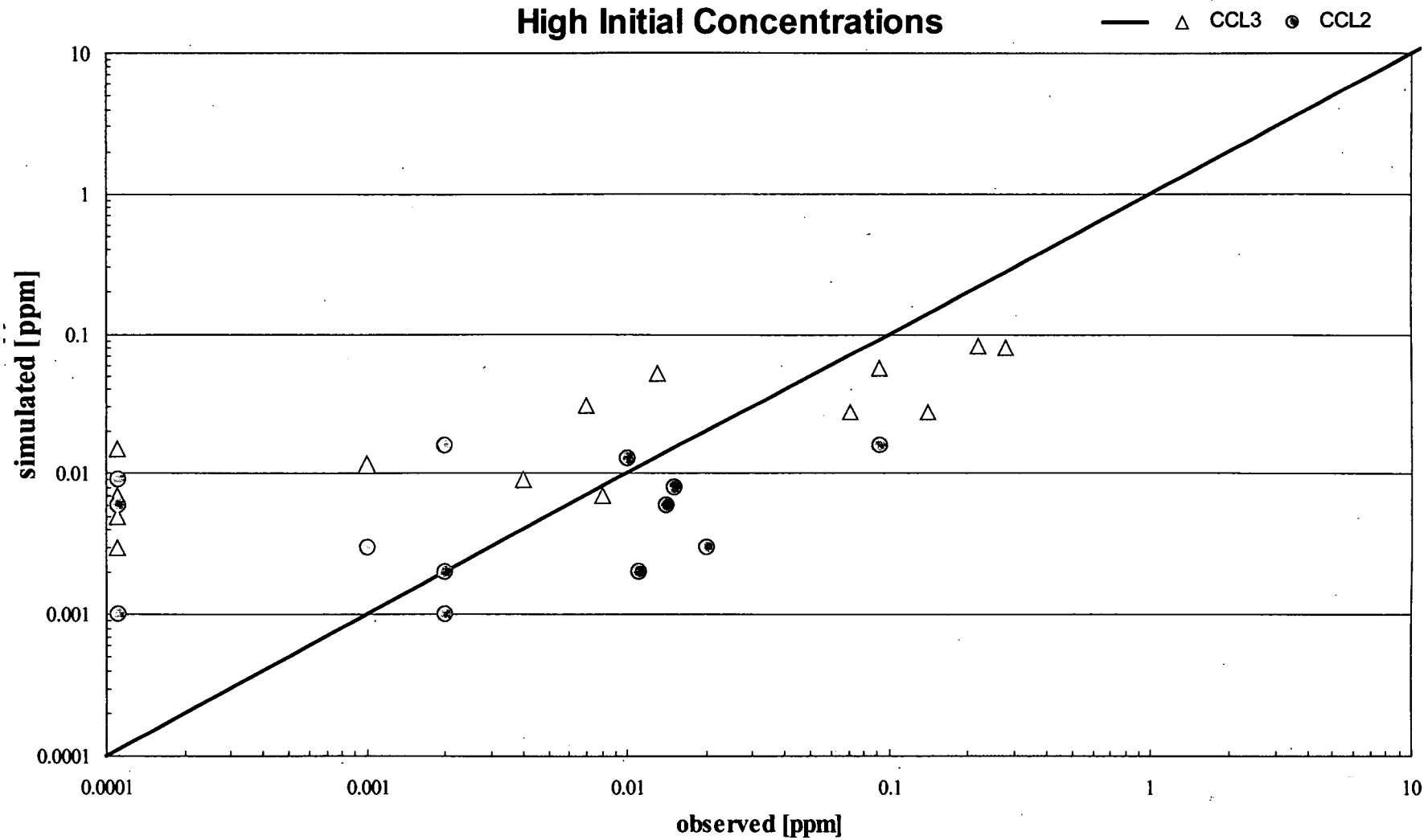


Figure D.13. PSA 14 - High CCl<sub>4</sub> Daughter Products.

259

PSA 14  
CCl<sub>4</sub> Chain  
Low Degradation Rates

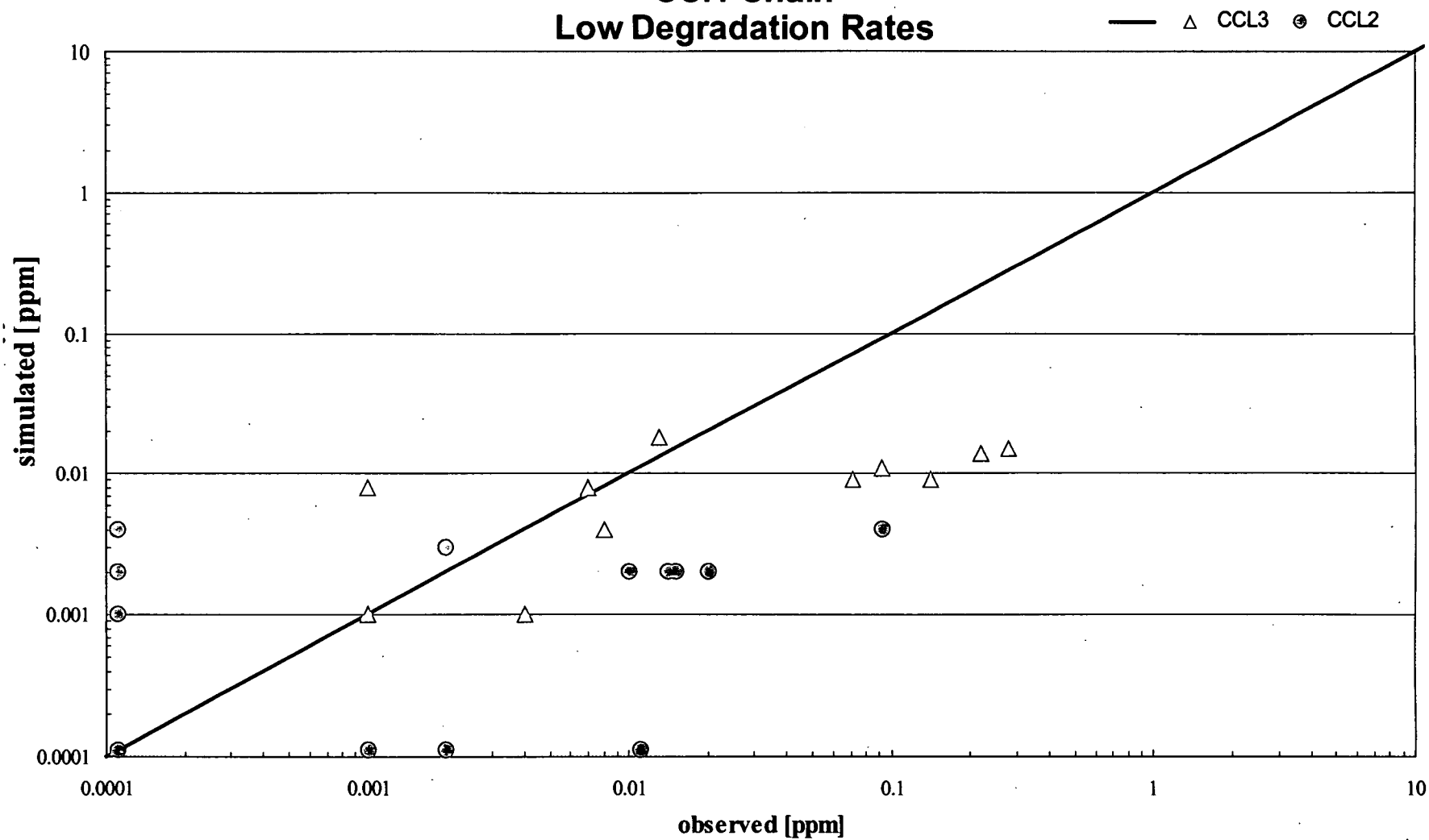


Figure D.14. PSA 14 - Low CCl<sub>4</sub> Daughter Products.

simulated [ppm]

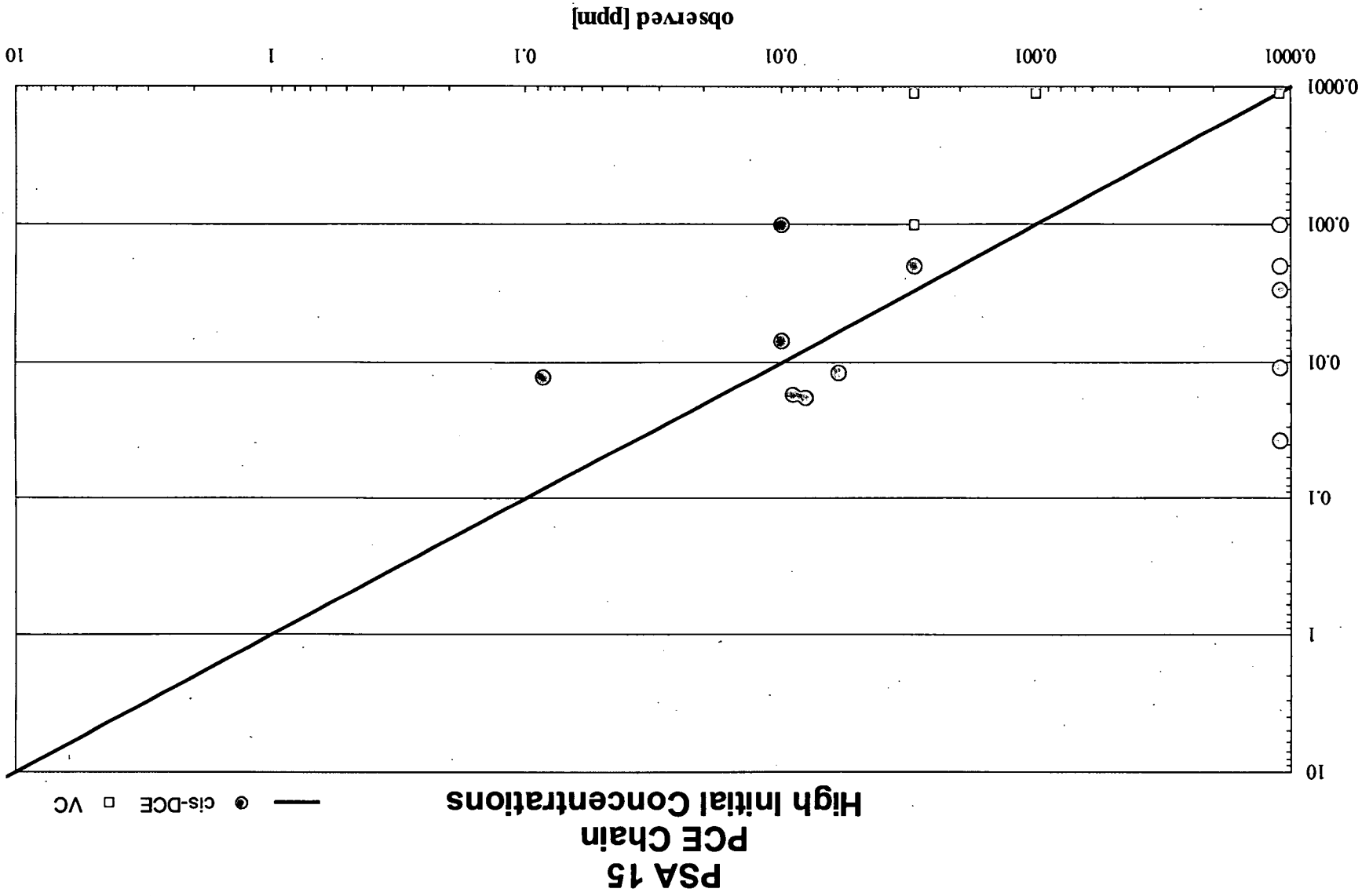
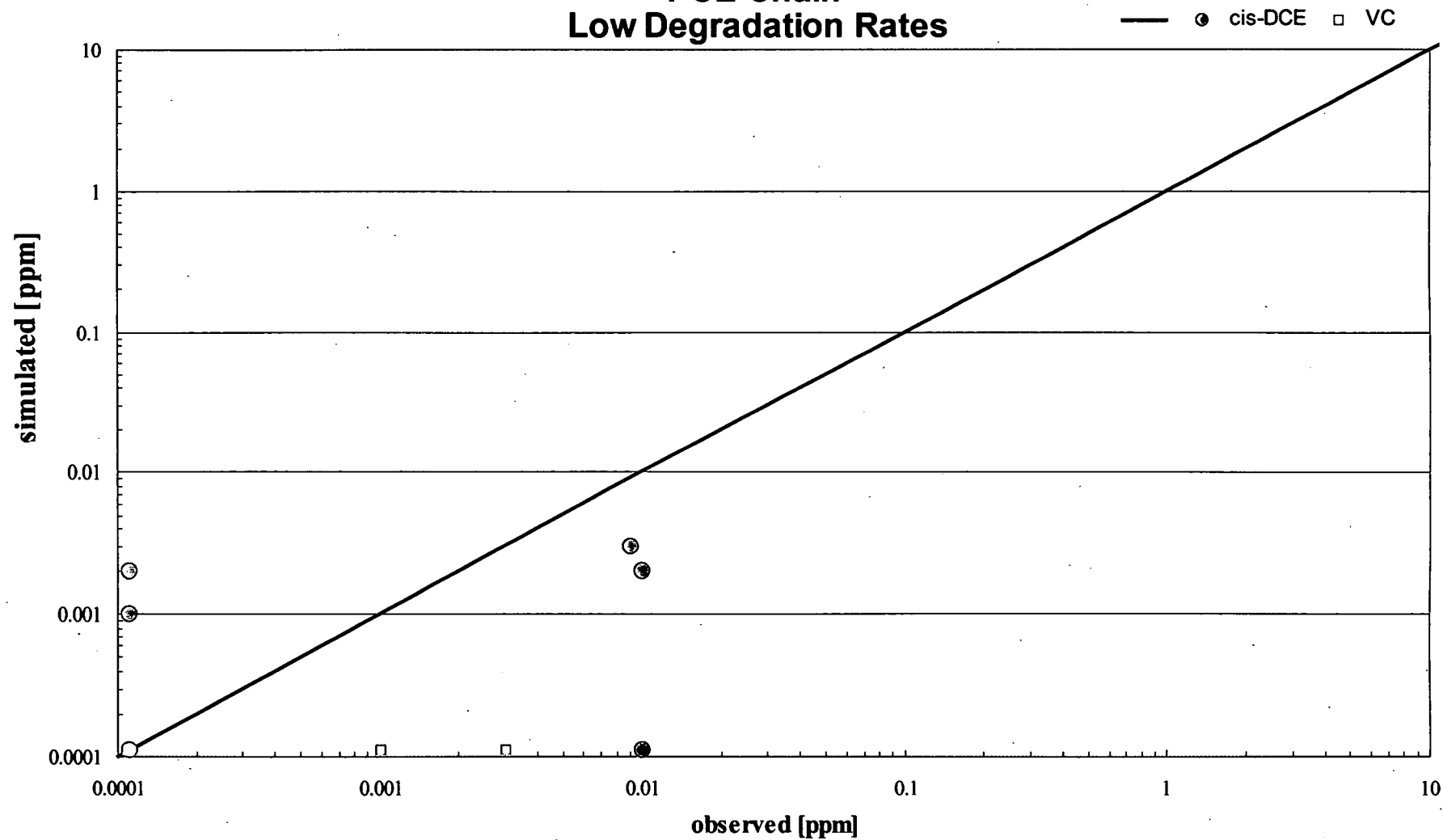


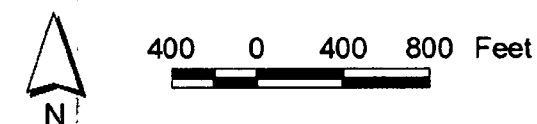
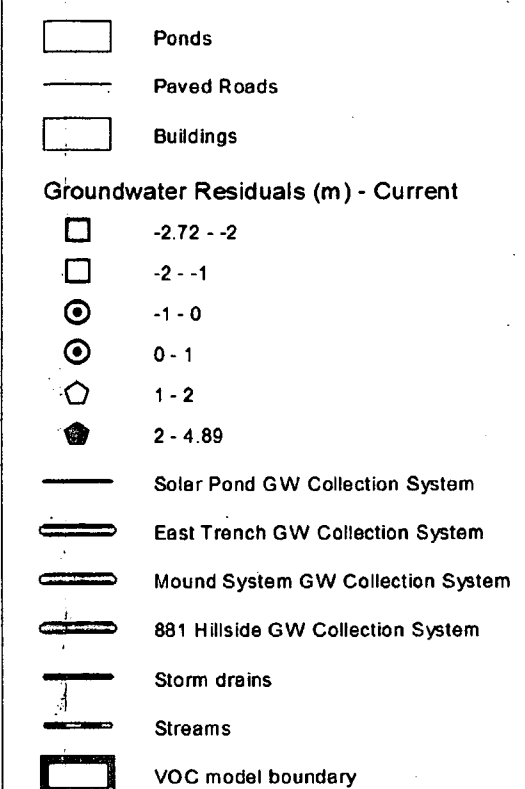
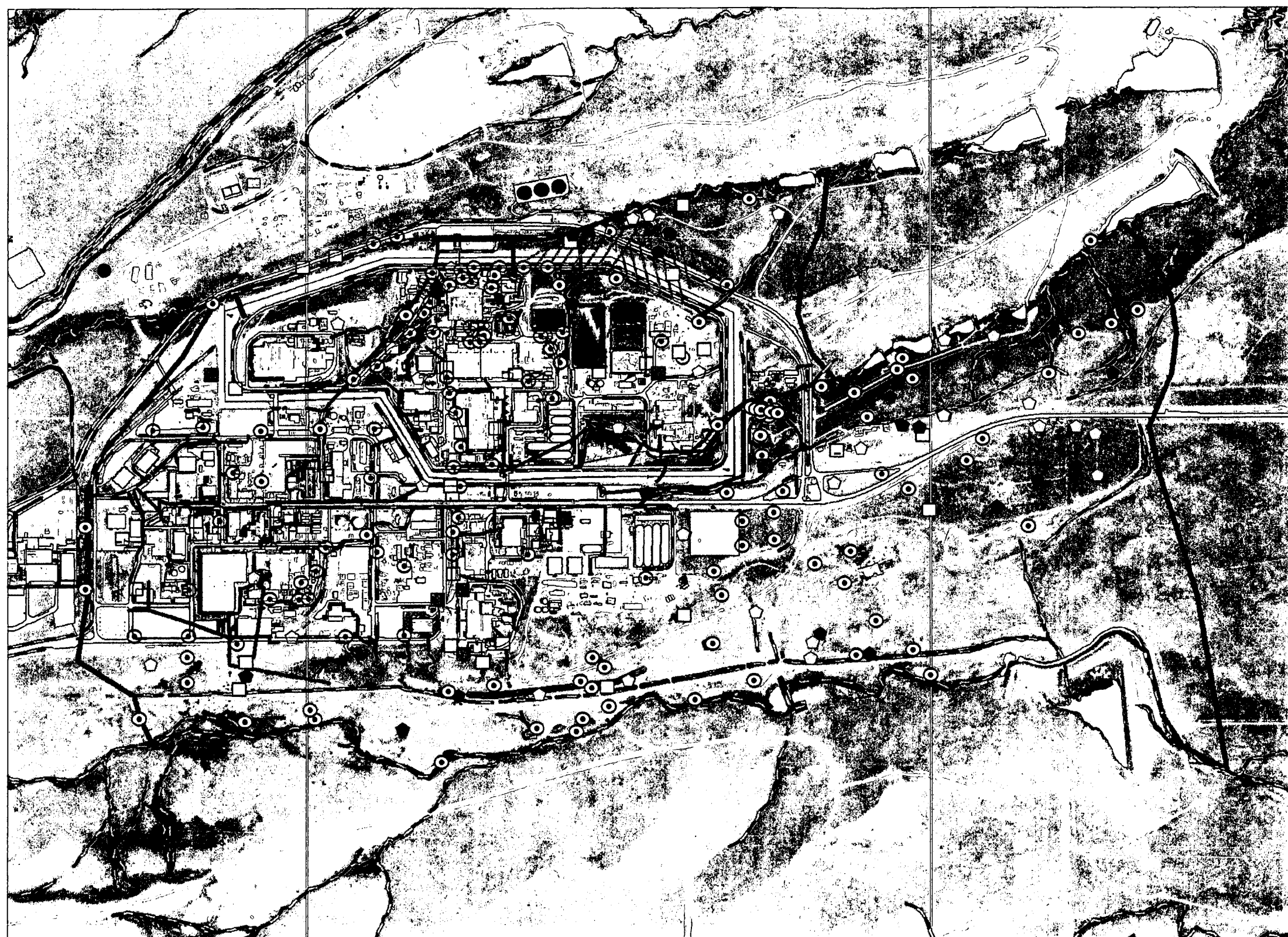
Figure D.15. PSA 15 - High PCE Daughter Products.

192  
261  
192

**PSA 15  
PCE Chain  
Low Degradation Rates**



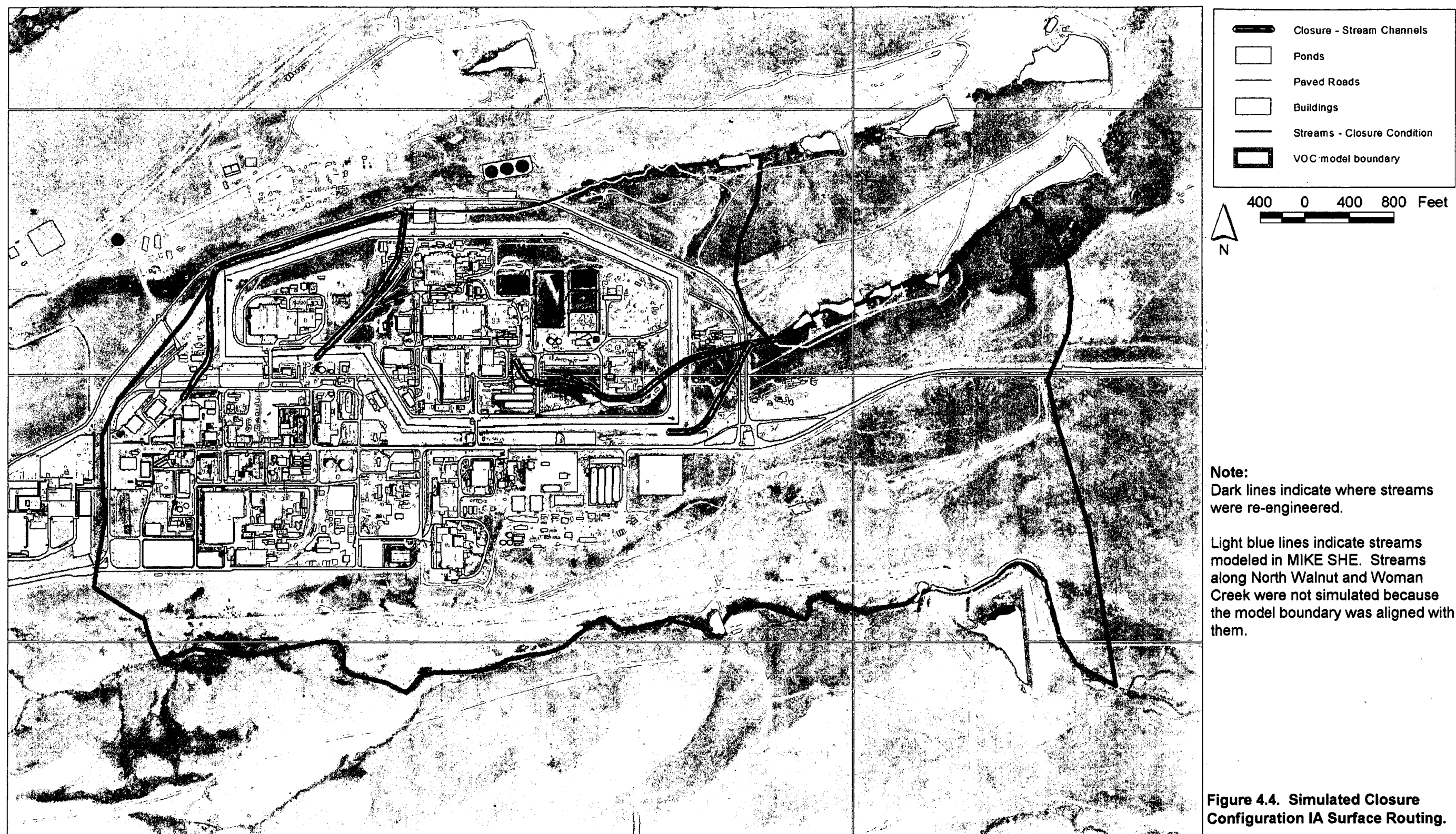
**Figure D.16. PSA 15 - Low PCE Daughter Products.**



**Note:**  
Positive numbers indicate simulated heads higher than observed.

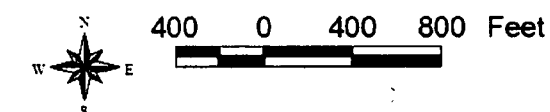
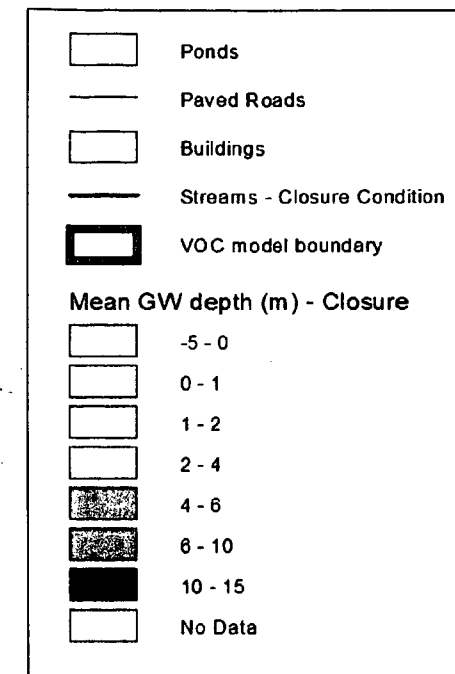
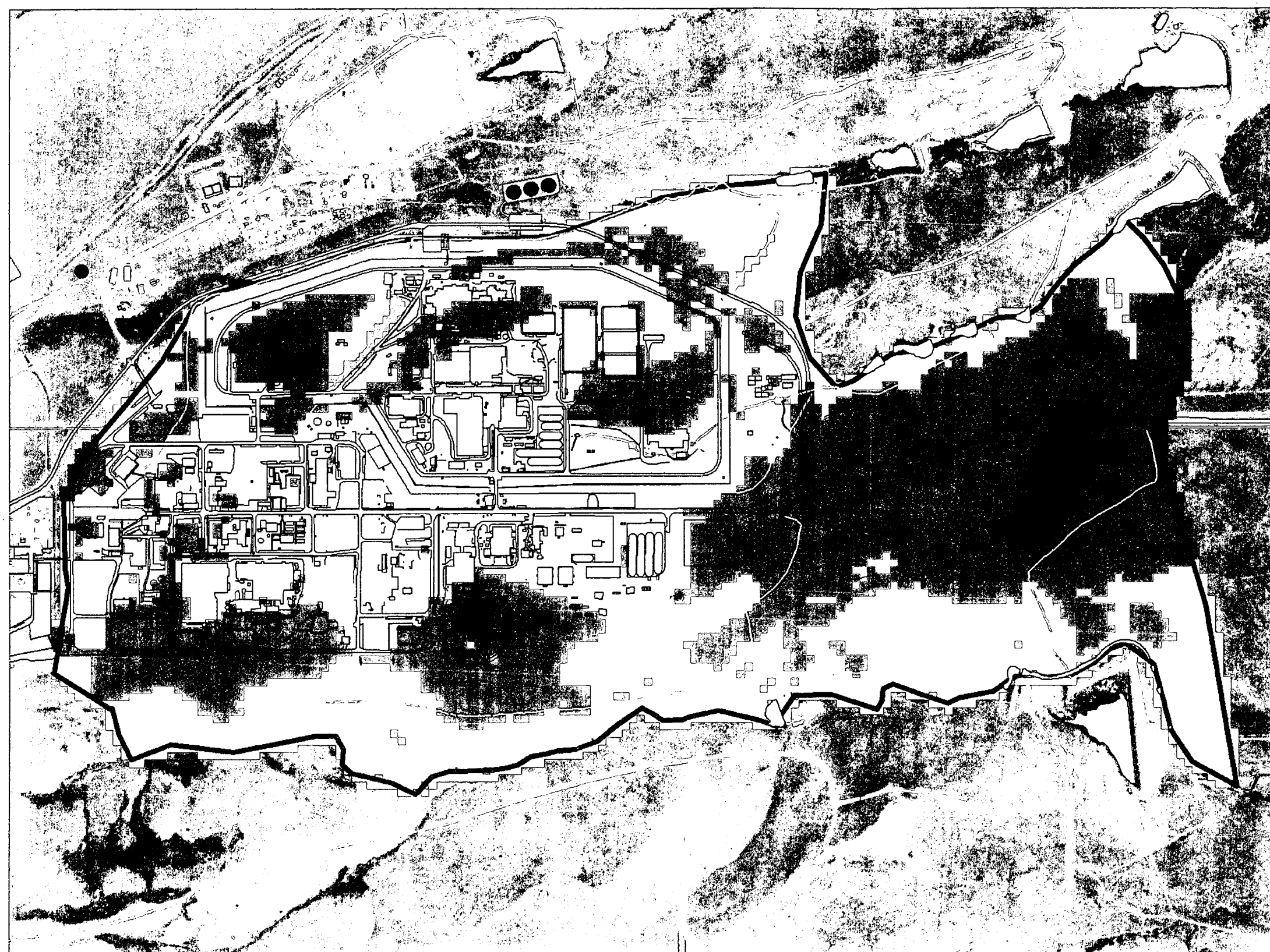
Negative numbers indicate simulated heads lower than observed.

**Figure 4.3. Comparison of Simulated and Observed Average Annual Heads.**



**Figure 4.4. Simulated Closure Configuration IA Surface Routing.**



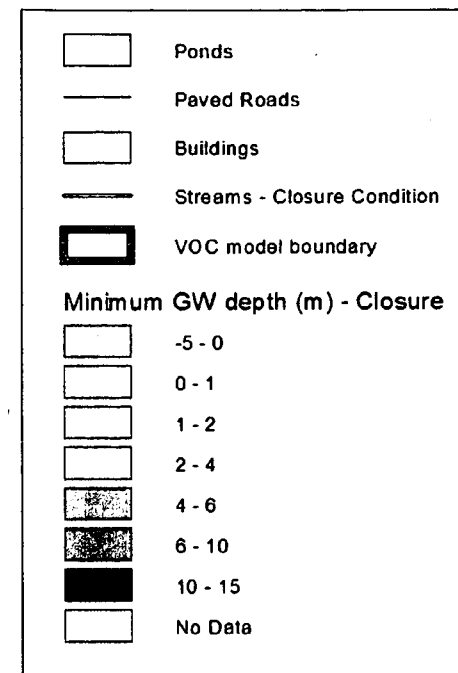
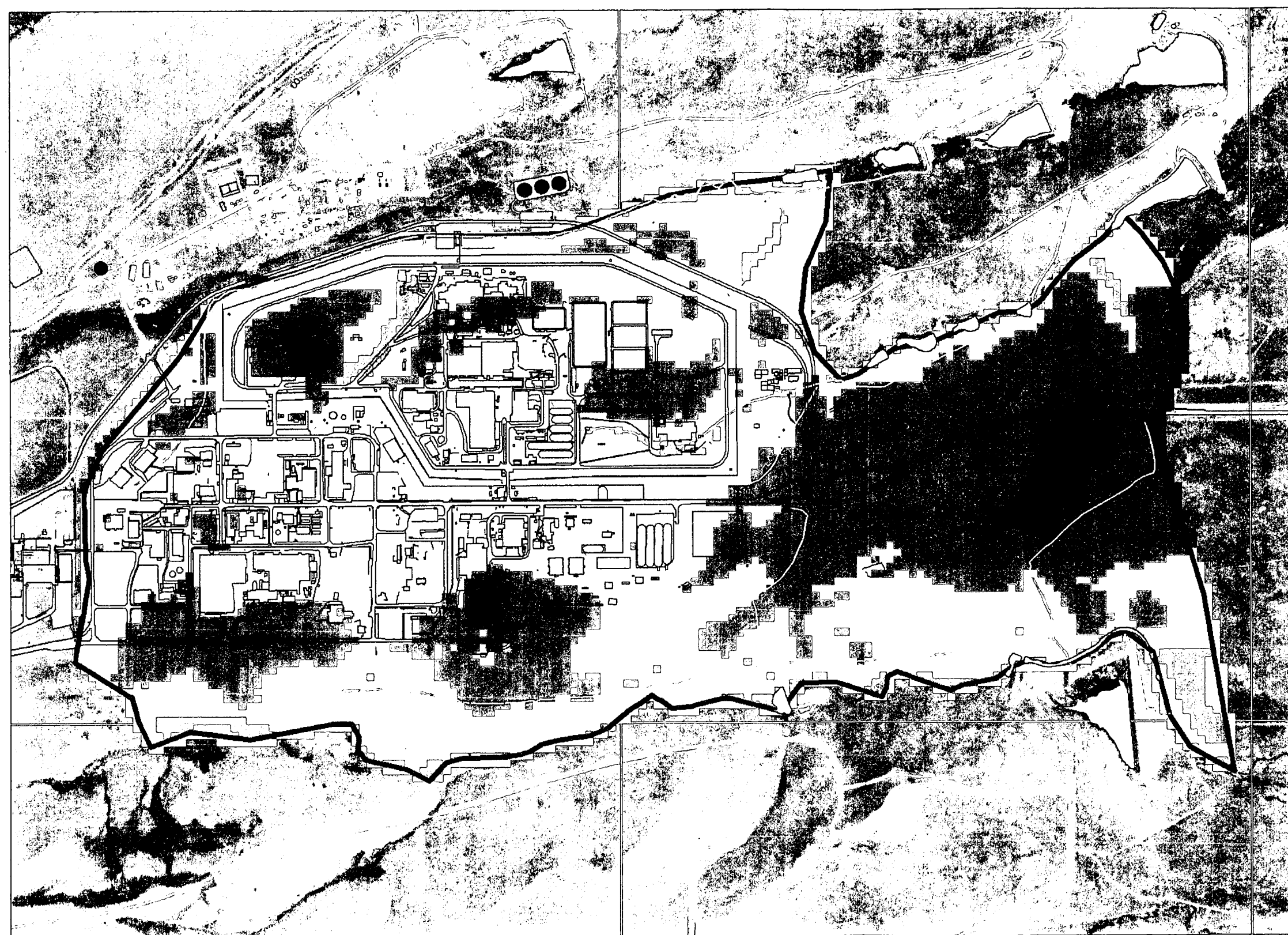


**Note:**  
Positive numbers indicate groundwater occurs below ground surface.

Negative numbers indicate groundwater intercepts ground surface (baseflow areas). See Figure 8.8 for actual model-predicted areas where groundwater discharges as baseflow, or as overland flow (seeps).

The lower right corner of the model area does not represent flow conditions accurately (just east of Pond C-2), but does not impact internal calculations near PSAs.

**Figure 4.5. Simulated Average Annual Groundwater Depths (m) - Typical Climate (WY2000).**



400 0 400 800 Feet

**Note:**

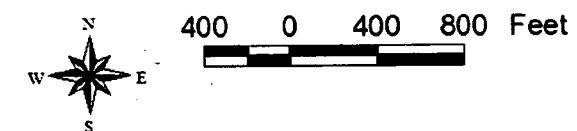
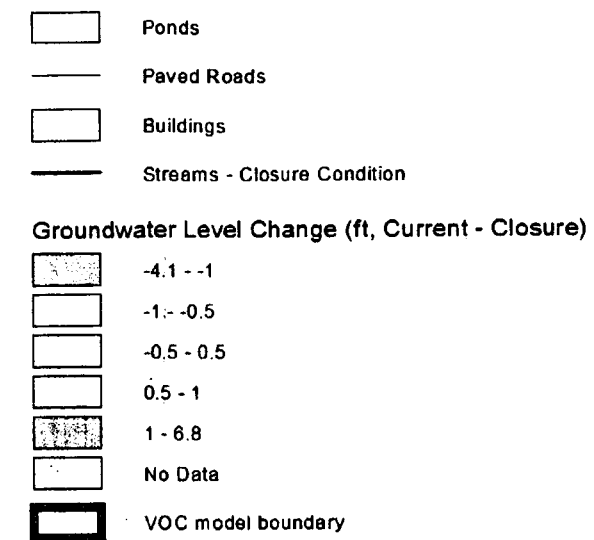
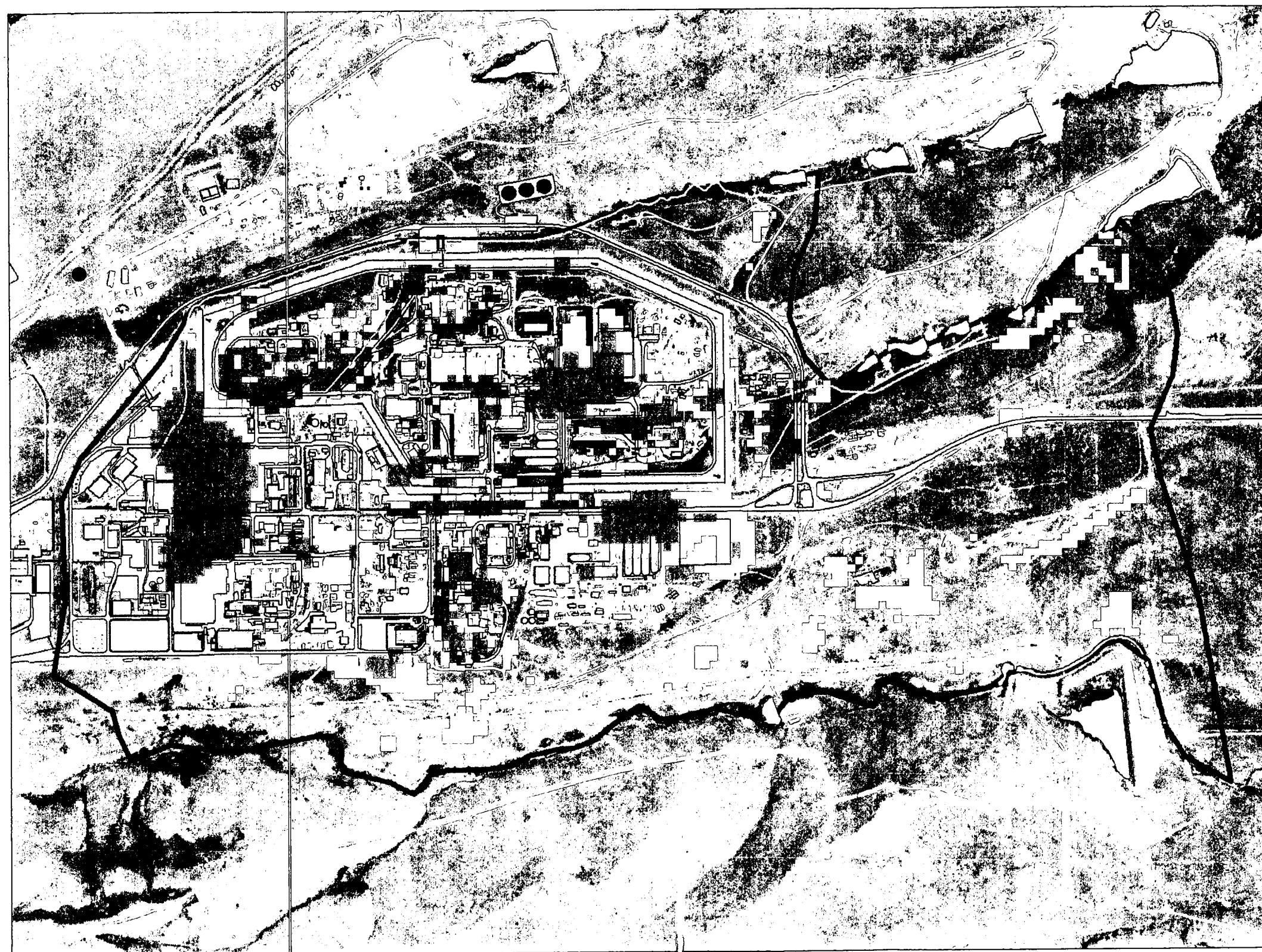
Positive numbers indicate groundwater occurs below ground surface.

Negative numbers indicate groundwater intercepts ground surface (baseflow areas). See Figure 8.8 for actual model-predicted areas where groundwater discharges as baseflow, or as overland flow (seeps).

The lower right corner of the model area does not represent flow conditions accurately (just east of Pond C-2), but does not impact internal calculations near PSAs.

**Figure 4.6. Simulated Minimum Annual Groundwater Depths (m) - Typical Climate (WY2000).**

Best Available Copy



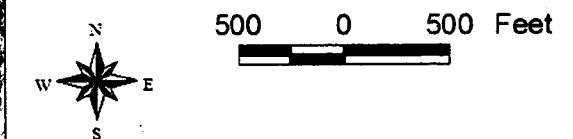
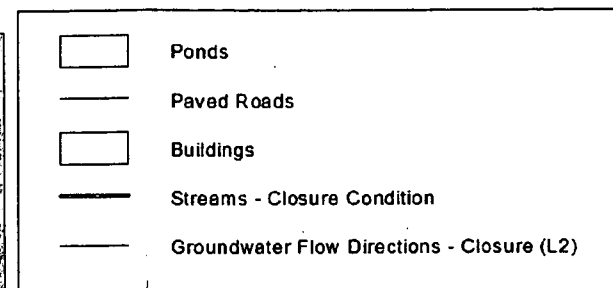
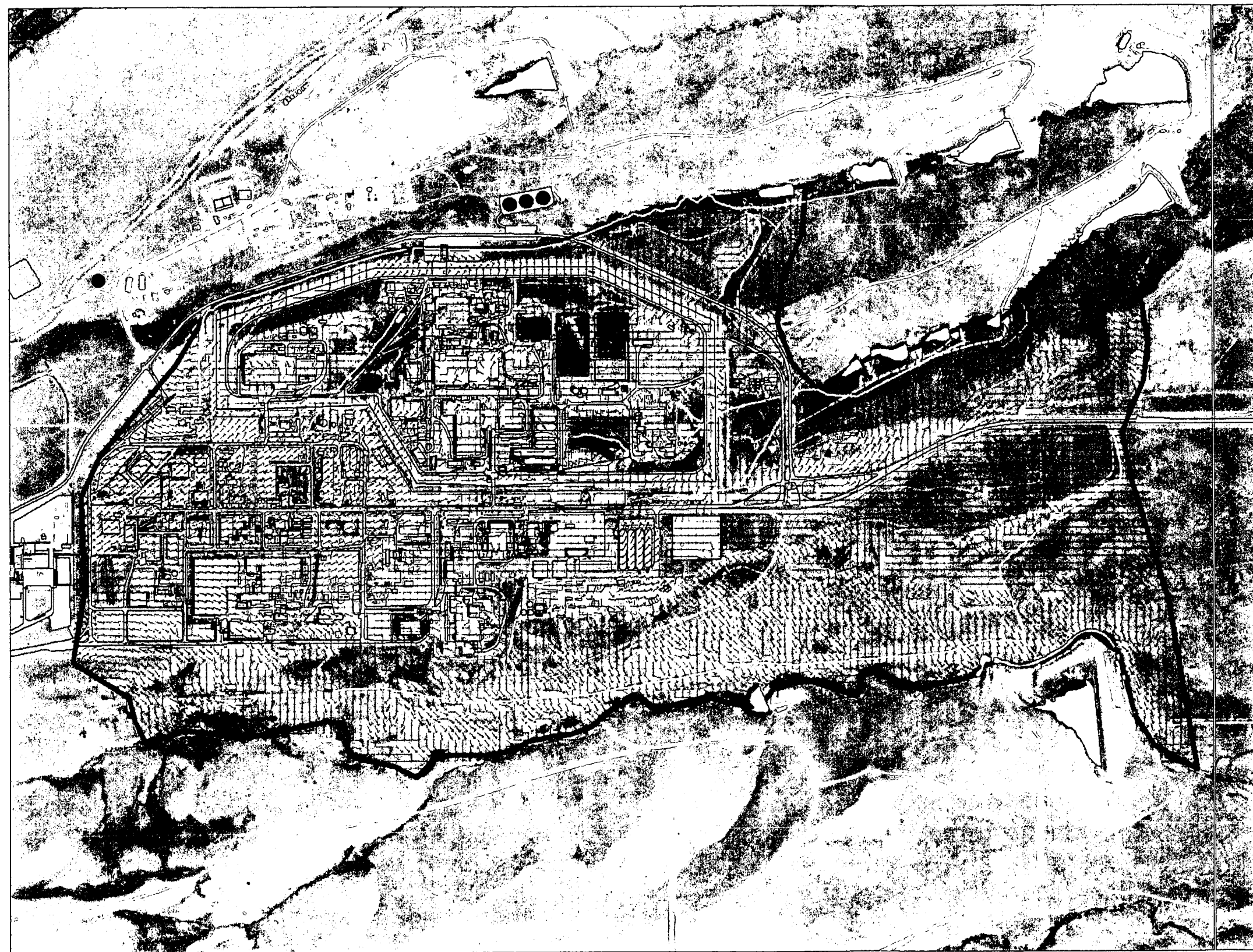
**Note:**  
Positive numbers denote areas where simulated groundwater depths increase from current (WY2000) conditions.

Negative numbers denote areas where simulated groundwater depths decrease from current (WY2000) conditions

All areas within the model, with no color, represent areas where simulated groundwater levels change less than 0.5 meters.

**Figure 4.7. Simulated Change in Average Annual Groundwater Levels from Current Conditions (m).**

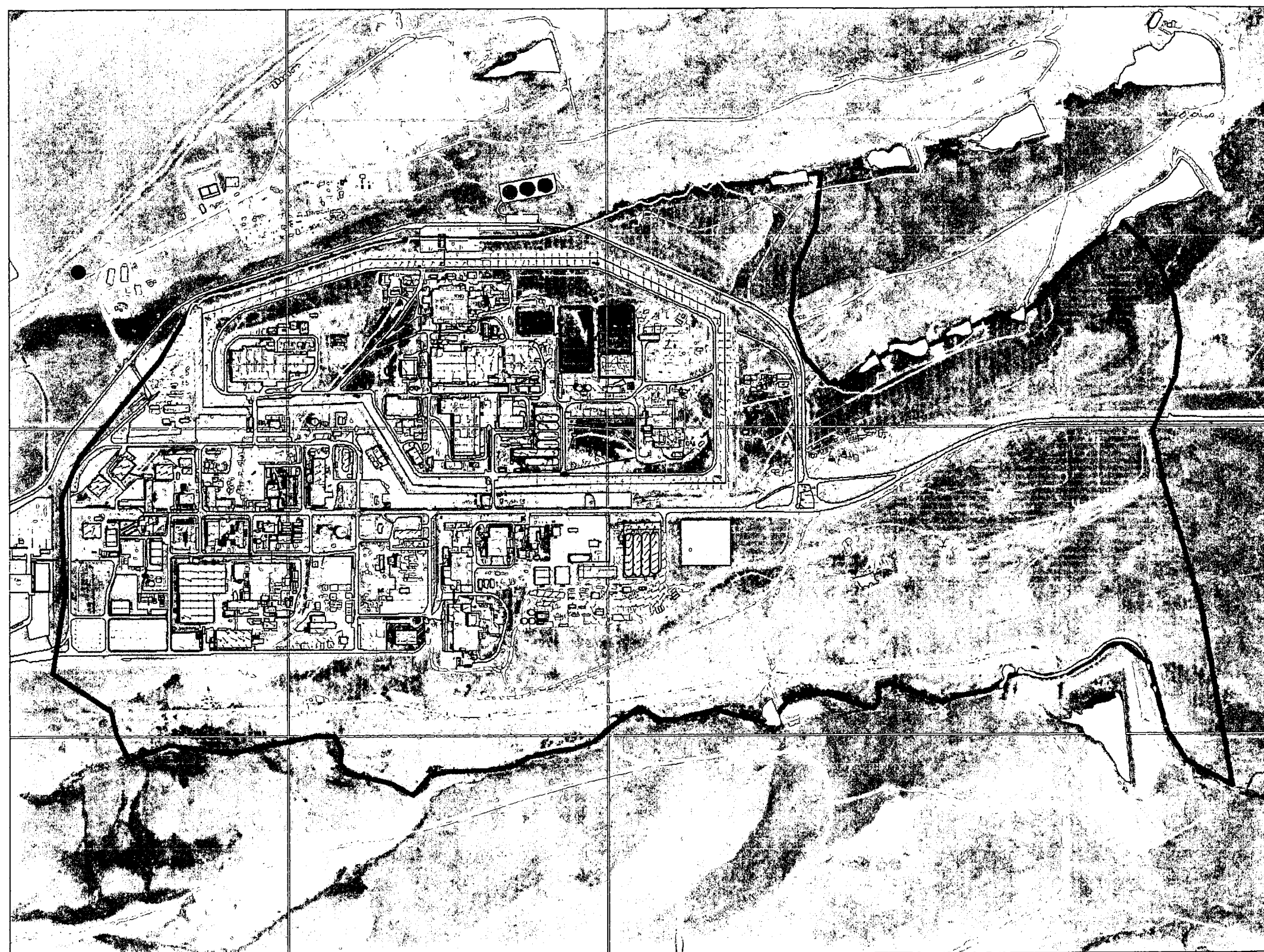










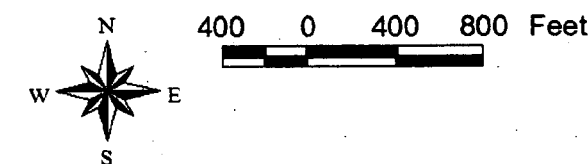
**Note:**  
Groundwater flow arrows are uniformly sized and do not reflect flow magnitudes.

Some areas show constant flow directions, either northward, or eastward (for example, east of the 903 Pad area). The unconsolidated material is unsaturated, or dewatered, in these areas and flow directions do not correctly reflect actual groundwater flow directions.

**Figure 4.8. Simulated Average Annual Groundwater Flow Directions - Lower Unconsolidated Material.**



-  Ponds
-  Paved Roads
-  Buildings
-  Streams - Closure Condition
-  Groundwater Flow Directions - Closure (L3)
-  VOC model boundary

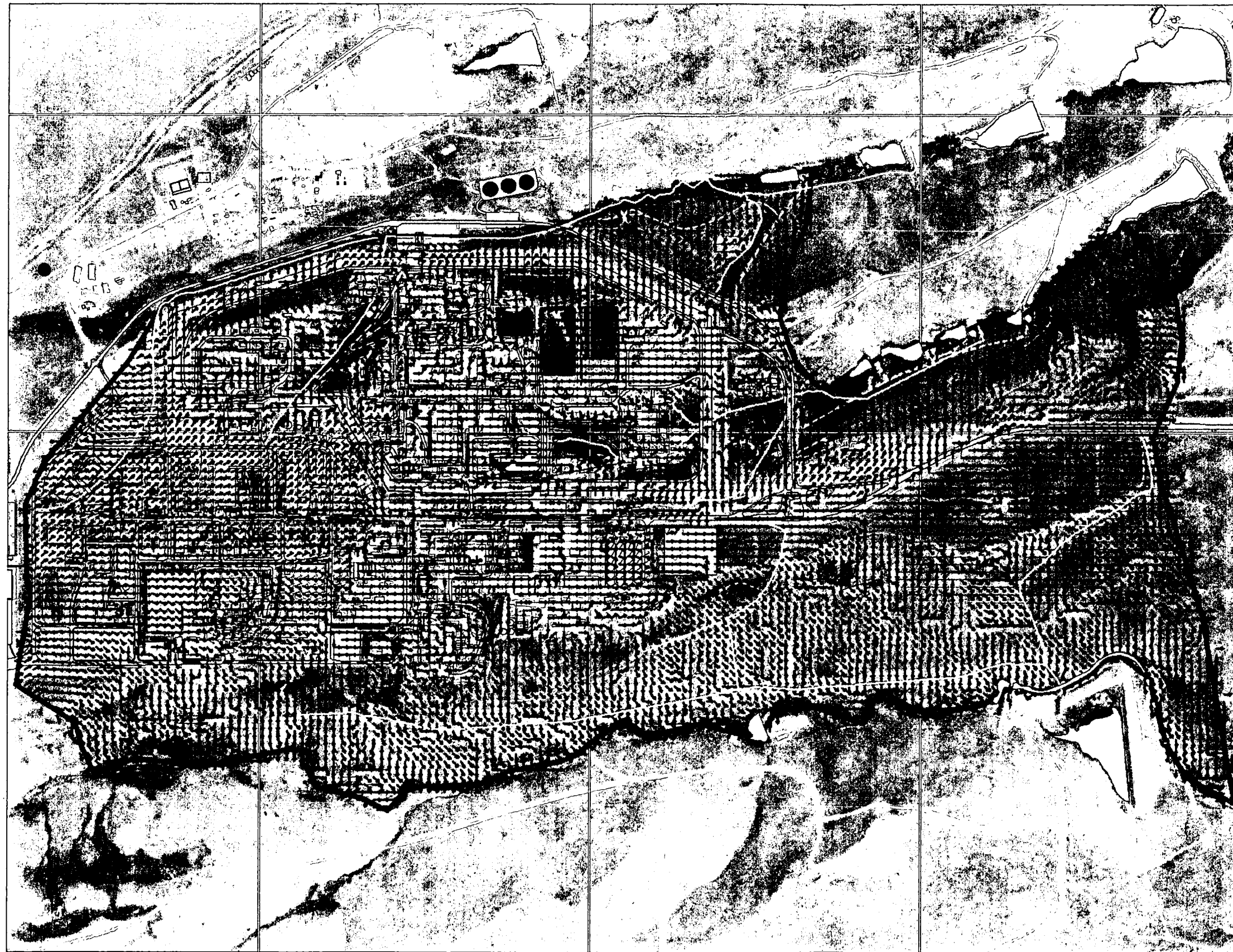









**Note:**  
Groundwater flow arrows are uniformly sized and do not reflect flow magnitudes.

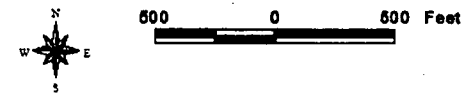
Some areas show constant flow directions, either northward, or eastward (for example, east of the 903 Pad area). The upper weathered bedrock material is unsaturated, or dewatered, in these areas and flow directions do not correctly reflect actual groundwater flow directions.

**Figure 4.9. Simulated Average Annual Groundwater Flow Directions - Upper Weathered Bedrock.**





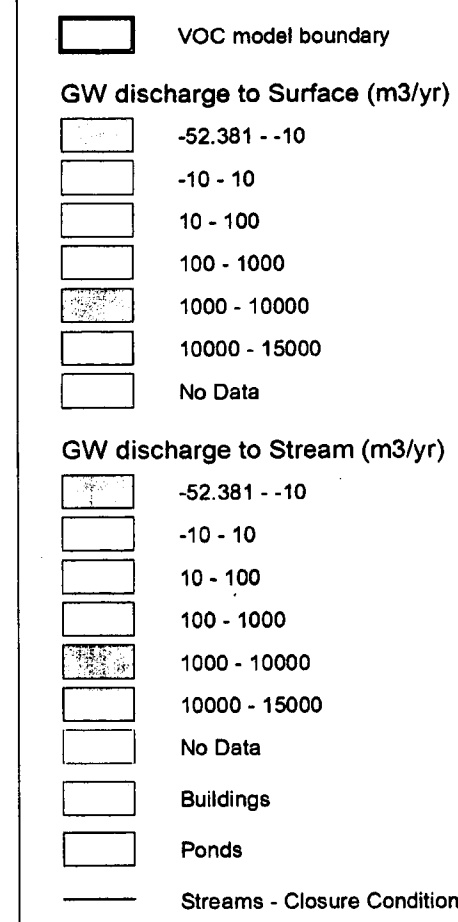
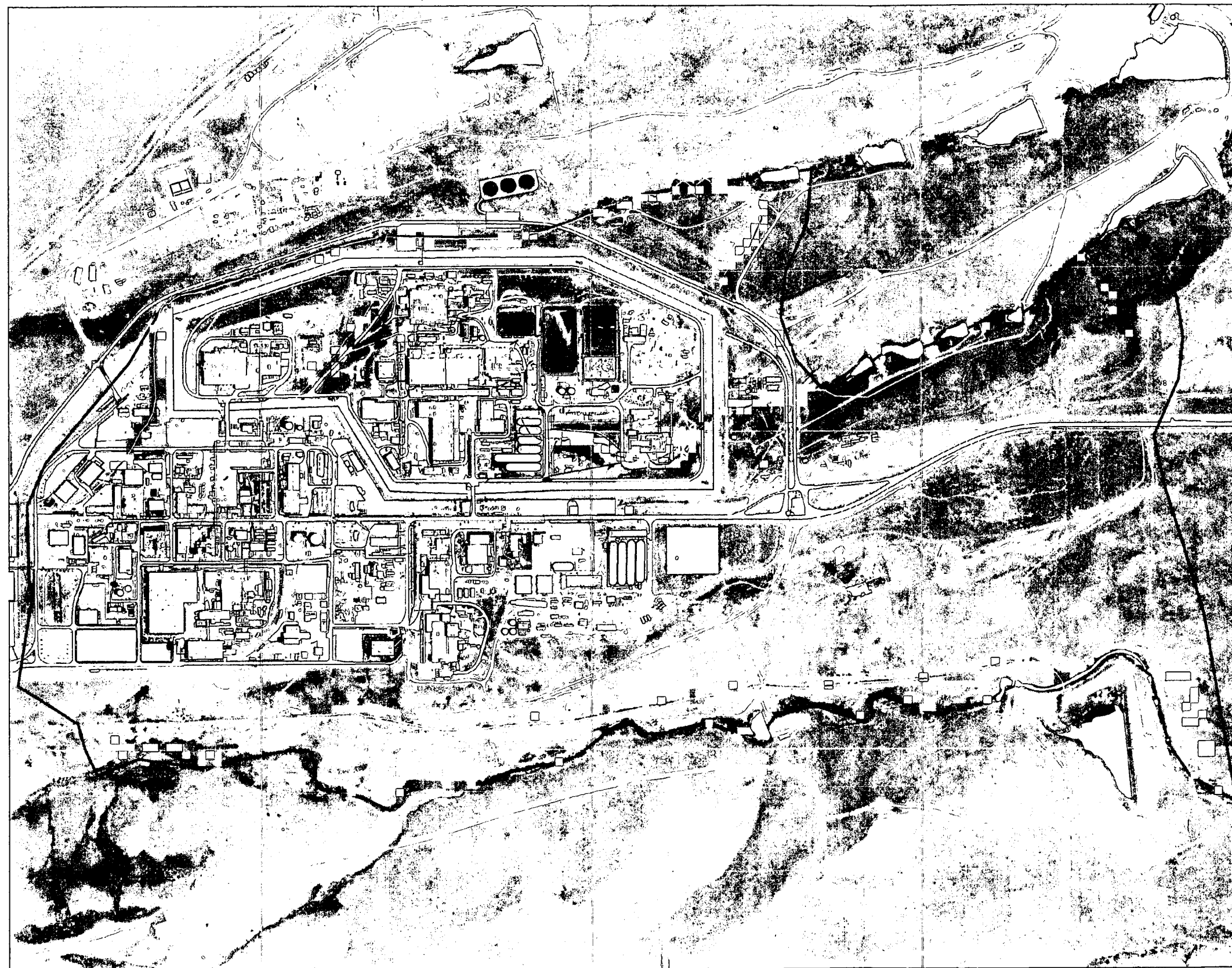
-  Ponds
-  Paved Roads
-  Buildings
-  Streams - Closure Condition
-  Groundwater Flow Directions - Closure
-  Groundwater Flow Directions - Current
-  VOC model boundary



**Note:**  
Groundwater flow arrows are uniformly sized and do not reflect flow magnitudes.

Some areas show constant flow directions, either northward, or eastward (for example, east of the 903 Pad area). The upper weathered bedrock material is unsaturated, or dewatered, in these areas and flow directions do not correctly reflect actual groundwater flow directions.

**Figure 4.10. Simulated Change in Groundwater Flow Directions - Upper Weathered Bedrock.**



**Note:**

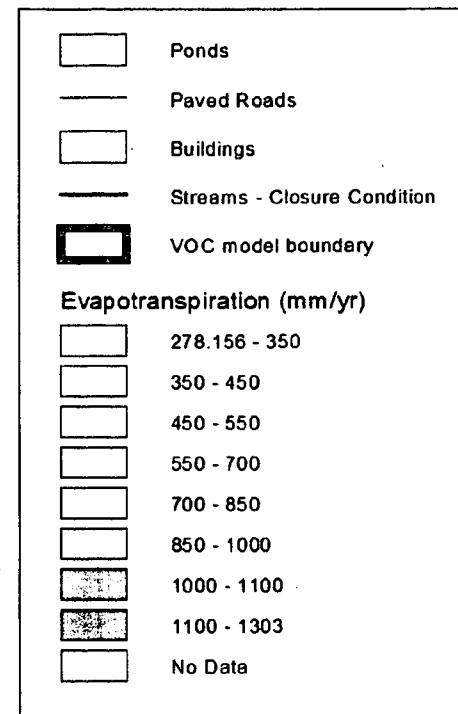
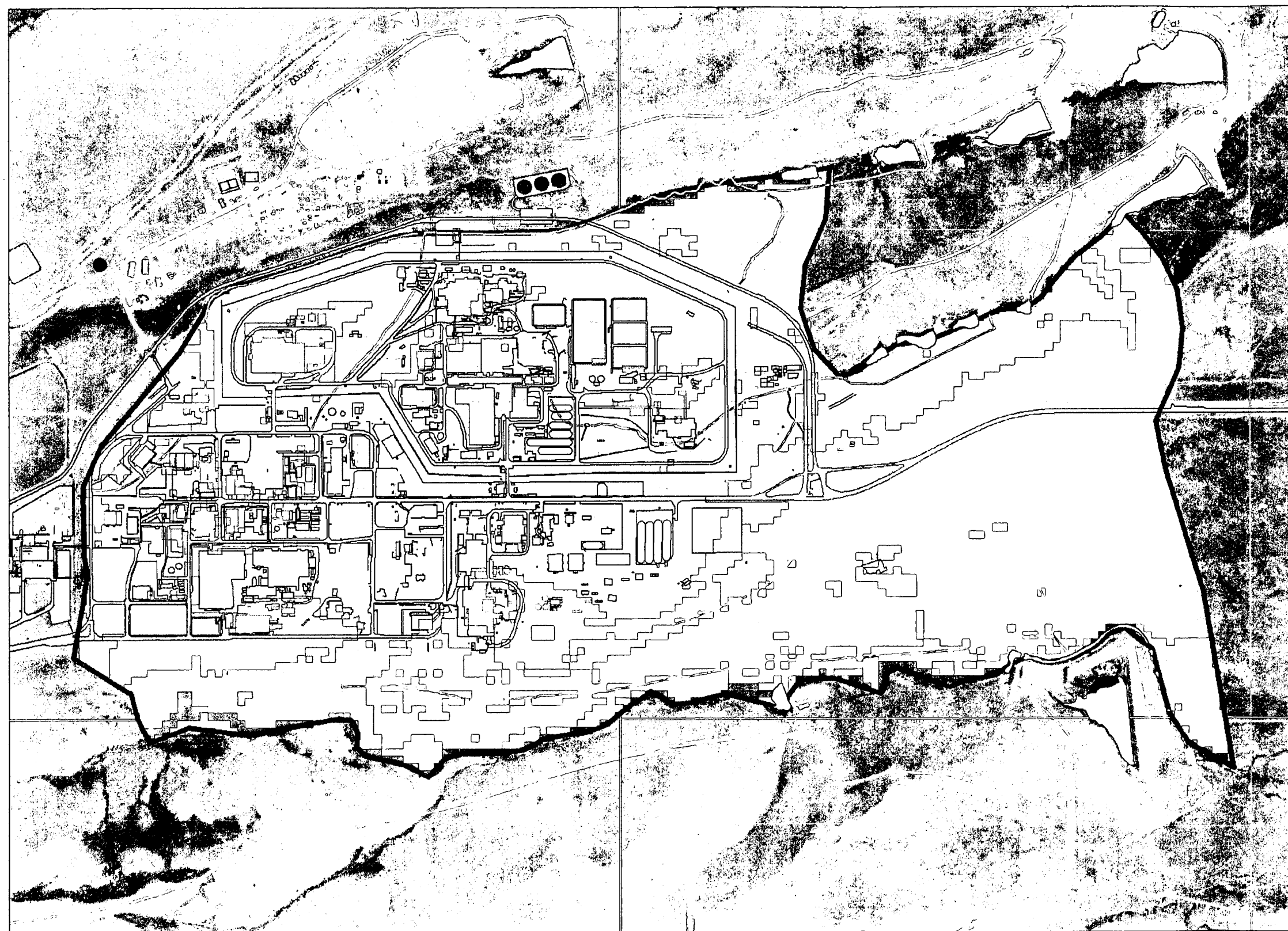
Positive numbers indicate areas where groundwater discharges to ground surface.

Negative numbers indicate areas where surface water recharges groundwater.

Discharge rates are in cubic meters per year. Rates do not reflect variability of discharge (or recharge) during the year.

North Walnut Creek and Woman Creek coincide with the integrated flow model boundary. As a result, discharge rates at the streams are not accurately calculated as streams in these areas were not simulated explicitly in the model.

**Figure 4.11. Simulated Annual Groundwater Discharge Areas.**



400 0 400 Feet

**Note:**

Evapotranspiration rates are given in mm/year. To convert to cubic meters/year, multiply by 38.7. Highest ET rates occur along stream areas, where groundwater is shallowest.

The lower right corner of the model area does not represent flow conditions accurately (just east of Pond C-2), but does not impact internal calculations near PSAs.

**Figure 4.12. Simulated Average Annual Evapotranspiration.**

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**Priority 1 and 2 Releases  
Location Map  
Central Portion of Site**

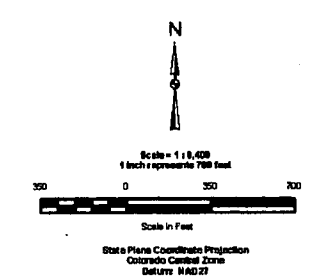
**Legend**

- Priority 1 (with Identifier Number)
- Priority 2 (with Identifier Number)

**Standard Map Features**

- ▭ Existing Building
- ▭ Demolished Building
- ▨ Solar Evaporation Ponds (SEPs)
- ▭ Lake or pond
- Stream, ditch, or other drainage feature
- Paved road
- Dirt road
- Trail
- Fence

**Note: Use Release Identifier Number to refer to table for information on each Release.**



U.S. Department of Energy  
Rocky Flats Environmental Technology Site

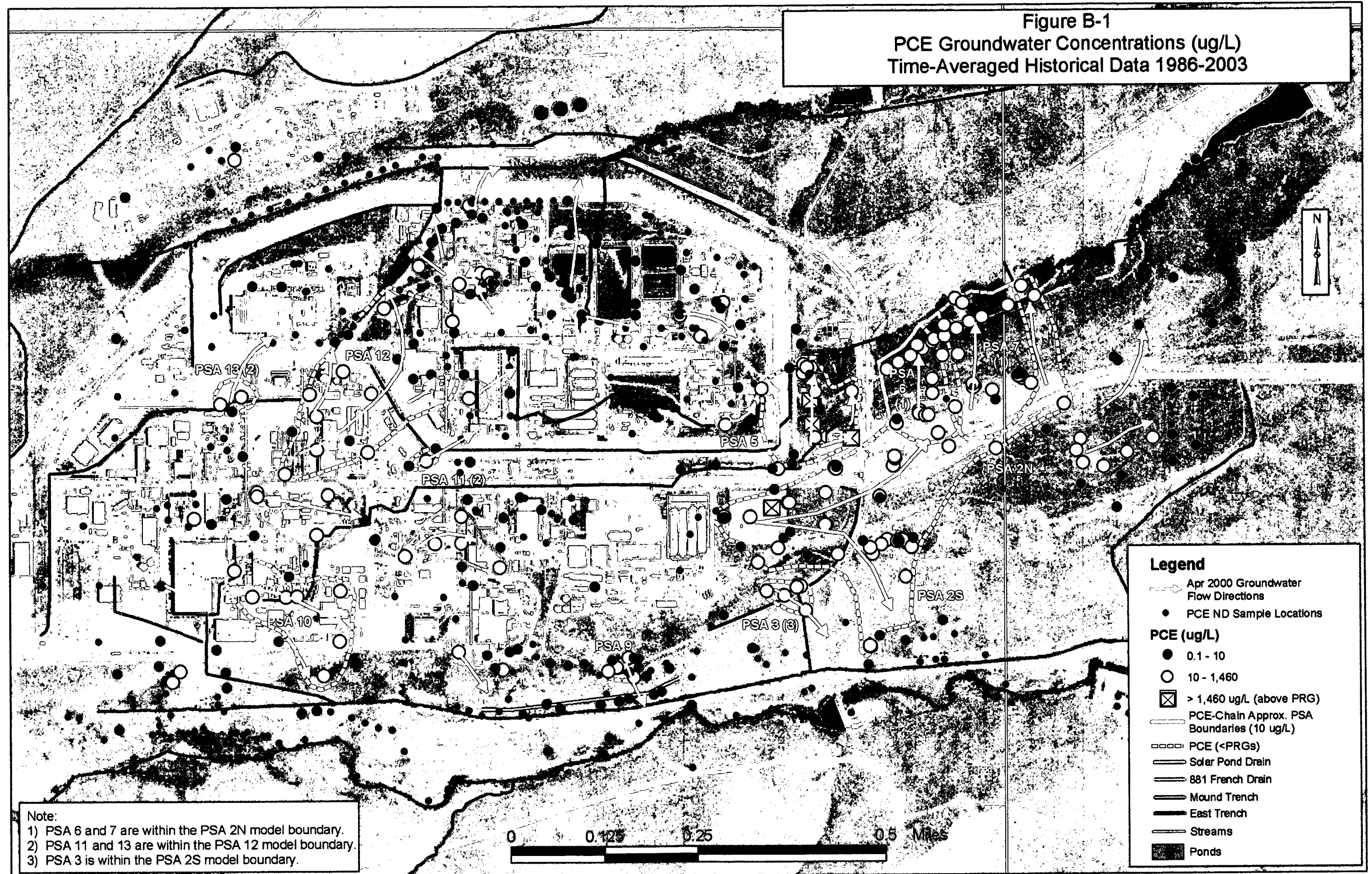
Prepared by: **URS**  
January 24, 2004

Prepared for: **LANL-100**

Best Available Copy

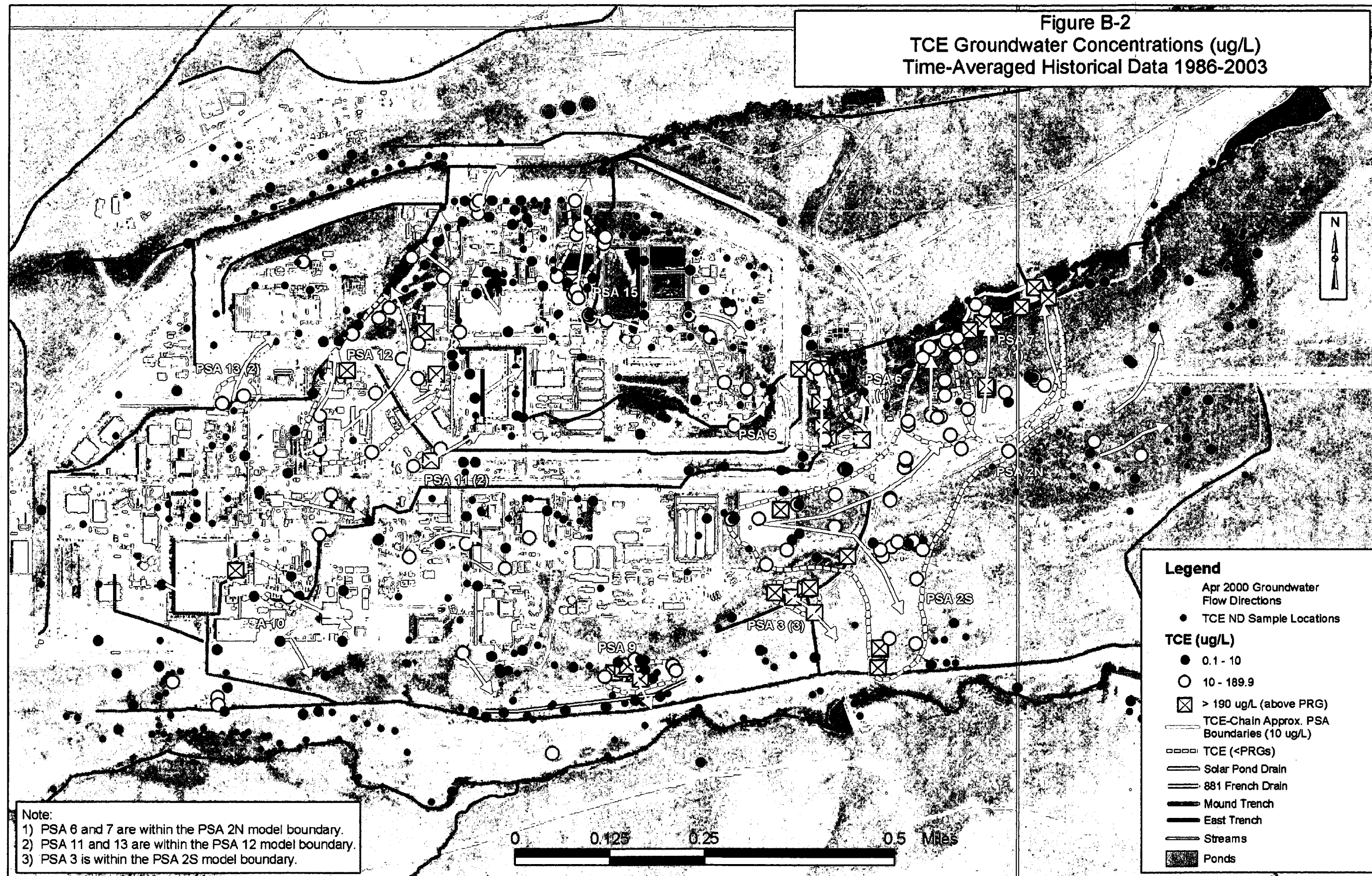
206

Figure B-1  
PCE Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003



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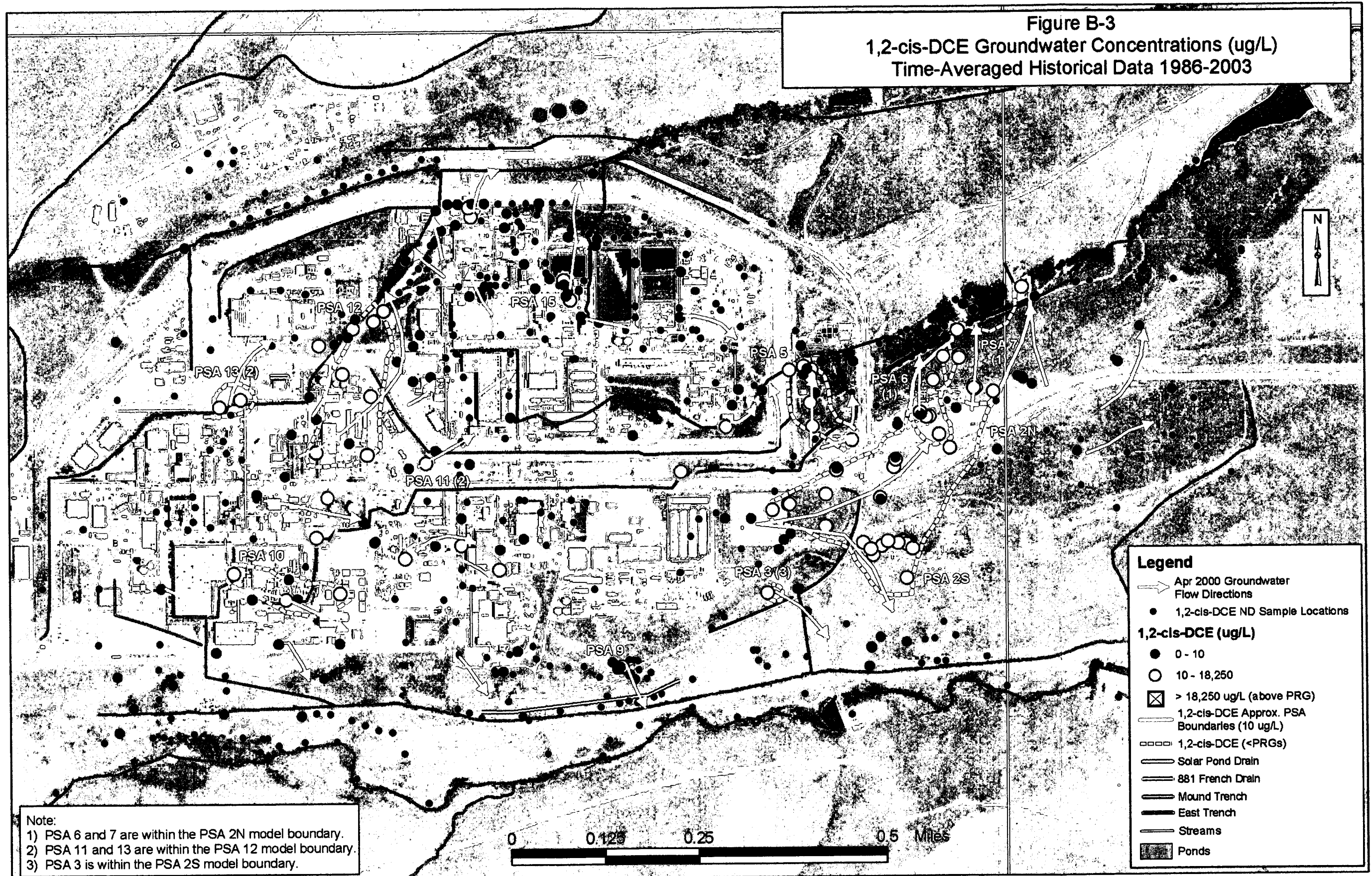
Figure B-2  
TCE Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003



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Figure B-3  
1,2-cis-DCE Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003



**Figure B-4**  
**Vinyl Chloride Groundwater Concentrations (ug/L)**  
**Time-Averaged Historical Data 1986-2003**

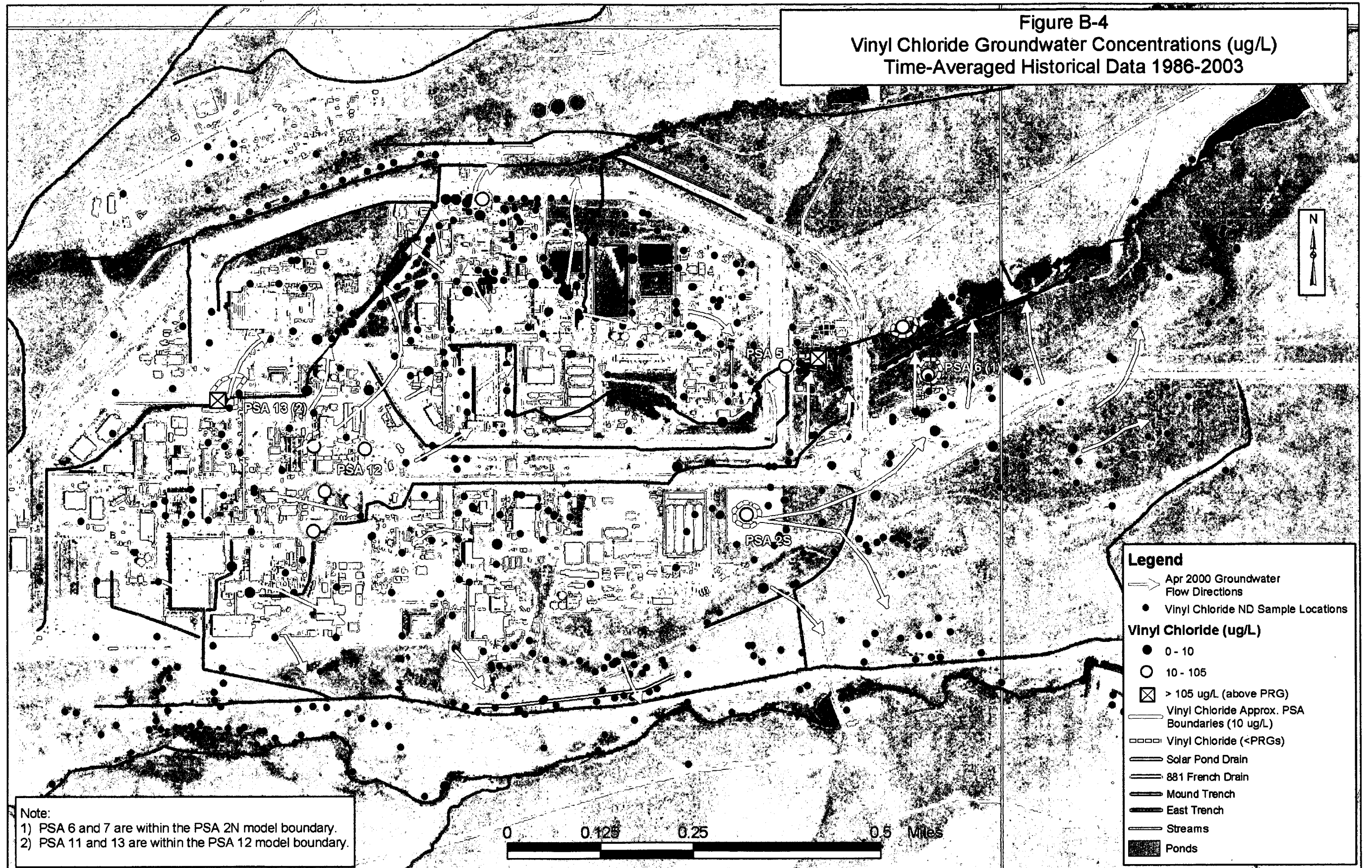




Figure B-5  
Carbon Tetrachloride (CCl<sub>4</sub>) Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003

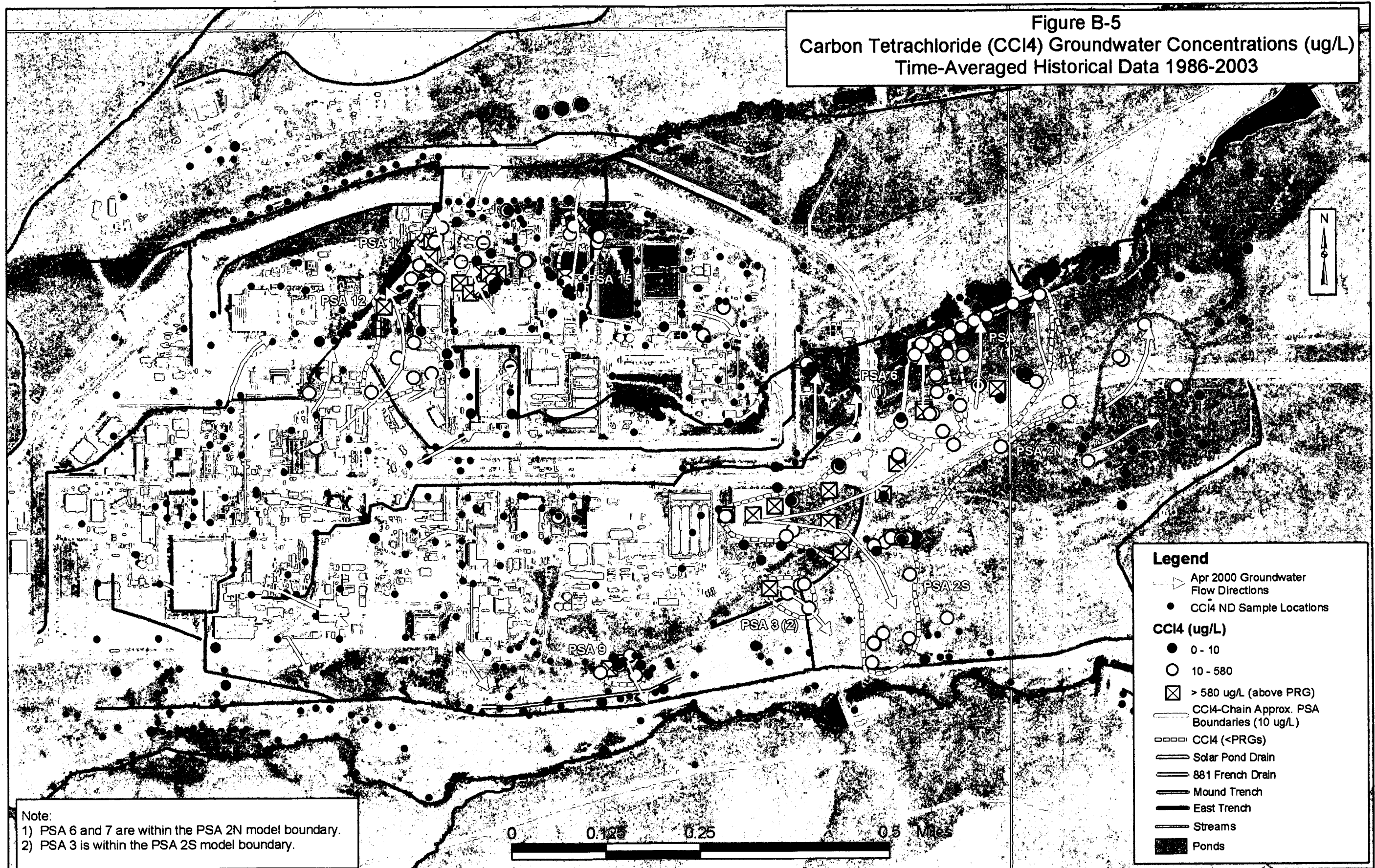
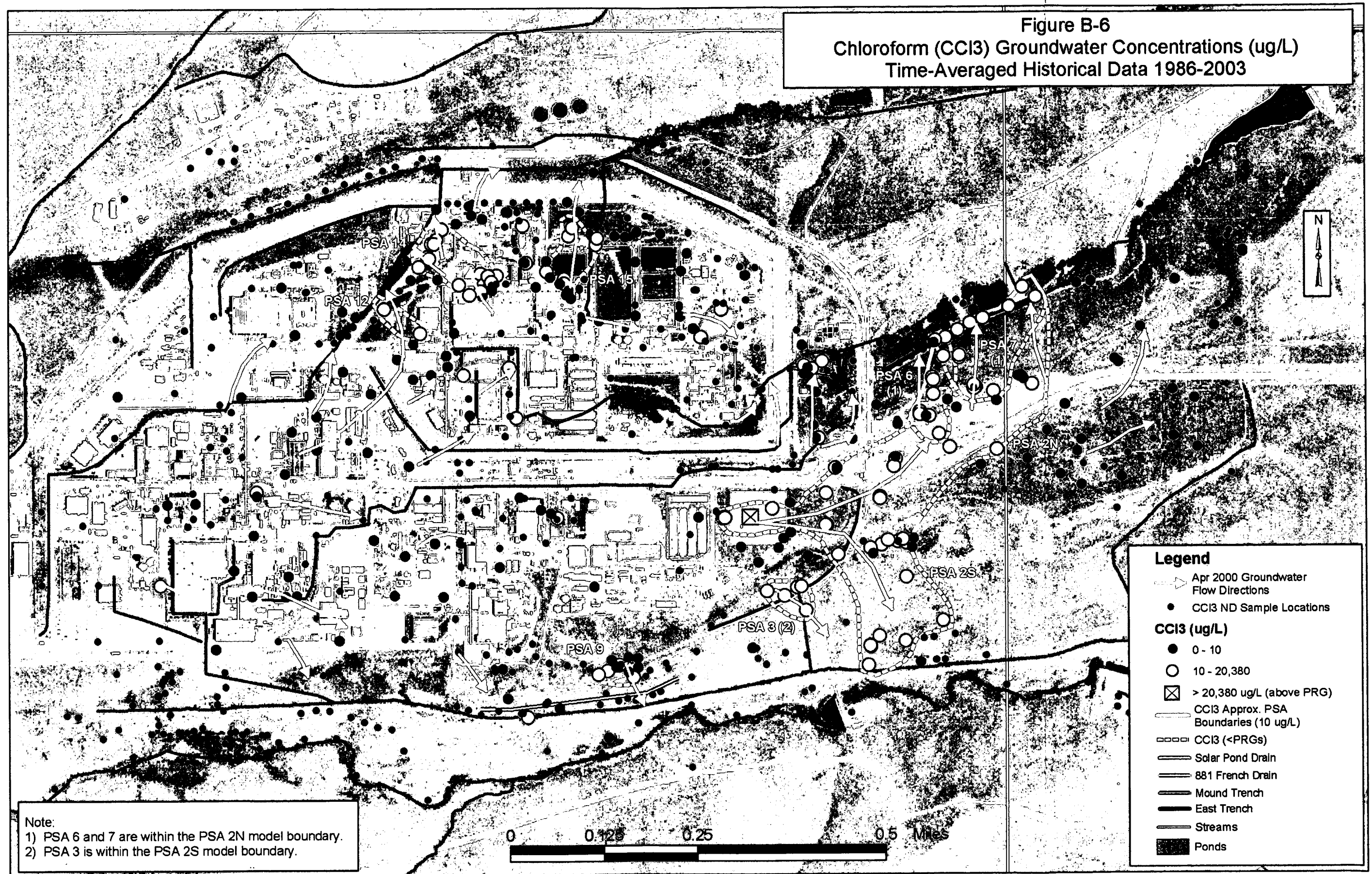


Figure B-6  
Chloroform (CCl<sub>3</sub>) Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003



**Legend**

- Apr 2000 Groundwater Flow Directions
- CCl<sub>3</sub> ND Sample Locations

**CCl<sub>3</sub> (ug/L)**

- 0 - 10
- 10 - 20,380
- > 20,380 ug/L (above PRG)

CCl<sub>3</sub> Approx. PSA Boundaries (10 ug/L)

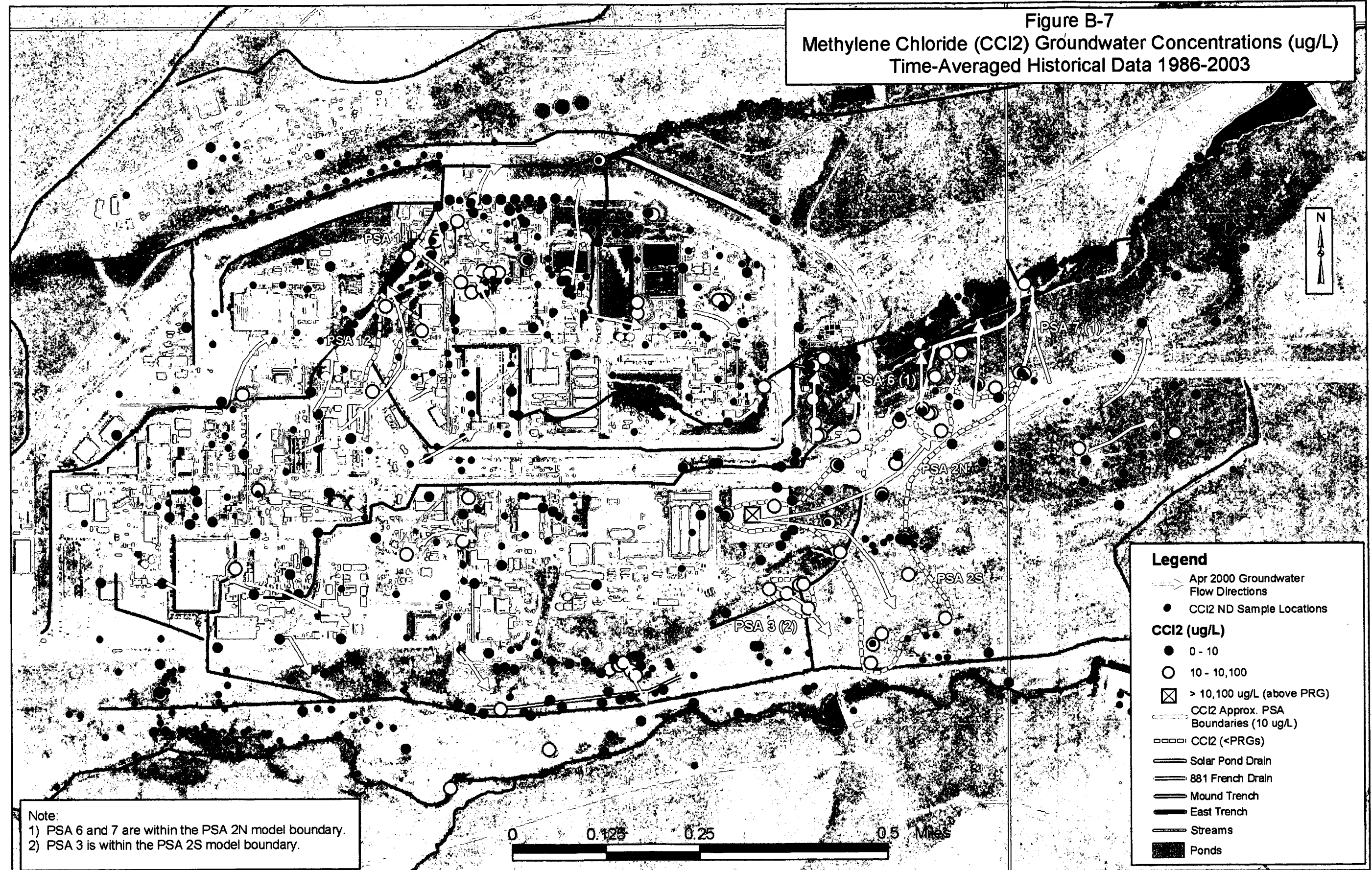
CCl<sub>3</sub> (<PRGs)

- Solar Pond Drain
- 881 French Drain
- Mound Trench
- East Trench
- Streams
- Ponds

Note:  
1) PSA 6 and 7 are within the PSA 2N model boundary.  
2) PSA 3 is within the PSA 2S model boundary.



Figure B-7  
Methylene Chloride (CCl<sub>2</sub>) Groundwater Concentrations (ug/L)  
Time-Averaged Historical Data 1986-2003



- Legend**
- Apr 2000 Groundwater Flow Directions
  - CCl<sub>2</sub> ND Sample Locations
  - CCl<sub>2</sub> (ug/L)**
    - 0 - 10
    - 10 - 10,100
    - ⊠ > 10,100 ug/L (above PRG)
  - CCl<sub>2</sub> Approx. PSA Boundaries (10 ug/L)
  - CCl<sub>2</sub> (<PRGs)
  - Solar Pond Drain
  - 881 French Drain
  - Mound Trench
  - East Trench
  - Streams
  - Ponds

Note:  
1) PSA 6 and 7 are within the PSA 2N model boundary.  
2) PSA 3 is within the PSA 2S model boundary.